

14 November 2019

Ref: 045500REP02\_Rev0

BHP Iron Ore  
125 St Georges Terrace  
PERTH, WA 6000

Attention: Nicole Romanczuk  
Study Specialist - Iron Ore / Asset Projects / Studies

Dear Nicole

**RE: MARILLANA CREEK INFILTRATION PROJECT  
GEOTECHNICAL MAPPING AND INVESTIGATION REPORT**

4DGeotechnics are pleased to present the results of the geotechnical mapping and investigations relating to the Marillana Creek Infiltration Project. This report presents the results of the mapping and investigations and an updated appraisal of the engineering geological materials and conditions within the area of interest. Engineering geological maps and design tables are presented for the purposes of incorporation into your Wetting Front modelling.

It should be noted that as mapping the creek channel itself was the primary goal of the mapping, areas beyond the creek were mapped in limited detail and interpretations have been extrapolated from the areas directly observed in the field. Ground breaking was limited to select accessible locations and has been extrapolated along the wetting front based on mapping observations.

This report supersedes our previous Desktop Study Geotechnical Report (Ref: 045500REP01\_Rev0). All the relevant data from the previous desktop report has been carried through and updated for this report with information gathered from the field mapping and ground breaking investigations.

Thank you for the opportunity to provide BHP with this report and 4DG look forward to further assisting you with this, or any other project as required. Please do not hesitate to contact Valdimar Jonsson or myself if you have any queries.

**4DGeotechnics Pty Ltd**



**Ian H Lewis**  
**PRINCIPAL ENGINEERING GEOLOGIST**

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**BHP IRON ORE**

**MARILLANA CREEK INFILTRATION PROJECT  
GEOTECHNICAL MAPPING AND INVESTIGATION REPORT**

**REF: 045500REP02\_REV0**

**14 NOVEMBER 2019**



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## 1.0 INTRODUCTION

As part of a planned dewatering project for the Mining Area C Mine (MAC), BHP Iron Ore (BHP) plan to pump excess water from mine dewatering into a tributary of the Marillana Creek.

A wetting front is defined as the interface between soil that is unchanged from the initial state of natural moisture and the newly wetted zone resulting from an infiltration or irrigation event. To support calculations of potential allowable discharge rates, volumes and locations, an understanding of the *in situ* geological conditions is required along the potential wetting front, which preliminary hydrological modelling suggests could reach Flat Rocks, west of the current BHP Yandi Mine open pit operations.

BHP have engaged 4DG to conduct a study of the geological conditions in the area of interest to support both the modelling of subsurface conditions and calculations of the wetting front movement along the alluvial channels through which the discharge water will flow.

4DG proposed a three phase approach to gather this information, consisting of a preliminary Desktop Geotechnical Study Phase, followed by a Field Mapping Phase and then a Ground Breaking Investigation (GBI) phase. The Desktop Geotechnical Study Phase created a baseline interpretation of the geological and geomorphological units in the area of interest which formed the base map for the field mapping phase, during which the characteristics of the units and their boundaries have been described in more detail. Updated geological maps have been included as Appendices A and B of this report. Additionally, to assist the assessment of the wetting front development, the streambed material types forming the alluvial channels have been sub-divided into groups (termed Domains for this report) corresponding to their variability in morphology and expected water infiltration and flow characteristics. The GBI phase investigated selected, readily accessible locations to define sub-surface profiles, as well as understand the typical creek bed material properties and permeability characteristics to support the infiltration modelling.

## 2.0 MAPPING

Mapping of the Marillana Creek and its tributaries through the area of potential saturation was conducted between June 13 and June 25, 2019 by two 4DG Engineering Geologists. The mapping area extended from the beginning of the incised tributary near the intersection of the Yandi Mine Access Road and the Great Northern Highway, through to the Flat Rocks area to the West of the BHP Yandi Mine. Figure 1 provides an overview of the mapped area.

### 2.1 Methodology

Mapping was carried out on foot, travelling mainly along the creek bed, with limited departures beyond the banks where required to identify rock outcrops where present in the banks, or adjacent hills. Additional observations of geology within the area of interest were made by vehicle traverse along existing accessible tracks.

As part of the mapping, particular attention was paid to the following:

- Identification of primary and secondary channels;
- Variations of creek bed materials along the primary and secondary channels, particularly variations with potential to influence permeability;
- Morphology of the channels and apparent typical flow conditions;
- Bedrock units intersected by the primary and secondary channels;
- Creek bed materials;
- Presence of vegetation;

- Indications for turbulent water flow;
- Signs of calcite precipitation; and
- Orientation of geological structural bedding and defects where suitable outcrop was encountered.

## 2.2 Engineering Geological Overview

The engineering geological units interpreted during the desktop study are presented on Engineering Geological Maps in Appendices A and B. The summary of encountered engineering geological units and their association with morphological forms are presented in a generalised stratigraphic column (Table 1) where interpreted units are listed generally from youngest to oldest.

### 2.2.1 Surficial Engineering Geological Units

Surficial unconsolidated material, including transported sediments, duricrust and decomposed weathered bedrock, varies significantly and may directly relate to the topography. Typically, coarse grained deposits occur on the steeper toe-slopes beneath cliffs (talus deposits) or adjacent to residually weathered hills. Coarser grained surficial materials are most common on steep terrain and have been deposited mainly by gravity and less so by water, and are typically mapped as colluvial slope deposits (Cg, Plate 1). Grainsizes typically decrease as the terrain becomes flatter and slopes are gentler. Slope deposits also include a gradation from residual soils (developed in place from weathering) to transported deposits such as colluvium and alluvial wash material. Surficial deposits also occur in alluvial / drainage features with adjacent flatter areas frequently comprising floodplain and older alluvial deposits.

Gravel-rich material in a fine grained matrix encountered on lower gradient downslope areas are produced mainly by slope wash and less so by gravity. These colluvial wash (Cw, Plate 2) deposits generally become finer towards the distal part of wash slopes due to preferential transportation of fines. Variations across wash slopes may accrue and include localised coarse gravel-rich areas, commonly on minor interfluves; and localised fines-rich areas, near wash channels and where surface flow is concentrated. Surface gravel lags of variable thickness are also common.

Low-lying areas can be wash-plains, with poorly developed drainage lines, dominated by distal slope wash material (Aw, Plate 3). The material is formed mainly by slow water wash processes sub-parallel to the main river channel. Surface gravel lags become rarer and accrue along areas where surface flow is concentrated.

Floodplain alluvial proximal (Fp, Plate 4) and distal (FpD, Plate 5) deposits are encountered adjacent to dominant primary alluvial (Al, Plate 6) and secondary (AIS, Plate 7) alluvial channels of the Marillana Creek and its tributaries. Floodplain deposits comprise mainly fine grained alluvial material, although coarser materials may be encountered where older active channels have been buried, or left behind by erosive channel migration. Gravel and sand material of Al and AIS dominate in seasonally active drainage lines, whereas channel margin and in-channel bars (AIB, Plate 8) typically comprise residual alluvial deposits which are no longer part of the typically active alluvial channels. Where alluvial channels intersect and expose duricrust and bedrock in the base of the channel these have been mapped as AIR (Plate 9).

The exposed bedrock surface along the corridor has been weathered and altered over geological time. This has resulted in the development of a secondary cemented duricrust in some areas. The presence of the duricrust obscures the underlying weathered rock characteristics. The main types of duricrust encountered in the area are:

- Calcrete (Czk, Plate 10) is sheet carbonate material that mainly comprises gravel and sand cemented together by calcium carbonate. It is red, pink, brown or white in colour. The material sometimes forms tabular rock and often contains voids and cavities. The calcrete outcrops are frequently overlain or fringed by calcrete soil deposits (Plate 11)

thought to be either poorly developed or more likely weathered calcrete deposits. In some areas ancient alluvial gravels (Aa, Plate 12) form partially consolidated rounded mounds. Run off and collection of fine calcareous silts and clays presumed to originate from dolerite gravels in low lying areas on the calcrete produces some areas described as Calcareous Gilgai (C\_Gilgai, Plate 13) characterised by frequent crab holes, dark brown appearance and frequent calcrete cobble float. The unit occasionally intersects the major active drainage lines but primarily forms elevated erosional surfaces, commonly overlying Robe Pisolite (Tp, Plate 14). The strength of the calcrete may vary from low to high depending on weathering and the degree of cementation.

- Robe Pisolite (Tp), often referred to more generally as Channel Iron Deposits (CID), originated as deposits within palaeochannels incised into the older bedrock. The material comprises iron-cemented, pisolitic limonite and haematite deposits, with minor scattered fossil wood and thin clay layers. Dependent on the degree of weathering, it can have low (extremely weathered Tp) to high strength (distinctly weathered Tp). The rock mass has weakly cemented zones and voids, and contains occasional thin clay layers.

It is important to note that the duricrust materials have the potential for strength inversion in the geological profile, with higher strength, cemented materials overlying lower strength units, uncemented superficial soils or weathered rock mass. The degree of cementation can vary widely over short distances and also has an impact on the mass permeability / porosity of the rock mass.

## 2.2.2 Bedrock Engineering Geological Units

The bedrock geology of the area comprises metamorphosed sedimentary and volcanic rocks of Weeli Wolli Formation in the eastern part of the area of interest, and localised Brockman Formation in the western part.

Banded Iron Formation (BIF, Plate 15) units within the Weeli Wolli and Brockman Formations comprise banded iron, chert, pelite, and jaspilite. The rock strength usually ranges from low to high. These are sedimentary units originally and the bedding forms a pervasive fabric through the rock mass, which, when combined with at least three sub-vertical defect sets, typically results in a poor rock mass quality. Many of the joints are open near surface and may be infilled with clay, gravel and rootlets. Shaly BIF (Plate 16) within the Weeli Wolli Formation consist mainly of pelite and is typically medium strength and subject to preferential erosion.

The dolerite units (Plate 17) are intruded within the Weeli Wolli Formation and form multiple sills between BIF units and may vary from very low strength when extremely weathered, to high strength when slightly to distinctly weathered. Dolerite can produce high plasticity fines and can be highly prone to erosion when extremely weathered. Where areas of fresh dolerite appear, the rock may range from high to very high strength and may exhibit a high resilience to weathering.

**TABLE 1: GENERALISED STRATIGRAPHIC COLUMN**

Geological Time / Supergroup / Group	EGU <sup>1</sup> Name	EGU <sup>2</sup> Code	Description <sup>3</sup>	Reference to WA Geological Survey Map (SF50-12)
Quaternary	Active Alluvial Deposits of Dominant/Main Channels	AI	Well-defined channels of ephemeral rivers, creeks and drainage lines subject to periodic water flow during seasonal rainfall events. In general, alluvial deposits consist of unconsolidated/loose silt (minor to none), sand, gravel and cobbles. Coarse material (boulders and cobbles) occur where rock sources are nearby. The deposits are formed mainly by high velocity flow. In some cases, shallow ponds of water may be encountered.	Qa
	Active and Stable Bars and Island Alluvial Deposits (i.e. heavy vegetated elevated areas)	AIB	Raised sediment deposits within channels and in some cases floodplains. These comprise: <ul style="list-style-type: none"> <li>“channel margin” deposits in forms of various bars and banks to channels; and</li> <li>“in-channel deposits” forming islands and mid channel bars.</li> </ul> The deposits mainly consist of unconsolidated/loose sand, and gravel. Fines (including silt) and sand/gravel material content varies and depends on morphology of the channel, sources of the material and flow regime.	
	Active Alluvial Deposits of Secondary Channels	AIS	Secondary channel deposits occur: <ul style="list-style-type: none"> <li>Along outside/elevated parts of dominant/main channels;</li> <li>Within proximal floodplains/flood ways; and</li> <li>Along tributary channels.</li> </ul> The deposits mainly consist of unconsolidated/loose sand, and gravel, in some cases of the coarser material, where rock sources are near. In general, deposits are formed under high flow conditions where the water flow exceeds the capacity of the primary channel and flow spreads across the greater alluvial channel area. Secondary channels are not flooded as frequently as the dominant channels.	
	Rock in Primary and Secondary Alluvial Channels	AIR	This unit is used to highlight where rock is encountered in dominant/main channels, as well as within secondary channels and rarely in proximal floodplains. The deposit comprises localised areas of exposed duricrust, bedrock and/or continuous rock exposures. It may form high points in the channels or be associated with permanent to semi-permanent deep rock pools. The rock is exposed under high velocity flow with limited sediment deposition.	
	Proximal Floodplain Alluvial Deposits	Fp	Floodplain deposits border the dominant channel(s) along the main flow path and are formed by variable, but mostly lower velocity flow areas on the outskirts of primary and secondary channels during seasonal floods. The deposits include loose silt, sand and gravel lag. The latter mainly occurs in areas of higher energy flow. The deposits include weakly consolidated silt, clay and sand with occasional areas of gravel content. The material often contains organic matter such as woody debris. Proximal floodplain is often highly vegetated compared to other parts of the alluvial system such as secondary channels.	
	Distal Floodplain Alluvial Deposits	FpD	Fine-grained alluvial deposits of distal floodplain/backplain typically accrue in flat areas distant from dominant channel(s). In some cases it could be affected by distal slope wash processes and grades into Alluvial Wash (Aw). The deposits include weakly consolidated silt, clay and sand with gravel lag. In general, FpD deposits are formed under lesser flow or / and under stagnation conditions, and flooding of these areas is not as frequent as the proximal floodplain.	
	Distal Slope Wash (Alluvial Wash)	Aw	Fine grained, wash deposits, which generally occur on relatively level to gently sloping areas dominated by sheetflow parallel to the main local channels. The unit comprises unconsolidated to medium dense silts, clays, sands and gravels. The surface is typically silt and silty sand dominated with local gravel lag. Alluvial wash areas tend to be broad and lack a defined alluvial channel.	Qw

**TABLE 1: GENERALISED STRATIGRAPHIC COLUMN**

Geological Time / Supergroup / Group	EGU <sup>1</sup> Name	EGU <sup>2</sup> Code	Description <sup>3</sup>	Reference to WA Geological Survey Map (SF50-12)
Quaternary	Colluvial Slope Wash Deposit (Colluvial Wash)	Cw	Unconsolidated to medium dense clay, sand and gravel mixtures with variable occurrence of cobbles. Down slope colluvial deposits, typically with increased fines content relative to Colluvial Gravel (Cg), often grading into Alluvial Wash (Aw). Localised iron cementation may occur.  Colluvial Wash is typically located in the lower to mid parts of the colluvial slope, dominantly formed by sheet wash down-slope processes.	Qw
	Proximal Slope Gravel Deposits (Colluvial Gravel)	Cg	Loose to cohesive deposits formed from a combination of gravity and water flow (sheetwash down slope). The deposits are sourced from adjacent hills and ridges, and typically occur on intermediate parts of the slope. The characteristics of the colluvial materials are heavily dependent on the nature of the parent rock, but typically comprise sandy clayey gravels, often with cobbles, and sometimes with a degree of cementation at depth.	Qc
Tertiary	Ancient Alluvium	Aa	Partially consolidated alluvial deposits forming elevated gravelly hills overlying calcrete deposits.	Czk
	Calcrete Soil	Calcrete_Soil	Unconsolidated weathered calcrete comprising calcareous gravels, sands and silts with minor pisolitic gravels. Typically a brown cream colour	
	Calcrete	Calcrete	Calcrete is sheet carbonate material that mainly comprises gravel and sand cemented together by calcium carbonate. It is red, pink, brown or white in colour. The material sometimes forms tabular rock and often has semi-continuous voids and cavities. The calcrete typically overlies Robe Pisolite/Channel Iron Deposits with variable thickness of up to 30 metres in some areas. The strength of the unit may vary from low to high depending on weathering degree of the cementation.	
	Robe Pisolite	Tp	Massive pisolitic and oolitic goethite and haematite ironstone deposits (CID). The materials are iron-cemented, pisolitic limonite and haematite deposits, with minor scattered fossil wood. The unit may contain occasional thin clay layers. Where exposed at the surface it can be distinctly weathered, and exhibit low to high strength.	Czp
	Goethitic Conglomerate	ConG	Goethitic conglomeritic rock typically exposed in the creek margins, underlying calcrete deposits. The unit may be similar to basal CID units but appear poorly developed and may represent the weathered surface from which the CID is derived. Upper exposed and weathered surfaces of the goethitic conglomerate present as a ferruginous cap, which whilst not dissimilar in appearance to CID, is typically less than one metre thick and quickly transitions to the more lateritic appearing goethitic conglomerate.	Czp

TABLE 1: GENERALISED STRATIGRAPHIC COLUMN				
Geological Time / Supergroup / Group	EGU <sup>1</sup> Name	EGU <sup>2</sup> Code	Description <sup>3</sup>	Reference to WA Geological Survey Map (SF50-12)
Mount Bruce Super Group / Hamersley Group	Weeli Wollli Formation	BIF	Banded iron-formation, commonly jaspilitic, encountered predominantly within the eastern half of the area of interest. Typically appears as cuesta-landforms and massive hills. Due to several dominant defects sets the unit frequently exhibits a poor rock mass quality. Joints are commonly open near surface and infilled with clay, gravel and rootlets;	Phj
		Shaly BIF	Dominantly Pelite overlain by Chert, encountered predominantly within the eastern half of the area of interest. The unit forms the lower parts of cuesta-landforms and massive hills, with low angle slopes. It typically is of medium strength and subject to preferential weathering and erosion.	
		Dolerite	Metadolerite sills interbedded with Shaly BIF, encountered predominantly within the eastern half of the area of interest. Rock strength may vary from very low strength in extremely weathered forms, to high strength in fresh to distinctly weathered materials.	
	Brockman Formation	BIF	The Brockman Formation BIF rock unit is encountered in the far western part of the area of interest. It comprises banded iron-formation, chert, and pelite. The Brockman Formation BIF may form high rounded hills, with drainage gullies and gorges characterised by near vertical cliff edges formed parallel to pervasive jointing, often with unstable blocks / pillars of rock along the edges. Large blocks, which have fallen from the cliffs, are common at the base of slopes (tallus).	Phb

<sup>1</sup> EGU – Engineering Geological Unit

<sup>2</sup> EGU codes listed in Table 1 are consistent with Engineering Geological Maps provided in Appendix A and Appendix B.

<sup>3</sup> The report N RR17 has been used as the reference for description of alluvial units in Table 1. (Ref 2)

## 2.3 Geomorphology

### 2.3.1 Terrain Geomorphology

The Marillana Creek drainage corridor crosses three main geomorphological areas (shown on Figure 1), which have distinct relationships between lithology, surficial materials and topography:

- Western Part – from near the Great Northern Highway where the channel first becomes incised to where several other tributary channels join and the channel flows in more of an easterly direction. The western part is surrounded by low calcrete rises and broad colluvial wash areas with localised calcrete and BIF. The Western Part river channel is dominated by high hydraulic conductivity sandy gravels with frequent grassy low flow channels;
- Central part – from where the significant tributaries join then easterly downstream to a point just upstream from the “Flat Rocks” area. The central part is underlain by calcrete with localised Dolerite and BIF. These rocks control the terrain for this part of the corridor and commonly present as a relatively low elevation erosional surface continuously underlain by calcrete, Dolerite or BIF. The area is dominated by Calcrete capped CID to the north, and weathered Weeli Wolli Formation to the south. The Central Part river channel is typically broad, frequently braided and dominated by high hydraulic conductivity sandy gravels, frequently with cobbles, regular exposures of goethitic conglomerate and BIF with some dolerite, grassy low flow sections and regular gum trees; and
- Eastern part – downstream and surrounding the “Flat Rocks” area, including the outskirts of the BHP Yandi Mine developments. The eastern part of the area of interest around ‘Flat Rocks’ is dominated by BIF, Shaley BIF and hills of Dolerite. Locally, the old valley infill caps Shaly BIF and forms low, standalone table hills. Here, Marillana Creek and its tributaries cross the dolerite units exposing rock along and within the channels, resulting in damming to form near permanent water pools. The Eastern Part river channel is dominated by high hydraulic conductivity cobbly gravels, shallow BIF and dolerite with patchy gravels and silty clay deposits, frequent to dense gum trees and paper barks.

Further to the east, through the BHP Yandi mine areas, the terrain comprises low flat areas underlain by Robe Pisolite surrounded by moderate to low sharp hills comprising BIF/Shaley BIF and rounded dolerite hills along the periphery areas of Yandi mine developments. The hills are dissected by incised drainage channels of various depths. The majority of the Marillana Creek within the eastern part of the corridor runs through the Yandi mine developments, where the natural flow is intercepted and deviated by cut-off and deviation channels as part of the mine development.

### 2.3.2 Channel Geomorphology

Creek flow throughout the area is sporadic and influenced by the semi-arid Pilbara climate. Rainfall amounts and intensities are highly variable and irregular due to periodic cyclonic events, thunderstorms and localised intense rainfall. Average annual rainfall varies from 300 to 400 mm. High rainfall events can occur in a short period of time, and over a relatively small area, leading to localised flooding along affected watercourses. Such events can lead to high velocity flows concentrating in dominantly seasonal channels of Marillana Creek and its main tributaries.

Vegetation cover through the area of interest generally comprises spinifex grasses and shrubs in colluvial and wash areas, with “snappy gum” trees growing on elevated areas of rock. White gums typically occur along significant drainage lines, with shrubby and grassy vegetation covering floodplain areas.

The primary channel as mapped does not follow what might be considered a typical river morphology whereby surface flow capacity increases downstream as various tributaries enter the main channel. Rather, it is typically observed that the entering tributaries frequently provide a significant boost to apparent flow which then dissipates and the channel appears to return to normal flow conditions. However, normal flow conditions for the wetting front are highly variable along the creek.

The creek bed frequently switches between open gravelly channels and heavily vegetated irregular channels. It is assessed that the variability in channel morphology can be attributed to two primary factors; depth to rock and isolated nature of the rainfall. Depth to rock is generally expected to be in the order of no more than 1 to 4 metres, and typically 1 to 2 m, although the thickness of gravels is highly dependent on channel position. Local hollows ~2m deeper than surrounding gravels were seen in several locations, and these typically contained calcrete or other rock exposures in the base, but were often associated with local channel narrowing and sharp channel direction changes, so the differential is exacerbated by the creation of a point bar.

Inconsistencies in creek morphology observed in the field, whereby an entering channel would frequently appear to carry more water than the main channel, is attributed to typical Pilbara rainfall events, whereby rainfall events are isolated to parts of the drainage catchment such that high rainfall may fall on one tributary to the primary channel, but not the primary channel itself. It is frequently observed on project sites that one area will see significant rainfall and water flow where nearby areas receive no rainfall. These events would cause “flash flooding” of a tributary or tributaries with an initial unsustainable surcharge of water flowing into the creek system which then dissipates into small flows in the low flow channels and/or subsurface flow through the river gravels.

It is expected that the only time the entire creek system would likely see rainfall in the same period would be during cyclonic events where widespread rainfall is common and as such the full creek system would flow. During such events, surface water flow would likely cover the full primary channel as well as extending into secondary channels and floodplains during peak flow, before eventually reducing to flow through low flow channels and then to subsurface flow through gravels.

The broader primary channel is considered to be erosional, with older terrace alluvial deposits frequently seen in the river banks. For this reason the creek bed is dominated by gravels with ~20-30% gravels and little to no fines, whereas the terrace alluvial banks are typically quite silty with coarser gravel bands.

The primary flow channel identified through mapping as presented in the Engineering Geological Maps (Appendices A and B) is typically characterised by a low flow channel within the broader higher flow primary channel. The domains and chainages as presented in the Domain Maps (Appendix C) and the Wetting Front Design Table (Appendix D) are based on this primary flow channel. It is assessed that this primary flow channel is where dewatering water pumped into the creek system will most likely be exposed surficially when the water volume exceeds the capacity of the creek gravels. For modelling purposes, it is expected that an average depth of gravels of 1m below the base of the primary / low flow channel would be a reasonable assumption, with the obvious exception of AIR areas where duricrust or bedrock is exposed in the base of the channel. Sub surface water flow through the river bed gravels could be expected to be carried for the full width of the primary channel, and potentially also extending below the secondary channel.

The creek channel has been broken down into typical morphological “types” for modelling purposes, consisting of the following:

- Vee (Plate 18)– A standard Vee shaped incised channel, relatively narrow with central flow;
- Trapezoidal (Plate 19) – A typical creek shape with relatively steep, well defined banks on both sides and a relatively flat base, low flow channels tend to meander from one side to the other due to development of minor lateral bars;

- Trapezoidal Convex (Plate 20) - A typical creek shape with relatively steep, well defined banks on both sides and a convex base, highest at the centre. low flow channels tend to be present on both sides of the channel;
- Wedge (Plate 21) – Typically occurring on bends in the creek the outer edge is typically steep and erosional with a point bar on the inside forming a gradual shallow bank. The low flow channel is along the steep side however the point bar gravels are loose and permeable;
- Undulating hollowed channel (Plate 22) – Typically occurs where trees are present in the channel forming local hollows with resultant downstream bars causing variability in the low flow channel position;
- Highly vegetated irregular (Plate 23) – Frequent grasses form small bars creating a highly braided channel with poorly defined channels. Water flow through these areas is expected to be predominantly subsurface through gravels, or very shallow at surface. These channels tend to be broad and relatively flat and are often associated with shallow rock and may indicate groundwater connection;
- Rising transition zone (Plate 24) – These sections of the creek channel are distinct in that the creek bed is at a higher elevation downstream than upstream. There is typically a loss of definition for the low flow channel suggesting increased subsurface flows through loose gravels;
- Bowls (Plate 25) – Gentle bowl shaped local lows (typically less than 50 metres) in the channel, typically lacking vegetation in the bed with a relatively regular base, expected to form pools of exposed flow when flow is mostly through the gravel base of surrounding channel areas; and
- Braided zone (Plate 26) – Typically associated with shallow rock and a high water table with frequent trees, channels are highly variable with shallow hollows and resultant bars.

The morphological types are a primary differentiator for the channel domains as described in Section 2.4.

## 2.4 Channel Domains

To distil mapping information collected in the field into a form that can be used as an input for infiltration and wetting front modelling, a Channel Domain Design Table has been created from the mapping data. The table, presented in Appendix D, references chainages generated to coincide with the defined primary channel centreline and is presented visually in the maps in Appendix C.

## 2.5 Duricrust and Bedrock Exposures / Channel intersections

Duricrust and bedrock exposures in the channel varied between small exposures in the base of hollows (Plate 27) or the edge of wedge channels (Plate 28); limited bouldery outcrops (Plate 29); or broad dominant outcrops across the full width of the channel (Plate 30), with the latter predominantly found towards the eastern end of the mapped area (Flat rocks).

Where duricrust and bedrock was observed during the mapping, this has been noted within the Channel Domain Design Table in Appendix D.

## 2.6 Calcite Precipitation

Limited calcite precipitation was observed during the mapping, and it is considered that this would likely be due to the short term, high velocity nature of normal flows in the creek. Where observed, calcite precipitation is noted in the Domain Design Table. In addition, occurrences of hollows indicative of turbulent flow which might facilitate calcite precipitation are noted in the Domain Design Table.

A black precipitate was noted to coat rocks in areas of pooling and low flow (Plate 31) in the eastern parts of the central domain particularly once the wetting front channel joined Marillana Creek

### 3.0 GROUND BREAKING INVESTIGATIONS

Ground breaking investigations in Marillana Creek and the Tributary that will contain the wetting front were conducted between July 5 and July 11, 2019 and consisted of *in-situ* permeability testing, Panda DCP testing and disturbed material sampling. Eleven potential investigation sites were selected and numbered in order of priority. It was targeted to investigate a minimum of five sites and of the eleven selected potential sites, eight were investigated. At two of the sites, local variability lent itself to performing two *in-situ* permeability tests. Field *in-situ* permeability tests were conducted at ten locations, and nine Panda DCP profiles were conducted. Disturbed bulk samples for laboratory testing were taken at each permeability test location.

#### 3.1 Methodology

#### 3.2 In-Situ Permeability Testing

*In-situ* permeability testing was conducted using a Guelph Permeameter. Due to the high permeability of the soil, drawdown of the stored water within the permeameter was extremely fast which made obtaining consistent readings difficult. Where consistent readings could not be obtained, multiple tests were conducted at varying heads, and relatively consistent drawdown periods selected based on the drawdown graph and averaged. Video was used to accurately record reservoir levels where drawdown was rapid.

Guelph Permeameter test records are presented in Appendix F and summarised in Appendix E with the laboratory test results.

#### 3.3 Panda DCP Testing

Panda DCP testing was conducted at several locations across the channel at each permeability test location. The goal of the Panda DCP testing was to provide insight to both the depth to rock, and the variability in creek bed materials, such as differing sandy and gravelly layers, and their relative density. In two locations where there was a vertical exposure of terrace alluvials, Panda DCP's were undertaken on the bank to allow correlation with exposed variability.

The Panda DCP is an instrumented variable energy penetrometer which reads cone resistance for each blow of a specially weighted hammer. The Panda DCP gives a precise penetration depth for each blow and the blow force can be varied by the operator to suit conditions. That is, where soils are soft, less force can be used to get a higher resolution of ground density.

Panda DCP logs are presented in Appendix G. Results can be interpreted as follows with respect to cone resistance (MPa) values:

- 0 - 7 MPa – very loose surface materials or loose to medium dense silty sand layers;
- 7 - 20 MPa – Sandy Gravels, loose to medium dense;
- 20 - 40 MPa - Higher ratio of gravels to sand, medium to coarse medium dense to dense gravels;
- >40 MPa – Weakly cemented sandy gravels, or extremely weathered rock; and
- >70 MPa – Rock or well cemented gravels.

A Panda DCP “Traverse” was conducted at each permeability test location to assist with understanding the subsurface creekbed profile across the primary channel. These traverses are visualised as cross sections presented in Appendix J. The creek bed surface profile for these traverses is generated from LIDAR and contour data provided by BHP. Due to the low

resolution of this data, the creek bed profile does not accurately represent the actual observed creek bed.

### 3.4 Material Sampling and Laboratory Testing

Bulk samples were taken by hand excavation using a shovel from locations in the immediate vicinity of field permeability testing. The samples were taken from between 0.05 and 0.25 metres depth. Samples were sent to soils laboratories for characterisation of the material properties (particle size distribution, natural moisture content and compaction) and for permeability testing.

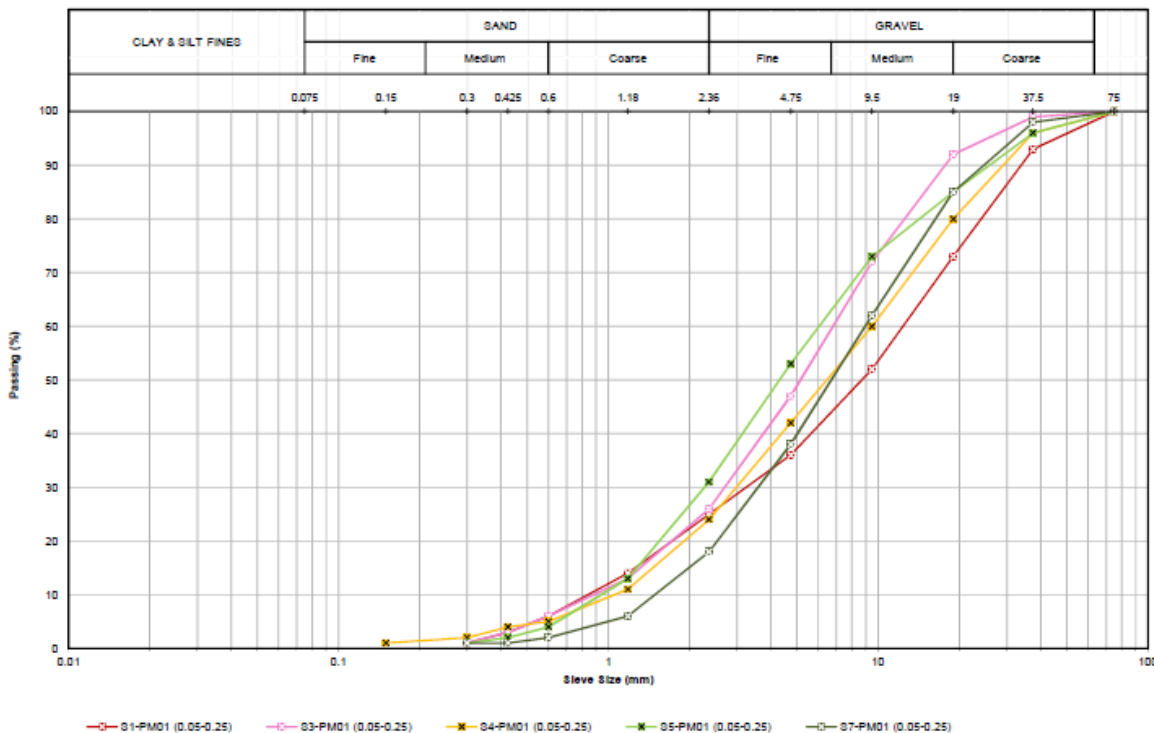
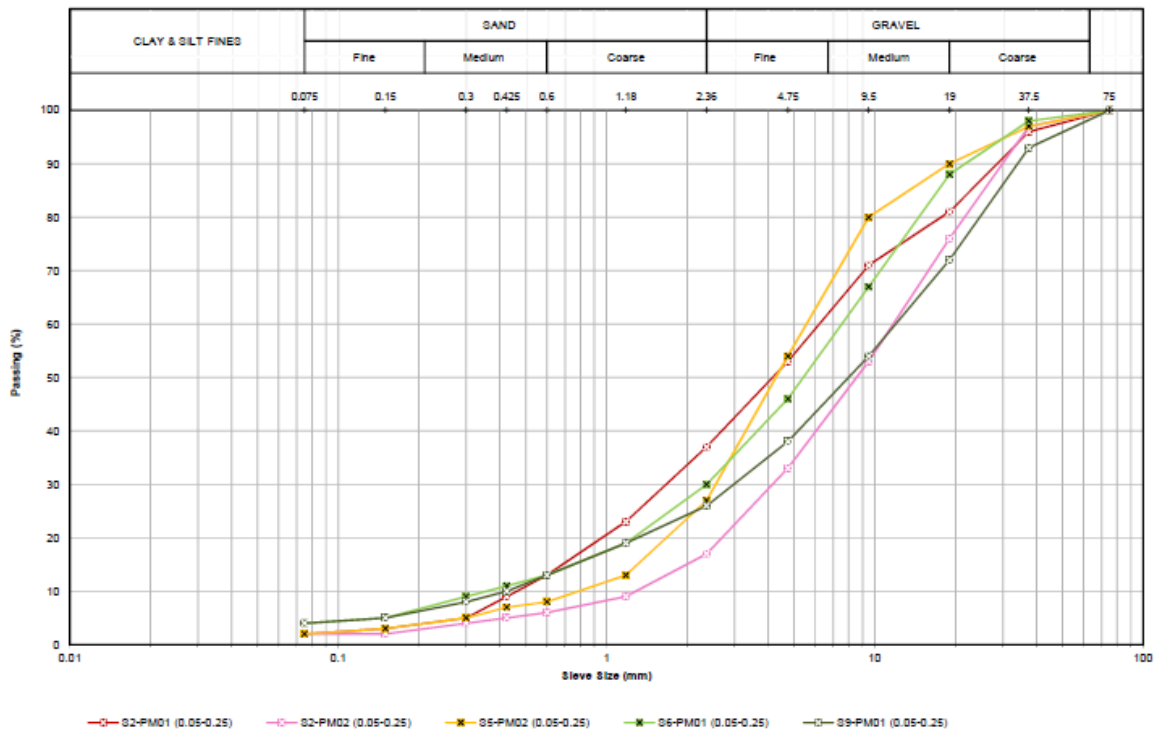
Table 2 summarises the samples taken and testing scheduled. Results are discussed in Section 4.0. Appendix E presents a summary table of test results.

<b>TABLE 2: SCHEDULED LABORATORY TESTING</b>							
Sample ID	Sample Depth	Natural Moisture Content	Particle Size Distribution	Maximum Dry Density	Calcium Carbonate Content	Soil Void Ratio	Constant Head Permeability
S1-PM01	0.05 - 0.025m		1	1	1	2	1
S2-PM01	0.05 - 0.025m	1	1	1		2	1
S2-PM02	0.05 - 0.025m		1	1		2	1
S3-PM01	0.05 - 0.025m		1	1		2	1
S4-PM01	0.05 - 0.025m		1	1		2	1
S5-PM01	0.05 - 0.025m		1	1		2	1
S5-PM02	0.05 - 0.025m		1	1	1	2	1
S6-PM01	0.05 - 0.025m		1	1		2	1
S7-PM01	0.05 - 0.025m		1	1		2	1
S9-PM01	0.05 - 0.025m		1	1		2	1
<b>Total Tests</b>		<b>1</b>	<b>10</b>	<b>10</b>	<b>2</b>	<b>20</b>	<b>10</b>

## 4.0 DISCUSSION OF INVESTIGATION RESULTS

### 4.1 Particle Size Distribution

Of the 10 particle size distribution results 5 contained fines while the other 5 did not. The particle size difference between samples is mostly attributable flow conditions, with coarser materials considered to indicate higher surface flow velocities. Fines, in the form of silt, comprised between 0 and 4% of the creek bed material, with sand comprising between 17 and 37%, and gravel between 63 and 82%. Figure 3 below shows the grading curve plots for those samples with fines present. Figure 4 below shows the grading curve plots for those samples with no fines present.



## 4.2 Natural Moisture Content

A single natural moisture content (NMC) test was conducted on sample S2-PM01 as this sample was wet from permeability testing conducted prior to sampling. The purpose of this test was to understand the residual moisture remaining in the soil after a saturation event. The NMC result was relatively low considering the material had been soaked, and reflects the non-plastic nature of the fines and high permeability nature of the material overall.

## 4.3 Permeability

Permeability depends on the particle size of the materials, and to a lesser extent on the void ratio. In the ground, it can vary massively from point to point because of natural inhomogeneity, and be strongly anisotropic (with horizontal  $k_h$  much greater than vertical  $k_v$ ) owing to soil fabric and structure. The potential range of permeability even for uniform soils is very wide and the degree of variation is far greater than for that of almost any other engineering parameter. In ground engineering, it is usual to use a fairly simple model for initial calculations. However, building a reliable picture from small scale tests can be difficult

Field testing was conducted on the upper creek bed materials which were found from the Panda DCP to be typically in a loose to medium dense state. It was established that true saturation could not be achieved during the testing due to the large volume of highly permeable creek bed materials into which the permeameter was releasing its limited supply of water. Whilst local saturation was achieved around the device, true saturation was not considered possible and the results are considered more like unsaturated permeability.

The presence of sand and silt influenced field permeability, with a silty surface typically leading to reduced field permeability. The creek gravels in more vegetated and irregular areas, suggestive of low surface flow, had a tendency towards slightly higher permeability than the more open areas, which is consistent with field observations that suggested flow might be through, rather over the creek bed gravels. More generally, apparent high flow areas had looser surface gravels, leading to higher field permeability which was not reflected as prominently in the laboratory where all samples were compacted to 92% of their maximum dry density.

Laboratory permeability of compacted specimens was typically an order of magnitude lower than field permeability. This is considered to be due to several differential factors between the style and performance of the two tests. The two primary contributing factors to the differences are considered to be compaction (or density of the specimen) and the achieved degree of saturation. Table 3 summarises the permeability results.

Laboratory permeability was conducted on a relatively small volume of fully saturated material, compacted to 92% of its maximum dry density (considered medium dense to dense) which is expected to be similar to the creek bed materials at depth, as indicated by the Panda DCP profiles in Appendix G.

However, it is expected that compaction is the primary reason for the difference between the field and laboratory permeability and as such the field permeability can be used for the materials in the upper 0.2 metres in the base of low flow channels, as well as the adjacent gravels of the broader primary channel, where the creek bed is higher than the low flow channel. It is expected that as the density of the creek bed gravels increases with depth, they will have permeability values similar to the laboratory permeability.

<b>TABLE 3: PERMEABILITY TESTING SUMMARY</b>						
Permeameter Test ID	Chainage (m)	Channel Description	Bed Description	Gravel Depth (m)	Field Permeability (k)	Lab Permeability (k)
S1-PM01	2,250	Highly vegetated irregular	Sandy GRAVEL, cobbly patches	0.6	7.40E-04 m/s	7.44E-05 m/s
S4-PM01	2,725	Undulating Hollowed Channel	GRAVEL with sand and Sandy GRAVEL, trace cobbles	0.7	5.48E-04 m/s	1.13E-04 m/s
S9-PM01	4,165	Highly vegetated irregular	Sandy GRAVEL, trace cobbles	1.2	2.67E-04 m/s	6.78E-05 m/s
S2-PM01	8,100	Highly vegetated irregular	Sandy GRAVEL, trace cobbles	3.6	4.60E-04 m/s	4.91E-05 m/s
S2-PM02	8,150	Wedge	Sandy GRAVEL, trace cobbles, patches of exposed Calcrete	1.2	Field permeability higher than permeameter measuring capability	2.56E-05 m/s
S6-PM01	10,090	Wedge	Sandy GRAVEL, trace cobbles	0.4	1.04E-05 m/s	4.51E-05 m/s
S7-PM01	13,660	Broad and shallow flowing	Sandy GRAVEL	0.9	6.48E-04 m/s	3.43E-05 m/s
S3-PM01	17,840	Highly vegetated irregular	Sandy GRAVEL, trace cobbles and Cobbly GRAVEL, with sand	1.2	6.76E-04 m/s	5.41E-05 m/s
S5-PM01	26,030	Irregular flat	Sandy GRAVEL over rock, cobbles	0.9	4.97E-04 m/s	6.11E-05 m/s
S5-PM02	26,030	Irregular flat (low flow)	Silty Sandy GRAVEL over rock	0.25	1.42E-05 m/s	3.92E-05 m/s

For modelling purposes it could be assumed that field permeability values can be applied to the upper 0.2 to 0.5 m of the creek bed and for short-term permeability as the wetting front passes down the creek. However, once full saturation of the gravel bed occurs after the initial wetting, it may be more appropriate to apply the laboratory permeability to the full creek bed depth.

### 4.3.1 Soil Void Ratio and Water Capacity

To better understand the water holding capacity of the alluvial gravels in the creek bed, a test was devised to allow direct measurement of the void space / latent water capacity. The goal of this test was to determine how much water the gravels can be expected to 'absorb' before the permeability is expected to reach a steady flow, saturated condition. Material was compacted at two differing moisture contents to achieve differing levels of compaction to better understand the influence of compaction on void space, any changes to which are then inferred to have the potential to similarly influence permeability.

Following compaction, water was added to the compacted sample to fill the available void space. This volume was measured and used to calculate the available void space which has been filled. Based on the results it appears that void space within the gravels is significantly influenced by density, and density increase can be expected to reduce void space, and in turn hydraulic conductivity. This is in line with the differences between field and laboratory permeability.

**TABLE 4: SOIL VOID RATIO RESULTS**

Sample ID	Initial Moisture Content (%)	Moisture Added for Compaction (%)	Optimum Moisture Content (%)	Achieved MDD Ratio (%)	Water added to fill voids (g)	Approximate Void Ratio (%)
S1-PM01	2	8.6	10.6	101	25	2
	2	4.6	10.6	97	115	11
S2-PM01	1.3	5.2	6.5	99	49	5
	1.3	3.9	6.5	95	130	13
S2-PM02	1.8	6.6	8.4	102	22	2
	1.8	2.6	8.4	95	148	15
S3-PM01	2.9	6.2	9.1	99	44	4
	2.9	2.2	9.1	96	113	11
S4-PM01	2	8.8	10.8	99	14	1
	2	4.8	10.8	93	123	12
S5-PM01	0.8	6.9	7.7	100	102	10
	0.8	2.9	7.7	96	174	17
S5-PM02	2.5	5.5	8.0	101	28	3
	2.5	1.5	8.0	97	66	7
S6-PM01	2.6	6.8	9.4	99	6	1
	2.6	2.8	9.4	94	56	6

<b>TABLE 4: SOIL VOID RATIO RESULTS</b>						
Sample ID	Initial Moisture Content (%)	Moisture Added for Compaction (%)	Optimum Moisture Content (%)	Achieved MDD Ratio (%)	Water added to fill voids (g)	Approximate Void Ratio (%)
S7-PM01	2.9	7.1	10.0	101	69	7
	2.9	3.1	10.0	99	153	15
S9-PM01	1.9	5.9	7.8	100	17	2
	1.9	1.9	7.8	93	86	9

## 5.0 CONCLUSIONS AND OUTPUTS

The field mapping and subsequent limited ground breaking information have provided insight into the variability of the creek along the potential wetting front. It is apparent that significant influences on the water holding and carrying capacity of the Marillana Creek and its tributaries include:

- The relatively shallow and variable depth of the alluvial deposits;
- The presence of local rock bars where it is expected that the water will be forced to the surface to flow visibly across the ground. In hot times, this will allow some evaporation to occur;
- The potential for changes in vegetation and wildlife in areas such as the rising transition zone, where it is expected that surface water ponding on a semi-permanent basis may occur; and
- The limited permeability of the duricrust and bedrock units, which means that no significant losses should be expected from infiltration into the floor beneath the alluvial materials.

Significant issues beyond the scope of this data collection and assessment study include:

- Understanding the impact of flooding related to natural rainfall or cyclonic events on the system when the alluvial materials are already saturated from the dewatering pumping;
- Understanding the impact of the arrival of the wetting front should it reach the BHP Yandi minesite, and the mine's ability to deal with the on-going flows.

The following digital data will be provided to BHP as an accompaniment to this report:

- The Channel Domain Design Table as an Excel file;
- A primary centreline polyline file in ESRI Shapefile format incorporating the domain design table as attribute data;
- Structural measurement data in Excel format for geological information collected during the mapping for incorporation into BHP's structural database;
- Georeferenced photographs from the field mapping; and
- Engineering geological interpretation of the Area of Interest in ESRI Shapefile format.

## 6.0 RECOMMENDATIONS FOR FURTHER WORK

Creating a profile along the identified primary channel as presented in the design table based on currently available data produces a saw tooth profile due to the coarseness of the elevation data which limits the vertical accuracy of the creek bed along the wetting front. In the field it was frequently observed that the creek bed would rise in parts, sometimes by as much as one to two metres which may or may not have a corresponding rise in underlying rock. It is recommended that high resolution LIDAR data be captured for the channel to better understand the profile of the creek bed and the potential for local pooling of water.

## 7.0 CLOSURE

The findings of this report are based on a desk study, field mapping observations, and limited subsurface investigations, as well as interpretation and opinion incorporating experience from previous projects undertaken in the Pilbara region. Since only limited ground-breaking geotechnical investigations have been undertaken, the inherent uncertainty in the findings of this report should be recognised, however within the limitations 4DG believe the information provided should be sufficient to increase accuracy of the wetting front modelling. It will be essential to review and revise the outputs should detailed LIDAR information become available.

## REFERENCES

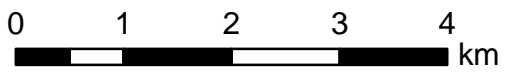
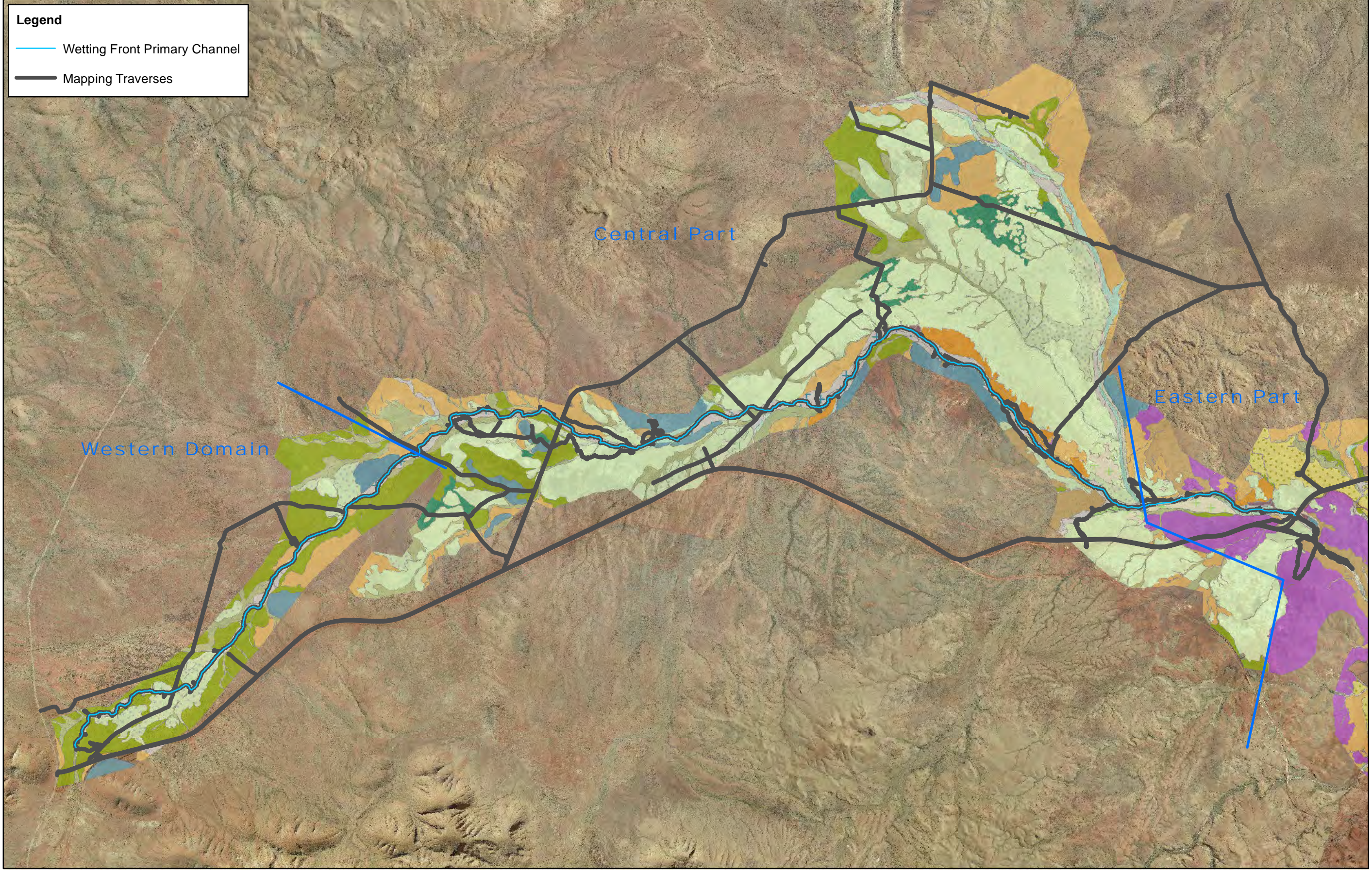
1. Thorne, A. M., and Tyler, I.M., 1996, Roy Hill, W.A. Sheet SF 50-12 (2<sup>nd</sup> edition): Western Australia Geological Survey, 1:250 000 Geological Series.
2. Water and Rivers Commission (2002) "Recognising Channels and Floodplains Forms". Water and Rivers Commission River Restoration Report N RR17, Perth, WA.

## FIGURES

V:\03 - Jobs\Active\045500 Marillana Creek Infiltration\05\_GIS\_Data\01\_ArcGIS\WORKING\01M\_045500\_Figure 1\_20190814.mxd - Thursday, 14 November 2019 - 1:08:48 PM

**Legend**

- Wetting Front Primary Channel
- Mapping Traverses



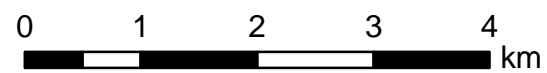
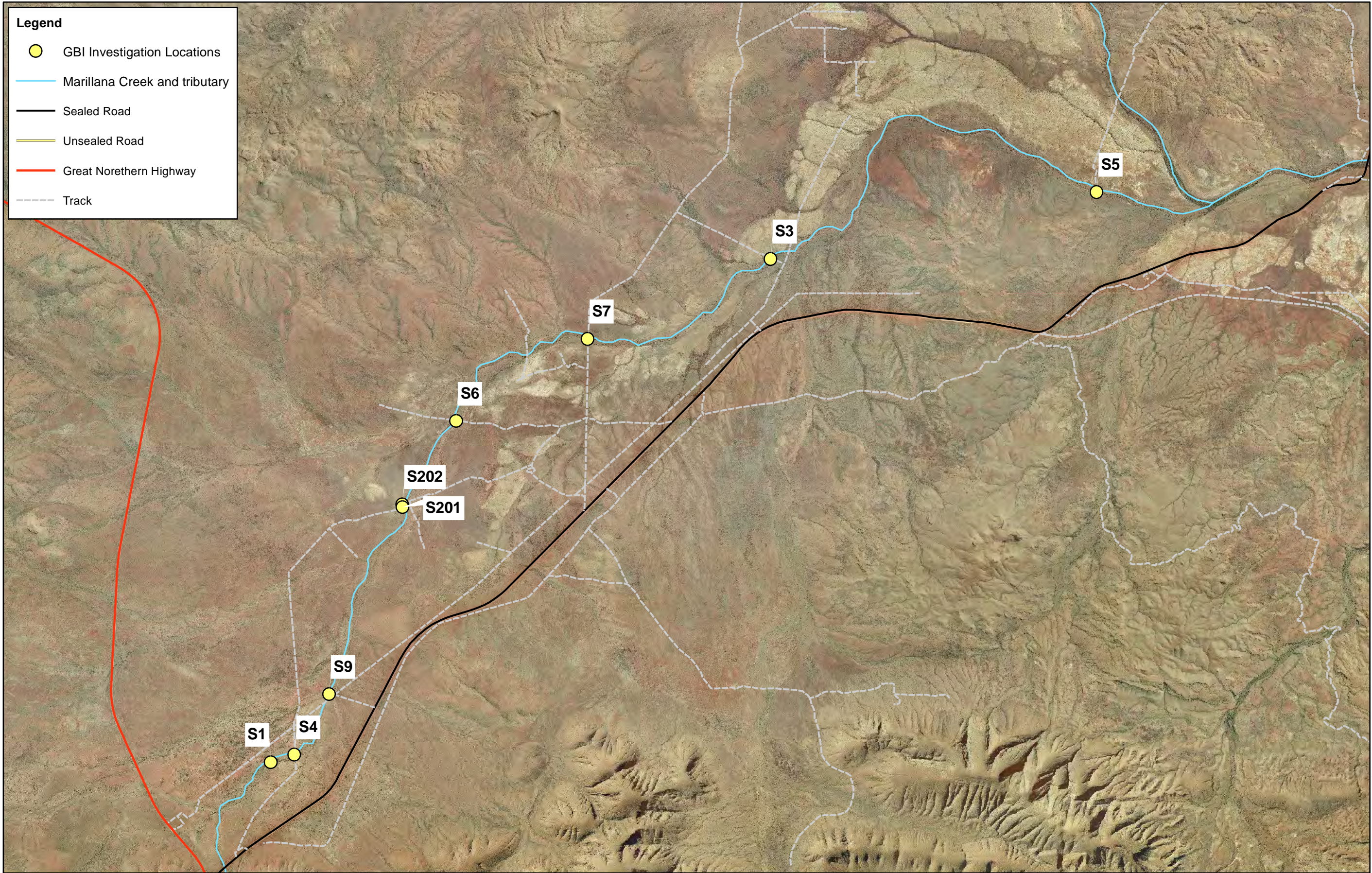
**Notes:**  
 1. Coordinate System: GDA 94 MGA Zone 50.  
 2. NOT FOR CONSTRUCTION. Illustration purposes only.



**Marillana Creek Infiltration Project**  
**Wetting Front Mapping**  
*Mapping Overview*

DRAWN	CNH	DATE	14/11/2019
CHECKED	VBJ	DATE	14/11/2019
SCALE	1:70,000	FIGURE No.	1
PROJECT No.	4DG045500		A3

V:\03 - JobsActive\045500 Marillana Creek Infiltration\05\_GIS\_Data\01\_ArcGIS\WORKING\01M\_045500\_Figure 2\_20190814.mxd - Thursday, 14 November 2019 - 1:10:58 PM



**Notes:**  
 1. Coordinate System: GDA 94 MGA Zone 50.  
 2. NOT FOR CONSTRUCTION. Illustration purposes only.



**Marillana Creek Infiltration Project**  
**Ground Breaking Investigations**  
*Investigation Location Overview*

DRAWN	CNH	DATE	14/11/2019
CHECKED	VBJ	DATE	14/11/2019
SCALE	1:65,000	FIGURE No.	2
PROJECT No.	4DG045500		A3

## PLATES

**APPENDIX A**  
**AREA OF INTEREST - ENGINEERING GEOLOGICAL MAPS (1:15,000)**

**APPENDIX B**  
**WETTING FRONT - ENGINEERING GEOLOGICAL MAPS (1:5,000)**

**APPENDIX C**  
**WETTING FRONT DOMAIN MAPS (1:5,000)**

**APPENDIX D**  
**WETTING FRONT DESIGN TABLE**

**TABLE D-1 - WETTING FRONT DESIGN TABLE**

Channel Change		Channel Physiology	Primary Channel Width		Primary Max Depth (m)	Low Flow Channel Location within Primary	Estimated max bank distance (m)	Secondary Channel Location	Entering drainage(s)		Dominant Creek Bed Material Description	Estimated Hydraulic Conductivity (High >10 <sup>-10</sup> m <sup>2</sup> /s, Moderate 10 <sup>-10</sup> - 10 <sup>-12</sup> m <sup>2</sup> /s, Low <10 <sup>-12</sup> m <sup>2</sup> /s)	Occurrence of Hollows	Calcrete Precipitations	Rock Outcropping	Dominant Rock Unit	Vegetation Within Primary Channel
to (m)	from (m)		Min (m)	Max (m)					Entering from	Size							
0	20	Vegetation	2	4	0.5		Mid Channel				Gravelly silt SAND, trace cobbles	Moderate					Patches of grasses and dense spinifex, patches of shrubs and occasional trees (<10m)
20	50	Undulating Hollowed Channel	3	5	0.5		Mid Channel				Gravelly silt SAND, trace cobbles	Moderate					Patches of grasses and dense spinifex, patches of shrubs and occasional trees (<10m)
50	80	Vegetation	2	4	0.5		Mid Channel				GRAVEL, with sand	High					Patches of grasses and dense spinifex, patches of shrubs and occasional trees (<10m)
80	600	Trapezoidal	3	12	1.5		Mid Channel				Sandy GRAVEL trace cobbles, Gravel and cobble lag, Low flow gravelly SAND	High					Patches of grasses and dense spinifex, patches of shrubs and occasional trees (<10m)
600	660	South bank	6	12	1.5		South bank		South	Minor	Sandy GRAVEL trace cobbles, Gravel and cobble lag, Low flow gravelly SAND	High	Yes	Possible Minor accretions			Scattered grasses
660	910	Highly vegetated irregular	8	20	0.5		South bank		North	Moderate	Sandy GRAVEL, with patches of exposed Ferricrete	Low					Ferricrete Moderate to dense grasses, occasional trees (<10m)
910	940	Trapezoidal	3	5	1		Mid Channel				Sandy GRAVEL	High					Weathered Calcrete Moderate to dense grasses
940	985	Vegetation	4	8	0.5		North bank				Sandy GRAVEL	High					Moderate to dense grasses
985	1,070	Undulating Hollowed Channel	6	6	1.5		Variable				Sandy GRAVEL	High	Yes				Scattered grasses
1,070	1,440	Highly vegetated irregular	3	12	1		Undefined				Sandy GRAVEL, patches of Clayey Silt SAND	High	Yes				Weathered Calcrete Moderate grasses becoming dense
1,440	1,840	Highly vegetated irregular	10	20	0.5		Undefined				Silty SAND, Gravelly SAND, and Sandy GRAVEL	High					Dense grasses
1,840	1,920	Trapezoidal	5	7	0.5		Undefined				Sandy GRAVEL	High					Dense grasses
1,920	1,980	Highly vegetated irregular	5	10	0.5		Undefined				Sandy GRAVEL	High					Dense grasses
1,980	2,080	Vegetation	8	20	1.5		North bank				Cobbly GRAVEL with sand, trace boulders	High					Weathered Calcrete Dense grasses
2,080	2,120	Highly vegetated irregular	15	20	0.5		Undefined				Sandy GRAVEL	High					Weathered Calcrete Dense grasses
2,120	2,160	Undulating Hollowed Channel	4	8	1		Variable				Sandy GRAVEL, cobble patches	High	Yes				Dense grasses
2,160	2,220	Highly vegetated irregular	8	12	0.5		Undefined				Sandy GRAVEL, cobble patches	High					Dense grasses
2,220	2,400	Vegetation	2	8	1		South bank				Sandy GRAVEL	High					Dense grasses
2,400	2,440	Trapezoidal	2	8	1		North and south banks				Sandy GRAVEL	High					Dense grasses
2,440	2,550	Highly vegetated irregular	20	30	0.5		Undefined				Silty SAND with patches of Sandy GRAVEL	Moderate					Weathered Calcrete Dense grasses
2,550	2,600	Vegetation	1	2	0.5		Mid Channel				Sandy GRAVEL	High	Yes				No vegetation Dense grasses
2,600	2,630	Trapezoidal	3	5	1		Mid Channel				Sandy GRAVEL	High					Weathered Calcrete Scattered grasses
2,630	2,690	Highly irregular rock and boulder channel	6	10	1		Undefined				Sandy GRAVEL with patches of exposed Calcrete	Low					Weathered Calcrete Moderate grasses
2,690	2,880	Undulating Hollowed Channel	8	15	2		North bank switching to south bank				GRAVEL with sand and Sandy GRAVEL, trace cobbles	High					Weathered Calcrete Dense grasses
2,880	3,040	Highly vegetated irregular	7	20	1		Undefined				Sandy GRAVEL, trace cobbles	High	Yes				Weathered Calcrete Dense grasses
3,040	3,080	Trapezoidal	8	10	1		North bank				GRAVEL with sand, trace cobbles	High					Weathered Calcrete Moderately spaced grasses
3,080	3,230	Vegetation	8	20	1		South bank				GRAVEL with sand, trace cobbles and Sandy GRAVEL	High					Weathered Calcrete Moderately to densely spaced grasses becoming absent
3,230	3,300	Highly vegetated irregular	15	20	1		Undefined				GRAVEL with sand, trace cobbles and Sandy GRAVEL	High					Weathered Calcrete Moderate grasses
3,300	3,380	Trapezoidal	4	8	0.5		North bank				GRAVEL with sand, trace cobbles and Sandy GRAVEL	High	Yes				Weathered Calcrete Moderate grasses
3,380	3,480	Vegetation	5	10	1		South bank				Sandy GRAVEL, patches of gravelly SAND	High					Sparse grasses Dense grasses
3,480	3,520	Highly vegetated irregular	8	12	0.5		Undefined				Sandy GRAVEL, patches of gravelly SAND	High					Dense grasses Moderate grasses
3,520	3,560	Vegetation	1	3	0.5		Mid Channel				Sandy GRAVEL with trace cobbles	High					Moderate grasses Moderate grasses
3,560	3,620	Undulating Hollowed Channel	4	8	0.5		Variable				Sandy GRAVEL with trace cobbles	High	Yes				Moderate grasses Moderate grasses
3,620	3,720	Highly vegetated irregular	15	25	0.5		Becoming defined on north bank				Sandy GRAVEL	High					Dense grasses Moderate to dense grasses
3,720	3,860	Trapezoidal	8	12	1		South bank				Sandy GRAVEL, trace cobbles	High	Yes				Weathered Calcrete Moderate grasses
3,860	4,040	Undulating Hollowed Channel	10	15	1		Variable				Sandy GRAVEL	High					Moderate to dense grasses Moderate to dense grasses
4,040	4,220	Highly vegetated irregular	15	25	1		South bank				Sandy GRAVEL, trace cobbles	High	Yes				Sparse to dense grasses Dense grasses
4,220	4,780	Broad and vegetated	15	25	0.5		Predominantly north bank				Sandy GRAVEL, trace cobbles, bouldery patches	High	Yes				Dense grasses Goethitic Conglomerate
4,780	4,980	Undulating Hollowed Channel	8	15	1		Variable				Sandy GRAVEL, trace cobbles, bouldery and cobble patches	High	Yes				Weathered Calcrete Dense grasses
4,980	5,230	Vegetation	4	6	1		North bank				Sandy GRAVEL, trace cobbles, bouldery and cobble patches, with patches of exposed Goethitic Conglomerate	Low	Yes				Goethitic Conglomerate Moderate grasses
5,230	5,350	Highly irregular rock and boulder channel	5	8	0.5		Undefined				Goethitic Conglomerate with surficial Sandy GRAVEL, trace cobbles bouldery and cobble patches	Low					Goethitic Conglomerate Moderate grasses
5,350	5,440	Vegetation	8	10	1		South bank				Sandy GRAVEL, trace cobbles, bouldery and cobble patches, patches of exposed Goethitic Conglomerate	Low					Goethitic Conglomerate Moderate grasses
5,440	5,480	Highly vegetated irregular	15	20	1		Undefined				Sandy GRAVEL, trace cobbles, bouldery and cobble patches	High					Dense grasses Moderate grasses
5,480	5,680	Highly irregular rock and boulder channel	7	10	1		Variable				Sandy GRAVEL, trace cobbles, bouldery and cobble patches, patches of exposed Goethitic Conglomerate	Low	Yes				Goethitic Conglomerate, BIF Moderate grasses
5,680	5,750	Trapezoidal	8	12	1		Variable				GRAVEL with sand and cobbles	High					Moderate grasses Moderate grasses
5,750	5,790	Undulating Hollowed Channel	8	12	0.5		Variable				GRAVEL with sand and cobbles, trace boulders	High	Yes				Moderate to dense grasses Moderate grasses
5,790	5,860	Trapezoidal	6	8	1		South bank				Sandy GRAVEL, patches of silt	High					No vegetation Moderate grasses
5,860	5,950	Broad and shallow flowing	15	30	1		Undefined				Sandy GRAVEL, trace cobbles, patches of exposed Goethitic Conglomerate	Low	Yes				Goethitic Conglomerate Moderately to sparsely grasses and scattered trees (<8m)
5,950	6,320	Highly vegetated irregular	25	40	0.5		Undefined				Sandy GRAVEL bouldery patches, patches of exposed Ferruginised BIF	Low	Yes				Ferruginised BIF Dense grasses and scattered large tree (15m)
6,320	6,420	Vegetation	9	11	1		South bank				Sandy GRAVEL bouldery patches, patches of exposed Goethitic Conglomerate	Low					Goethitic Conglomerate Dense grasses and scattered large tree (15m)
6,420	6,520	Trapezoidal	9	11	1		South bank				Sandy GRAVEL, trace cobbles	High					Goethitic Conglomerate Dense grasses and scattered large tree (15m)
6,520	6,620	Vegetation	5	10	1		South bank				Sandy GRAVEL, patches of exposed Goethitic Conglomerate	Low					Dense grasses Goethitic Conglomerate
6,620	6,700	Highly irregular rock and boulder channel	3	5	1		Mid Channel				Goethitic Conglomerate, Ferruginised BIF, with surficial boulders	Low	Yes				Goethitic Conglomerate, Ferruginised BIF Moderate to dense grasses
6,700	6,750	Vegetation	6	12	1		South bank				Sandy GRAVEL, trace cobbles	High					Goethitic Conglomerate, Ferruginised BIF Sparse to moderate grasses
6,750	6,840	Highly vegetated irregular	15	25	1		Undefined				Sandy GRAVEL, patches of exposed Goethitic Conglomerate	Low					Goethitic Conglomerate Dense grasses
6,840	6,860	Incoming drainage depositional zone	10	20	1		South bank switching to north				Sandy GRAVEL, patches of exposed Goethitic Conglomerate	Low					Sparse to moderate grasses Goethitic Conglomerate
6,860	6,940	Vegetation	8	12	1		North bank				Sandy GRAVEL, trace cobbles, small patches of exposed Goethitic Conglomerate	Low					Goethitic Conglomerate Sparse to dense grasses
6,940	7,040	Highly irregular rock and boulder channel	8	12	1		Mid Channel				Sandy GRAVEL, patches of exposed Goethitic Conglomerate	Low					Goethitic Conglomerate Moderate grasses
7,040	7,090	Trapezoidal	10	20	2		Undefined				Sandy GRAVEL, trace boulders, small patches of exposed Goethitic Conglomerate	High					Goethitic Conglomerate / Ferricrete Sparse to moderate grasses and scattered shrubs
7,090	7,140	Vegetation	8	15	0.5		North bank				Sandy GRAVEL	High					Dense grasses Moderate grasses
7,140	7,320	Trapezoidal	15	20	1		North and south banks				Sandy GRAVEL, with cobble and bouldery patches	High	Yes				Sparse to moderate grasses, scattered trees and large tree Moderate grasses and scattered shrubs
7,320	7,560	Highly vegetated irregular	20	30	0.5		Undefined				Sandy GRAVEL, trace cobbles	High					Goethitic Conglomerate, Calcrete Dense grasses, scattered shrubs and occasional large trees
7,560	7,660	Highly vegetated irregular	20	30	0.5		Undefined				Sandy GRAVEL, trace cobbles, with bouldery patches, patches of exposed Calcrete	Low					Goethitic Conglomerate, Calcrete Moderate to scattered grasses
7,660	7,840	Trapezoidal	8	12	0.5		Undefined				Sandy GRAVEL, trace cobbles, patches of exposed Calcrete	High	Yes				Calcrete Moderate to scattered grasses
7,840	7,940	Rising transition zone	15	25	0.5		Undefined				Sandy GRAVEL, trace cobbles	High					Calcrete Dense grasses
7,940	8,100	Vegetation	8	12	1		South bank				Sandy GRAVEL, trace cobbles, with bouldery patches and patches of exposed Calcrete	Low	Yes				Calcrete Moderate to dense grasses
8,100	8,130	Highly vegetated irregular	6	10	0.5		North bank				Sandy GRAVEL, trace cobbles, patches of exposed Calcrete	Low					Dense grasses Moderate to dense grasses
8,130	8,220	Vegetation	8	12	1		North bank				Sandy GRAVEL, trace cobbles	High	Yes				Calcrete / Ferricrete Sparse to moderate grasses
8,220	8,380	Trapezoidal convex	20	30	0.5		North and south banks				Sandy GRAVEL, trace cobbles	High					Dense grasses and scattered shrubs Moderate to sparse grasses, sporadic large trees
8,380	8,560	Trapezoidal	5	10	1		South bank				Sandy GRAVEL	High					Calcrete Moderate to sparse grasses, sporadic large trees
8,560	8,540	Trapezoidal convex	10	20	1		North and south banks				Sandy GRAVEL	High					Calcrete Moderate to sparse grasses, sporadic large trees
8,630	8,890	Vegetation	10	20	1		South bank				Sandy GRAVEL	High	Yes				Calcrete / Goethitic Conglomerate Moderate to sparse grasses
8,890	8,910	Rising transition zone	10	20	0.5		Mid Channel				Sandy GRAVEL	High	Yes				Dense grasses Moderate grasses
8,910	9,070	Vegetation	8	12	1		North bank				Sandy GRAVEL, silt veneer in low flow	High	Yes				Goethitic Conglomerate Moderate grasses, occasional large trees
9,070	9,100	Undulating hollowed channel	12	18	0.5		North bank				Sandy GRAVEL	High					Moderate grasses, occasional large trees Moderate grasses, occasional large trees
9,100	9,180	Trapezoidal convex	12	18	0.5		North and south banks				Sandy GRAVEL	High					Moderate grasses, occasional large trees Moderate grasses, occasional large trees
9,180	9,240	Undulating hollowed channel	10	15	1		Variable				Sandy GRAVEL, patches of exposed Goethitic Conglomerate	Low	Yes				Goethitic Conglomerate Moderate to dense grasses occasional large trees
9,240	9,370	Broad and shallow flowing	25	35	0.5		Undefined				Sandy GRAVEL, patches of exposed BIF	Low	Yes				BIF Moderate to dense grasses
9,370	9,510	Trapezoidal convex	10	15	0.5		North bank				Sandy GRAVEL, trace cobbles	High	Yes				Goethitic Conglomerate Moderate to moderate grasses
9,510	9,600	Broad and shallow flowing	10	15	0.5		North bank				Sandy GRAVEL	High					Goethitic Conglomerate Moderate grasses and scattered large trees
9,600	9,750	Trapezoidal convex	1	5	0.5		North and south banks				Sandy GRAVEL	High					BIF Dense grasses scattered shrubs
9,750	9,810	Undulating hollowed channel	5	15	1		South bank				Sandy GRAVEL with silt	High	Yes				Moderate grasses, occasional shrubs and large trees Moderate grasses, occasional shrubs
9,810	9,895	Broad and shallow flowing	10	15	0.5		South bank				GRAVEL, with sand, trace cobbles	High					Moderate grasses, occasional shrubs Moderate grasses, occasional shrubs
9,895	9,940	Trapezoidal	5	7	1		South bank				Sandy GRAVEL, trace cobbles	High	Yes				Scattered grasses Scattered grasses
9,940	9,980	Vegetation	7	9	1		South bank				Sandy GRAVEL, trace cobbles	High					Scattered grasses Scattered to moderate grasses
9,980	10,070	Vegetation	7	9	1		South bank				Sandy GRAVEL, trace cobbles	High					Scattered grasses Scattered grasses
10,070	10,145	Vegetation	7	9	1		South bank				Sandy GRAVEL, trace cobbles	High					Scattered grasses Scattered grasses
10,145	10,170	Rising transition zone	10	20	0.5		South bank				Sandy GRAVEL, trace cobbles	High					Scattered grasses Scattered grasses
10,170	10,340	Vegetation	7	9	1		North bank				Sandy GRAVEL, trace cobbles, patches of exposed Calcareous Conglomerate	Low	Yes				Calcareous Conglomerate Moderate grasses
10,340	10,370	Rising transition zone	10	15	0.5		Undefined				Sandy GRAVEL, trace cobbles	High					Moderate grasses Moderate grasses
10,370	10,430	Vegetation	4	6	1		North bank				Sandy GRAVEL, trace cobbles	High					Scattered to moderate grasses Scattered to moderate grasses
1																	



**TABLE D-1 - WETTING FRONT DESIGN TABLE**

Channel Chainage		Channel Physiology	Primary Channel Width		Primary Max Depth (m)	Low Flow Channel Location within Primary	Estimated max bank to bank distance	Secondary Channel Location	Entering drainage(s)		Dominant Creek Bed Material Description		Estimated Hydraulic Conductivity (High $>10^{-2} \text{ms}^{-1}$ , Moderate $10^{-2} - 10^{-4} \text{ms}^{-1}$ , Low $<10^{-4} \text{ms}^{-1}$ )	Occurrence of Hollows	Calcrete Precipitations	Rock Outcropping	Dominant Rock Unit	Vegetation Within Primary Channel
to (m)	from (m)		Min (m)	Max (m)					Entering from	Size	Material Description	Simplified Description						
29,940	30,110	Wedge	2	15	1	North bank	100	South bank			Sandy GRAVEL	Sandy GRAVEL	High					Scattered grasses and reed patches
30,110	30,180	Undulating hollowed channel	2	5	0.5	North bank	90	South bank			Sandy GRAVEL, trace cobbles, areas of thick silt veneer	Sandy GRAVEL	High	Yes	Possible minor accretions	CID?, BIF, Calcrete soil		Moderate grasses and shrubs, scattered reed patches
30,180	30,200	Braided	10	20	0.5	Mid channel	100	South bank			Silty sandy GRAVEL	Silty sandy GRAVEL	Low		Minor accretions			Dense grasses and shrubs
30,200	30,290	Swampy bowl	10	15	2	Secondary to south	90	South bank			Silty sandy GRAVEL, patches bouldery	Silty sandy GRAVEL	Low					Dense patches of reeds
30,290	30,350	Rock bowl channels	10	15	2	Secondary to south	100	South bank			Shale with surficial Sandy GRAVEL	Rock with surficial gravels	Low			Shale		Dense patches of reeds
30,350	30,550	Highly irregular rock and boulder channel	5	15	1.5	Variable	110	South bank			BIF with surficial Bouldery Cobble Sandy GRAVEL	Rock with surficial gravels	Low					Dense patches of reeds become absent
30,550	30,590	Braided zones	35	45	0.5	Undefined	110	South bank			Silty cobbly sandy GRAVEL with boulders	Silty cobbly sandy GRAVEL	Moderate					Dense grasses, shrubs, and trees
30,590	30,680	Cobbly bowl	5	15	0.5	Mid channel	120	South bank			Sandy GRAVEL, trace silt with patches of boulders and cobbles	Sandy GRAVEL	High					Dense patches of reeds
30,680	30,860	Braided zones	35	45	0.5	Undefined	130	South bank			Silty GRAVEL	Silty GRAVEL	Moderate					Dense grasses, shrubs and trees
30,860	30,860	Cobbly bowl	5	10	1	Mid channel	130	South bank			Silty Sandy GRAVEL with bouldery patches	Silty sandy GRAVEL	Moderate				BIF	Dense reed patches and scattered grasses
30,860	30,890	Braided zones	25	30	0.5	Undefined	120	South bank			Silty Sandy GRAVEL with bouldery patches	Silty sandy GRAVEL	Moderate					Dense grasses, shrubs and trees
30,890	30,960	Highly irregular rock and boulder channel	25	30	0.5	Undefined	120				CID with surficial Sandy GRAVEL, areas of silt veneer	Rock with surficial gravels	Low		Minor accretions (joint infill)	CID		Moderate shrubs and trees
30,960	31,145	Gentle bowl	15	60	1.5	Mid channel	70	South	Moderate to major		Sandy GRAVEL and Sandy SILT	Sandy GRAVEL	High					Scattered trees and patches of reeds
31,145	31,398	Braided zones	80	100	1	Undefined	170				CID with surficial Silty SAND and patches of Silty Sandy GRAVEL	Rock with surficial gravels	High		Minor accretions (joint infill)	CID		Live and dead large trees, and shrubs on braided bars

**APPENDIX E**  
**SUMMARY OF LABORATORY TEST RESULTS**

**TABLE E-1: SUMMARY OF LABORATORY TEST RESULTS**

Sample ID (Ordered by centreline chainage)	Primary Channel Chainage	Channel type	Sample Depth		Field Saturated Hydraulic Conductivity (m/s)								Laboratory Hydraulic Conductivity (m/s)	Gravel Depth (m)	Composition (Dry)			Compaction Related		Calcium Carbonate Content (%)
					Test A		Test B		Test C		Test D				Percentage (%)			MDD (t/m <sup>3</sup> )	OMC (%)	
			From (m)	To (m)	Head (cm)	Conductivity (m/s)	Head (cm)	Conductivity (m/s)	Head (cm)	Conductivity (m/s)	Head (cm)	Conductivity (m/s)			Gravel	Sand	Fines			
S1-PM01	2,250	Highly vegetated irregular	0.05	0.25	14	5.36E-04	14	5.00E-04	5	8.44E-04	8	7.40E-04	7.44E-05	0.6	75	25	0	2.322	10.6	50.3
S4-PM01	2,725	Undulating Hollowed Channel	0.05	0.25	12	5.48E-04	9	6.69E-04					1.13E-04	0.7	76	24	0	2.42	11.0	
S9-PM01	4,165	Highly vegetated irregular	0.05	0.25	4	2.67E-04	5	3.62E-04					6.78E-05	1.2	70	26	4	2.60	8.0	
S2-PM01	8,100	Highly vegetated irregular	0.05	0.25	14	3.81E-04	14	4.45E-04	8	4.60E-04			4.91E-05	3.6	63	37	2	2.51	6.5	
S2-PM02	8,150	Wedge	0.05	0.25	Not obtainable, permeability exceeded permeameter capabilities								2.56E-05	1.2	81	17	2	2.59	8.5	
S6-PM01	10,090	Wedge	0.05	0.25	7	1.04E-05							4.51E-05	0.4	66	30	4	2.66	9.5	
S7-PM01	13,660	Broad and shallow flowing	0.05	0.25	7	6.76E-04	10	6.48E-04					3.43E-05	0.9	82	18	0	2.25	10.0	
S3-PM01	17,840	Highly vegetated irregular	0.05	0.25	8	6.76E-04	8	6.12E-04					5.41E-05	1.2	74	26	0	2.57	9.0	
S5-PM01	26,030	Irregular flat	0.05	0.25	7	4.97E-04	5	5.38E-04					6.11E-05	0.9	69	31	0	2.33	7.5	
S5-PM02	26,030	Irregular flat (low flow)	0.05	0.25	8	1.48E-05	5	2.19E-05					3.92E-05	0.25	71	27	2	2.60	8.0	59

## APPENDIX F FIELD PERMEABILITY RESULTS





## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	9/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S1-PM01A	
<b>Test Location</b>	Easting	682680	Northing	7475704	RL	NA	

### Subsurface Profile

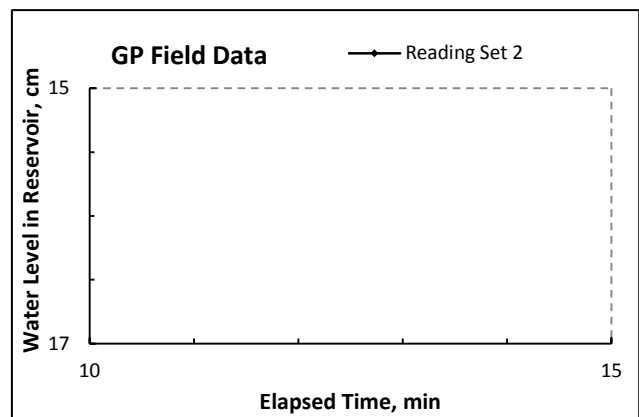
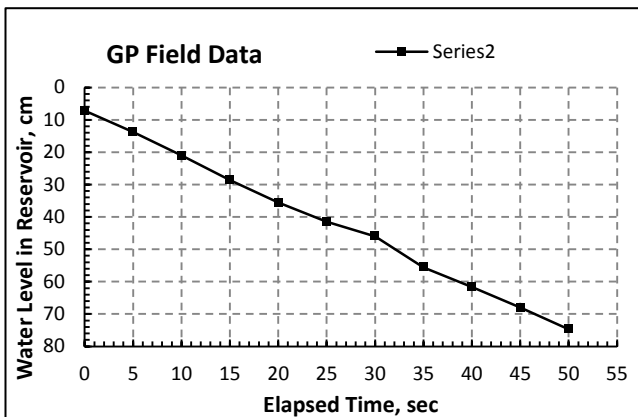
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; orangish brown, loose to medium dense, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse.
	<i>Test conducted in the middle of a trapazoidal primary channel, moderately spaced grasses and slight riffing.</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.597	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.1	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	14	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0.083	7.1	0	0					
5	0.083	13.8	6.7	80.4					
10	0.083	21	7.2	86.4					
15	0.083	28.6	7.6	91.2					
20	0.083	35.5	6.9	82.8					
25	0.083	41.5	6	72					
30	0.083	46	4.5	54					
35	0.083	55.6	9.6	115.2					
40	0.083	61.6	6	72					
45	0.083	68	6.4	76.8					
50	0.083	74.7	6.7	80.4					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				81.12	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 5.00E-02 \text{ cm/s OR } 5.00E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	9/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S1-PM01B	
<b>Test Location</b>	Easting	682680	Northing	7475704	RL	NA	

### Subsurface Profile

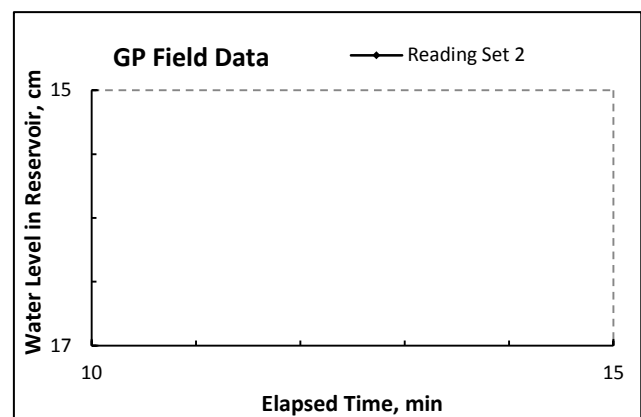
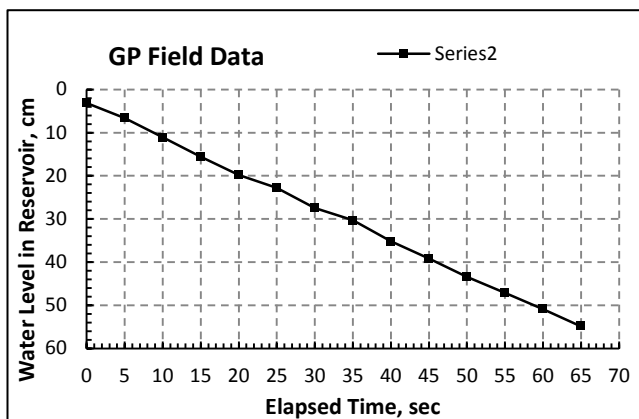
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; orangish brown, loose to medium dense, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse.
	<i>Test conducted in the middle of a trapazoidal primary channel, moderately spaced grasses and slight ruffling.</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	0.803	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.05	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	5	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0.083	3.1	0	0					
5	0.083	6.6	3.5	42					
10	0.083	11.1	4.5	54					
15	0.083	15.6	4.5	54					
20	0.083	19.8	4.2	50.4					
25	0.083	22.8	3	36					
30	0.083	27.5	4.7	56.4					
35	0.083	30.3	2.8	33.6					
40	0.083	35.2	4.9	58.8					
45	0.083	39.2	4	48					
50	0.083	43.4	4.2	50.4					
55	0.083	47.1	3.7	44.4					
60	0.083	50.9	3.8	45.6					
65	0.083	54.9	4	48					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				47.8153846	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 8.44E-02 \text{ cm/s OR } 8.44E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	9/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration Project				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S1-PM01C	
<b>Test Location</b>	Easting	682680	Northing	7475704	RL	NA	

### Subsurface Profile

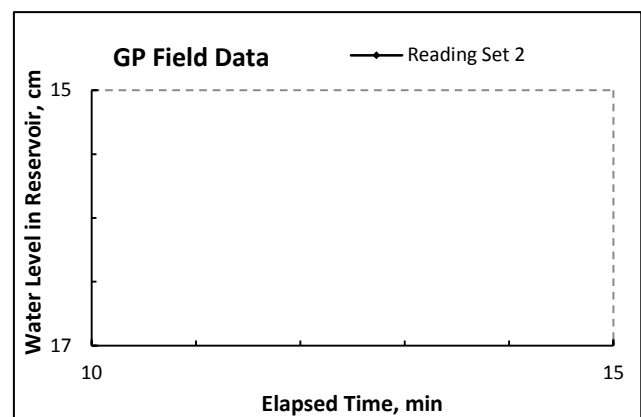
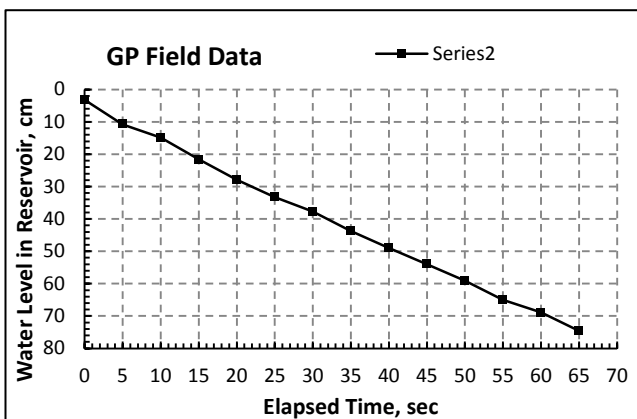
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; orangish brown, loose to medium dense, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse.
	<i>Test conducted in the middle of a trapazoidal primary channel, moderately spaced grasses and slight riffing.</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.11	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.1	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	8	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	
0	0.083	3.2	0	0						
5	0.083	10.8	7.6	91.2						
10	0.083	14.9	4.1	49.2						
15	0.083	21.6	6.7	80.4						
20	0.083	27.9	6.3	75.6						
25	0.083	33.3	5.4	64.8						
30	0.083	37.7	4.4	52.8						
35	0.083	43.7	6	72						
40	0.083	49	5.3	63.6						
45	0.083	54	5	60						
50	0.083	59.1	5.1	61.2						
55	0.083	65	5.9	70.8						
60	0.083	68.9	3.9	46.8						
65	0.083	74.6	5.7	68.4						
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				61.92	Steady Rate for 3 readings R <sub>2</sub> (cm/min)					



Field Saturated Hydraulic Conductivity

$K_{fs} = 7.04E-02 \text{ cm/s OR } 7.04E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

Client	BHP			Test Date	6/07/2019		
Project Number	045500			Tested By	MAS		
Project	Marillna Creek Infiltration GBI			Checked By	IHL		
Site Location	S2			Test ID Number	S2-PM01		
Test Location	Easting	685007	Northing	7480216	RL	NA	

### Subsurface Profile

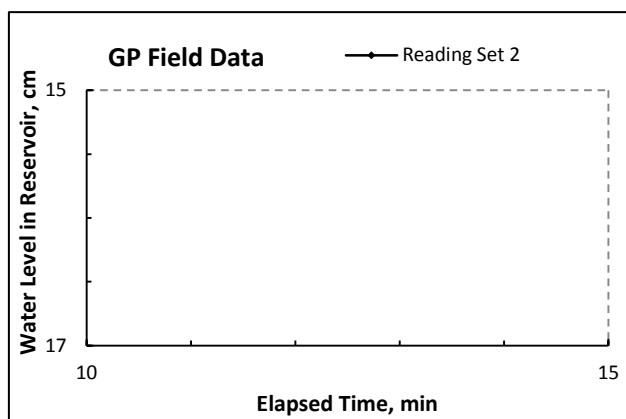
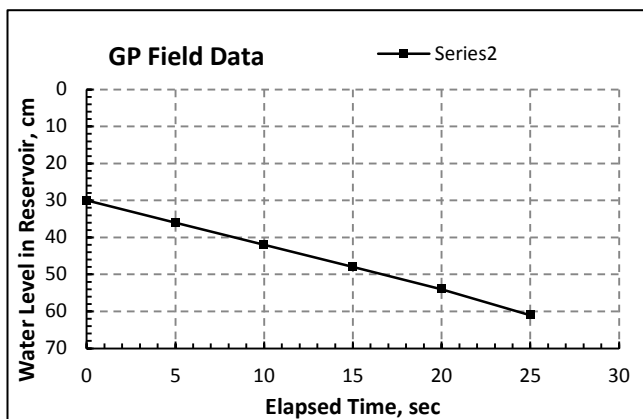
Depth	Description
0 - 0.25 m	GRAVEL with sand, trace silt
	Fine to coarse gravels
	Rounded to sub angular gravels
	Red Brown, dry, loose to medium dense.
	<i>Test conducted in low flow of a riffled channel</i>

Soil Type	α*	C1	C2
4	0.36	1.386	0

### GP Field Data

Type of Reservoir used	<u>Inner</u>			Reservoir Constant	RC	35.22	cm <sup>2</sup>
Radius of Well hole	r	3.75	cm	Depth of Well hole	d	0.075	m
Well water height - Set 1	H <sub>1</sub>	14	cm	Well Water Height - Set 2	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0	30	0	0					
5	0.083	36	6	72.3					
10	0.083	42	6	72.3					
15	0.083	48	6	72.3					
20	0.083	54	6	72.3					
25	0.083	61	7	84.3					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				72	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 3.81E-02 \text{ cm/s OR } 3.81E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	6/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS	
<b>Project</b>	Marillna Creek Infiltration GBI				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S2-PM01A	
<b>Test Location</b>	Easting	685007	Northing	7480216	RL	NA	

### Subsurface Profile

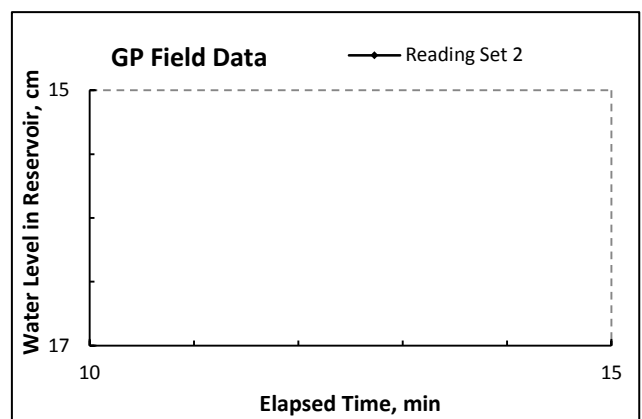
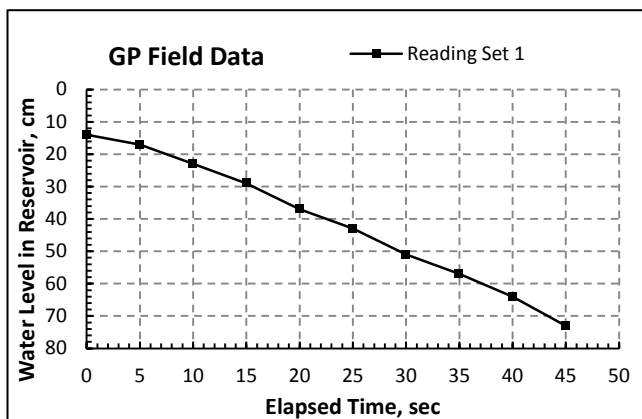
Depth	Description
0 - 0.25 m	GRAVEL with sand, trace silt
	Fine to coarse gravels
	Rounded to sub angular gravels
	Red Brown, dry, loose to medium dense.
Site Description	Test conducted in low flow of a riffled channel (test not videoed).

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.386	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>		<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>	
<b>Radius of Well hole</b>	r	3.75	cm	<b>Depth of Well hole</b>	d	0.075	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	14	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0	14	0	0					
5	0.083	17	3	36					
10	0.083	23	6	72					
15	0.083	29	6	72					
20	0.083	37	8	96					
25	0.083	43	6	72					
30	0.083	51	8	96					
35	0.083	57	6	72					
40	0.083	64	7	84					
45	0.083	73	9	108					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				84	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 4.45E-02$  cm/s OR  $4.45E-04$  m/s



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	6/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS	
<b>Project</b>	Marillna Creek Infiltration GBI				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S2-PM01B	
<b>Test Location</b>	Easting	685007	Northing	7480216	RL	NA	

### Subsurface Profile

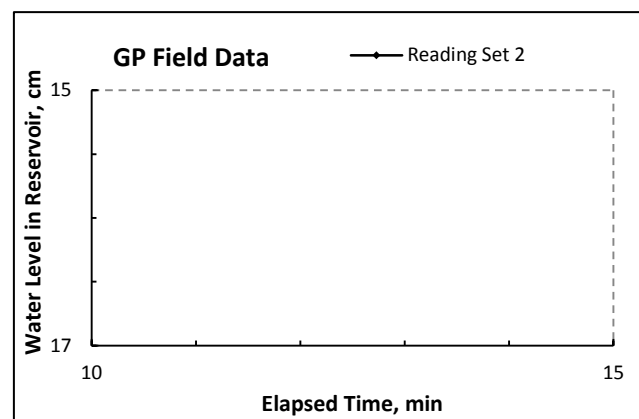
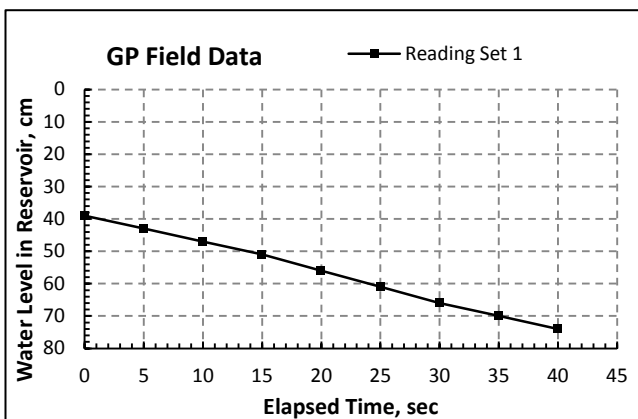
Depth	Description
0 - 0.25 m	GRAVEL with sand, trace silt
	Fine to coarse gravels
	Rounded to sub angular gravels
	Red Brown, dry, loose to medium dense.
Site Description	<i>Test conducted in low flow of a riffled channel</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	0.954	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>		<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3.75 cm	<b>Depth of Well hole</b>	d	0.05	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	8 cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0	39	0	0					
5	0.083	43	4	48					
10	0.083	47	4	48					
15	0.083	51	4	48					
20	0.083	56	5	60					
25	0.083	61	5	60					
30	0.083	66	5	60					
35	0.083	70	4	48					
40	0.083	74	4	48					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				48	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 4.60E-02 \text{ cm/s OR } 4.60E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	6/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration Project				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S2-PM02	
<b>Test Location</b>	Easting	685000	Northing	7480266	RL	NA	

### Subsurface Profile

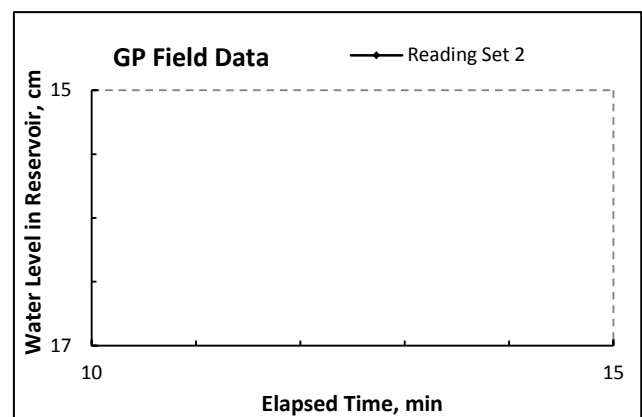
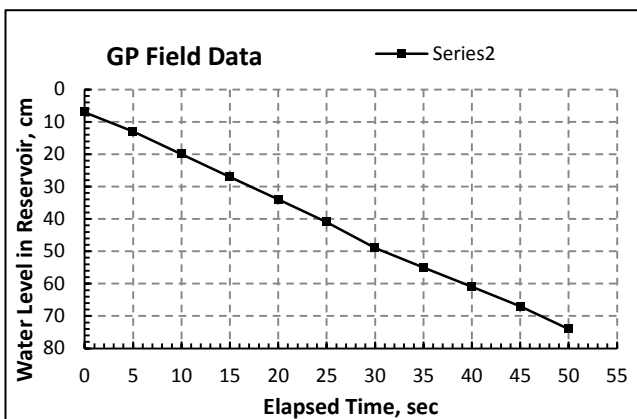
Depth	Description
0 - 0.05 m	Silt veneer
0.05 - 0.1 m	Sandy GRAVEL, trace silt; orangey brown, loose, dry; well graded, gravel, fine to coarse, rounded to sub angular; sand, fine to coarse.
Site Description	<i>Test conducted in the low flow seperated from primary by gravel bar, localised silt veneer.</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	2.246	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>		<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.01
<b>Well water height - Set 1</b>	H <sub>1</sub>	25	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>	cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0	7	0	0					
5	0.083	13	6	72					
10	0.083	20	7	84					
15	0.083	27	7	84					
20	0.083	34	7	84					
25	0.083	41	7	84					
30	0.083	49	8	96					
35	0.083	55	6	72					
40	0.083	61	6	72					
45	0.083	67	6	72					
50	0.083	74	7	84					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				84	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 2.50E-02$  cm/s OR  $2.50E-04$  m/s



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	7/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S3-PM01	
<b>Test Location</b>	Easting	691535	Northing	7484604	RL	NA	

### Subsurface Profile

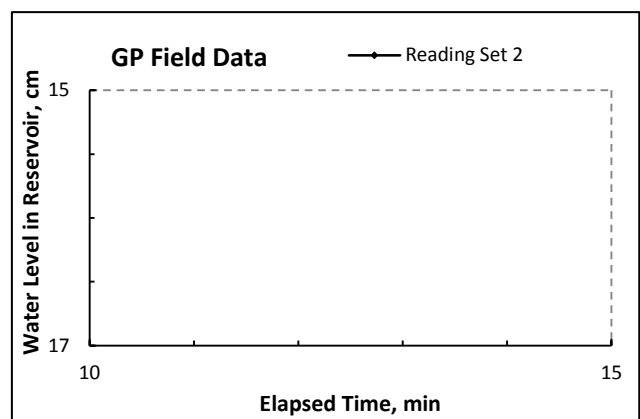
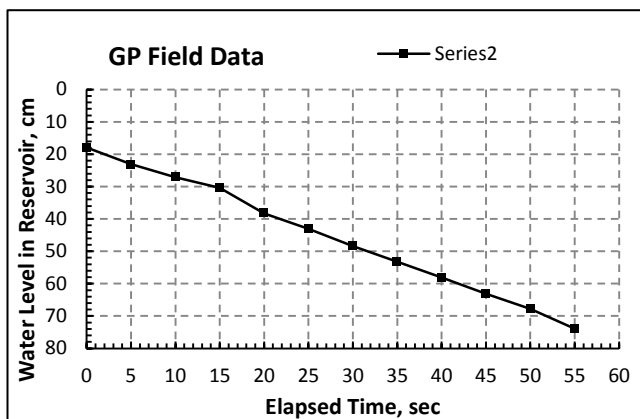
Depth	Description
0 - 0.10 m	Sandy GRAVEL, with cobbles; red to brown, loose to medium dense, well graded; dry; gravel, fine to coarse, rounded to subangular; sand fine to coarse; cobbles 6%. <i>Test conducted in low flow channel of wedge, base is riffled with scattered trees and grasses</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.11	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.1	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	8	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0.083	17.9	0	0					
5	0.083	23.1	5.2	62.4					
10	0.083	27.1	4	48					
15	0.083	30.4	3.3	39.6					
20	0.083	38.3	7.9	94.8					
25	0.083	43.1	4.8	57.6					
30	0.083	48.4	5.3	63.6					
35	0.083	53.2	4.8	57.6					
40	0.083	58.1	4.9	58.8					
45	0.083	63.1	5	60					
50	0.083	67.8	4.7	56.4					
55	0.083	73.9	6.1	73.2					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				59.5	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 6.76E-02 \text{ cm/s OR } 6.76E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP			<b>Test Date</b>	7/07/2019	
<b>Project Number</b>	045500			<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration			<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi			<b>Test ID Number</b>	S3-PM01A	
<b>Test Location</b>	Easting	691535	Northing	7484604	RL	NA

### Subsurface Profile

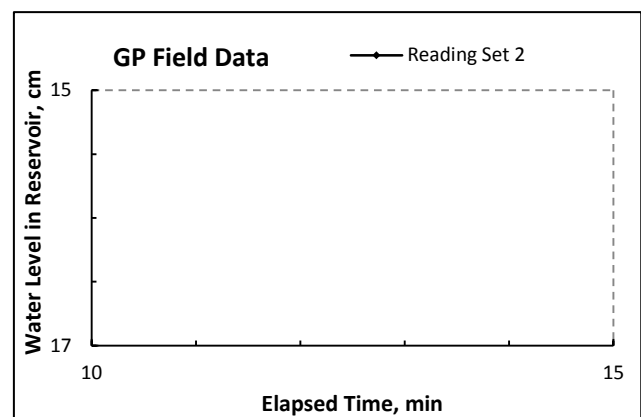
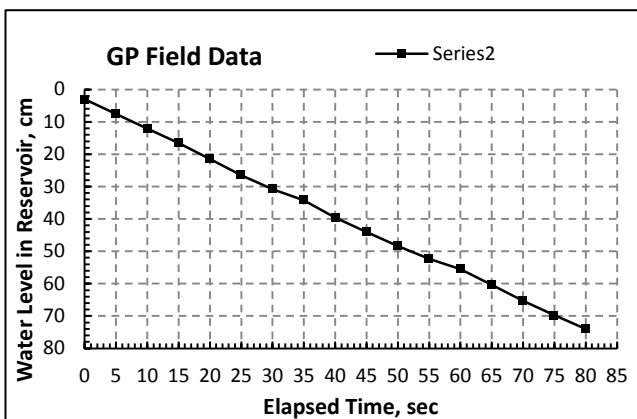
Depth	Description
0 - 0.10 m	Sandy GRAVEL, with cobbles; red - brown, loose to medium dense, well graded; dry; gravel, fine to coarse, rounded to subangular; sand fine to coarse; cobbles 6%
	<i>Test conducted in low flow channel of wedge, base is riffled with scattered trees and grasses</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.11	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>		<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.12
<b>Well water height - Set 1</b>	H <sub>1</sub>	8	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>	

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0.083	3	0	0					
5	0.083	7.5	4.5	54					
10	0.083	12.1	4.6	55.2					
15	0.083	16.5	4.4	52.8					
20	0.083	21.5	5	60					
25	0.083	26.5	5	60					
30	0.083	30.8	4.3	51.6					
35	0.083	34.2	3.4	40.8					
40	0.083	39.6	5.4	64.8					
45	0.083	44	4.4	52.8					
50	0.083	48.4	4.4	52.8					
55	0.083	52.3	3.9	46.8					
60	0.083	55.5	3.2	38.4					
65	0.083	60.4	4.9	58.8					
70	0.083	65.3	4.9	58.8					
75	0.083	69.8	4.5	54					
80	0.083	74	4.2	50.4					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				53.8	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 6.12E-02 \text{ cm/s OR } 6.12E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	8/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S4-PM01	
<b>Test Location</b>	Easting	683098	Northing	7475839	RL	NA	

### Subsurface Profile

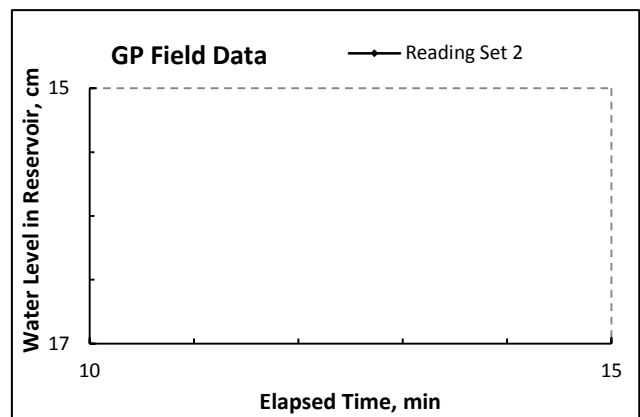
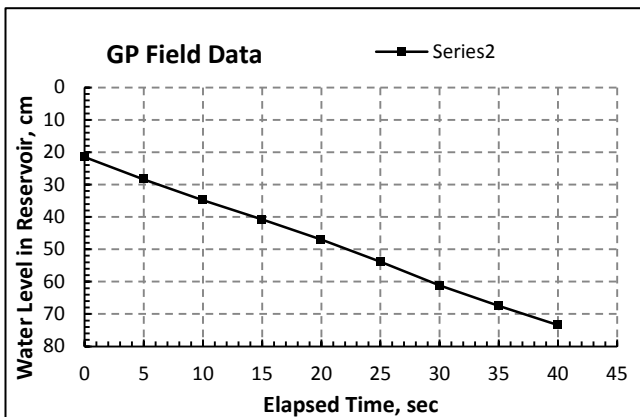
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; reddish brown, loose to medium dense, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse
Site Description	<i>Test conducted irregular riffled trapazoidal channel</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.449	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.1	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	12	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0.083	21.5	0	0					
5	0.083	28.4	6.9	82.8					
10	0.083	34.8	6.4	76.8					
15	0.083	40.8	6	72					
20	0.083	47	6.2	74.4					
25	0.083	53.8	6.8	81.6					
30	0.083	61.2	7.4	88.8					
35	0.083	67.5	6.3	75.6					
40	0.083	73.4	5.9	70.8					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				74.4	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 5.48E-02 \text{ cm/s OR } 5.48E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	8/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S4-PM01A	
<b>Test Location</b>	Easting	683098	Northing	7475839	RL	NA	

### Subsurface Profile

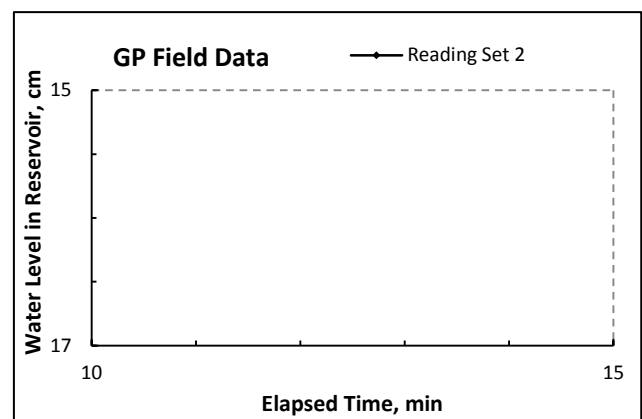
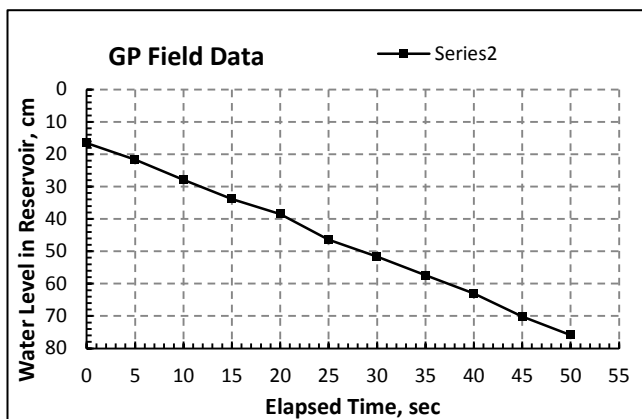
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; reddish brown, loose to medium dense, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse
Site Description	<i>Test conducted irregular riffled trapazoidal channel</i>

Soil Type	α*	C1	C2
4	0.36	1.201	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>		<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>	
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.1	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	9	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0.083	16.5	0	0					
5	0.083	21.7	5.2	62.4					
10	0.083	27.9	6.2	74.4					
15	0.083	33.9	6	72					
20	0.083	38.5	4.6	55.2					
25	0.083	46.4	7.9	94.8					
30	0.083	51.7	5.3	63.6					
35	0.083	57.4	5.7	68.4					
40	0.083	63	5.6	67.2					
45	0.083	70.2	7.2	86.4					
50	0.083	75.9	5.7	68.4					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				66.4	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 6.69E-02 \text{ cm/s OR } 6.69E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	10/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S5-PM01	
<b>Test Location</b>	Easting	697291	Northing	7485802	RL	NA	

### Subsurface Profile

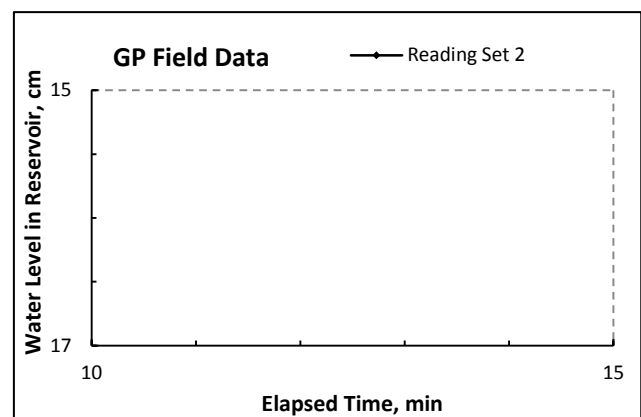
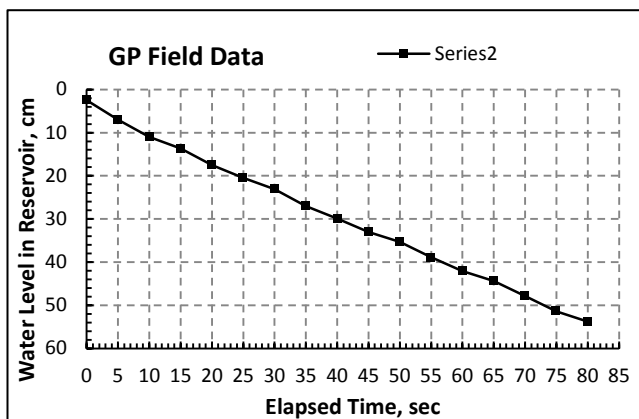
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; orangish brown, loose, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse
	<i>Test in broad uniform primary channel, central gravel bar</i>
	<i>seperates primary from low flow channel on the southern bank</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.014	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.75	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	7	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapse d time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapse d time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	
0	0.083	2.5	0	0						
5	0.083	7	4.5	54						
10	0.083	11	4	48						
15	0.083	13.7	2.7	32.4						
20	0.083	17.5	3.8	45.6						
25	0.083	20.5	3	36						
30	0.083	23.1	2.6	31.2						
35	0.083	27	3.9	46.8						
40	0.083	29.9	2.9	34.8						
45	0.083	33	3.1	37.2						
50	0.083	35.3	2.3	27.6						
55	0.083	38.9	3.6	43.2						
60	0.083	42.1	3.2	38.4						
65	0.083	44.4	2.3	27.6						
70	0.083	47.9	3.5	42						
75	0.083	51.4	3.5	42						
80	0.083	53.8	2.4	28.8						
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				38.3	Steady Rate for 3 readings R <sub>2</sub> (cm/min)					



Field Saturated Hydraulic Conductivity

$K_{fs} = 4.97E-02 \text{ cm/s OR } 4.97E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	10/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S5-PM01A	
<b>Test Location</b>	Easting	697291	Northing	7485802	RL	NA	

### Subsurface Profile

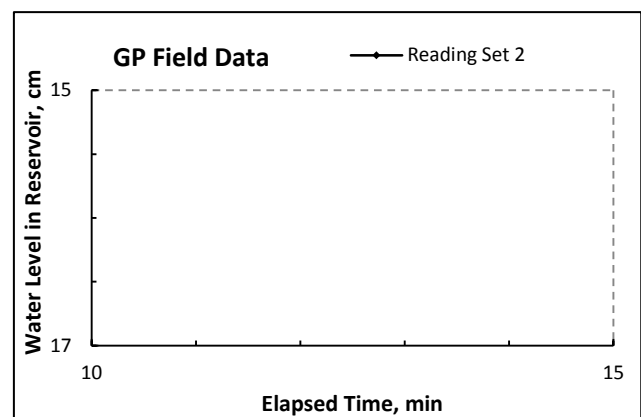
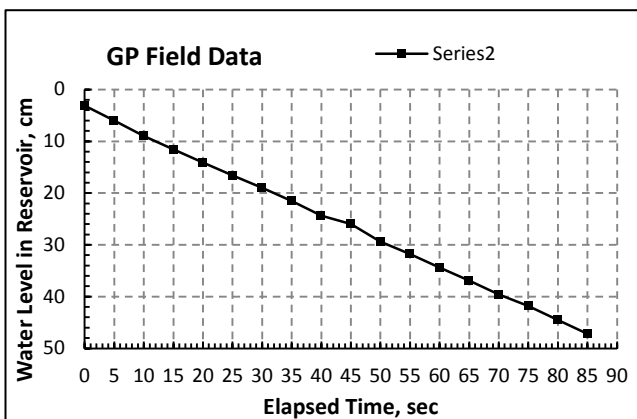
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; orangish brown, loose, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse
	<i>Test in broad uniform primary channel, central gravel bar</i>
	<i>seperates primary from low flow channel on the southern bank</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	0.803	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.75	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	5	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	
0	0.083	3.1	0	0						
5	0.083	6	2.9	34.8						
10	0.083	9	3	36						
15	0.083	11.6	2.6	31.2						
20	0.083	14.1	2.5	30						
25	0.083	16.6	2.5	30						
30	0.083	19	2.4	28.8						
35	0.083	21.5	2.5	30						
40	0.083	24.4	2.9	34.8						
45	0.083	26	1.6	19.2						
50	0.083	29.4	3.4	40.8						
55	0.083	31.8	2.4	28.8						
60	0.083	34.4	2.6	31.2						
65	0.083	36.9	2.5	30						
70	0.083	39.6	2.7	32.4						
75	0.083	41.8	2.2	26.4						
80	0.083	44.5	2.7	32.4						
85	0.083	47.2	2.7	32.4						
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				30.5	Steady Rate for 3 readings R <sub>2</sub> (cm/min)					



Field Saturated Hydraulic Conductivity

$$K_{fs} = 5.38E-02 \text{ cm/s OR } 5.38E-04 \text{ m/s}$$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	10/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S5-PM02	
<b>Test Location</b>	Easting	697290	Northing	7485774	RL	NA	

### Subsurface Profile

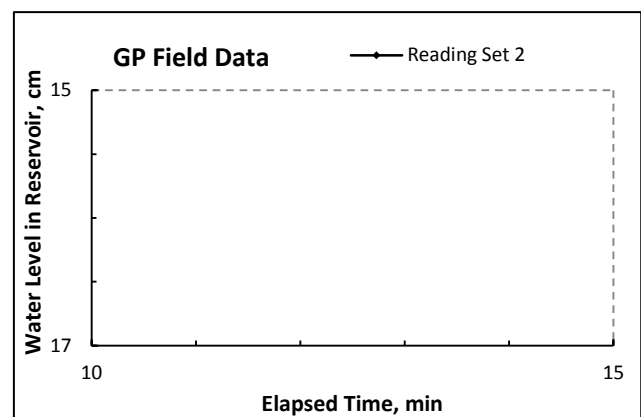
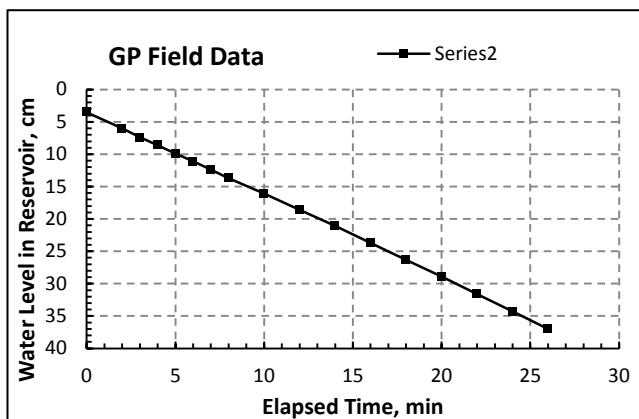
Depth	Description
0 - 0.05 m	Silt veneer
0.05 - 0.1 m	Silty sandy GRAVEL; orangey brown, loose, dry, well graded; gravel, fine to medium with minor coarse, rounded to sub angular; sand, fine to coarse; fines low to non plastic.
Site Description	Test conducted in the low flow seperated from primary by gravel bar, localized silt veneer.

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.11	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.1	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	8	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	
0	0	3.5	0	0						
2	2	6	2.5	1.25						
3	1	7.4	1.4	1.4						
4	1	8.6	1.2	1.2						
5	1	9.9	1.3	1.3						
6	1	11.1	1.2	1.2						
7	1	12.4	1.3	1.3						
8	1	13.7	1.3	1.3						
10	2	16.1	2.4	1.2						
12	2	18.6	2.5	1.25						
14	2	21.1	2.5	1.25						
16	2	23.7	2.6	1.3						
18	2	26.3	2.6	1.3						
20	2	28.9	2.6	1.3						
22	2	31.6	2.7	1.35						
24	2	34.3	2.7	1.35						
26	2	37	2.7	1.35						
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				1.3	Steady Rate for 3 readings R <sub>2</sub> (cm/min)					



Field Saturated Hydraulic Conductivity

$K_{fs} = 1.48E-03 \text{ cm/s OR } 1.48E-05 \text{ m/s}$





## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	8/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S6-PM01	
<b>Test Location</b>	Easting	685961	Northing	7481740	RL	NA	

### Subsurface Profile

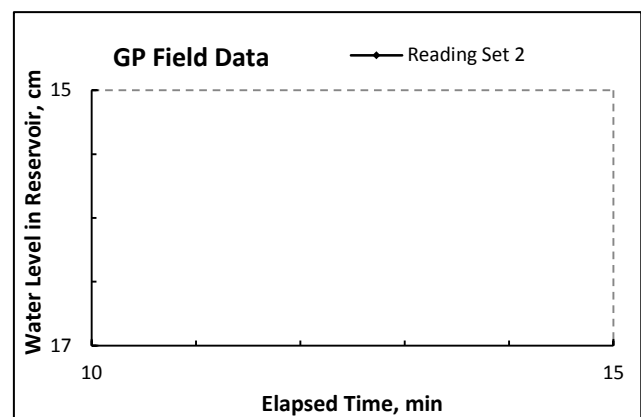
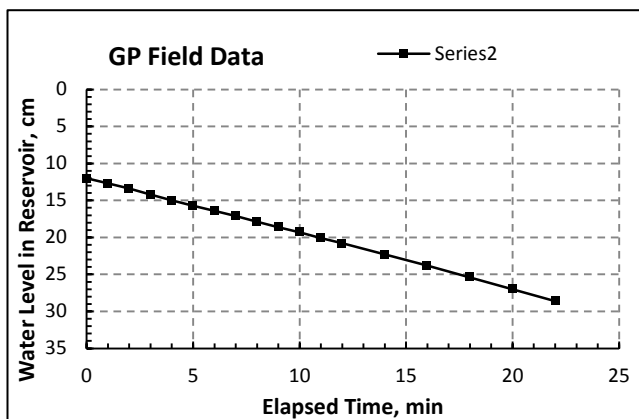
Depth	Description
0 - 0.05 m	Silt veneer
0.05 - 0.1 m	Silty sandy GRAVEL; orangey brown, loose, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse; fines low to non plastic.
Site Description	<i>Test conducted in low flow of a wedge channel, silt sourced from bank erosion forming localized silt veneer.</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.014	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>		<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3 cm	<b>Depth of Well hole</b>	d	0.1	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	7 cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	
0	0	12	0	0						
1	1	12.7	0.7	0.7						
2	1	13.4	0.7	0.7						
3	1	14.2	0.8	0.8						
4	1	15	0.8	0.8						
5	1	15.7	0.7	0.7						
6	1	16.4	0.7	0.7						
7	1	17.1	0.7	0.7						
8	1	17.9	0.8	0.8						
9	1	18.6	0.7	0.7						
10	1	19.3	0.7	0.7						
11	1	20.1	0.8	0.8						
12	1	20.8	0.7	0.7						
14	2	22.3	1.5	0.75						
16	2	23.8	1.5	0.75						
18	2	25.4	1.6	0.8						
20	2	27	1.6	0.8						
22	2	28.6	1.6	0.8						
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				0.8	Steady Rate for 3 readings R <sub>2</sub> (cm/min)					



Field Saturated Hydraulic Conductivity

$K_{fs} = 1.04E-03 \text{ cm/s OR } 1.04E-05 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	9/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S7-PM01	
<b>Test Location</b>	Easting	688288	Northing	7483198	RL	NA	

### Subsurface Profile

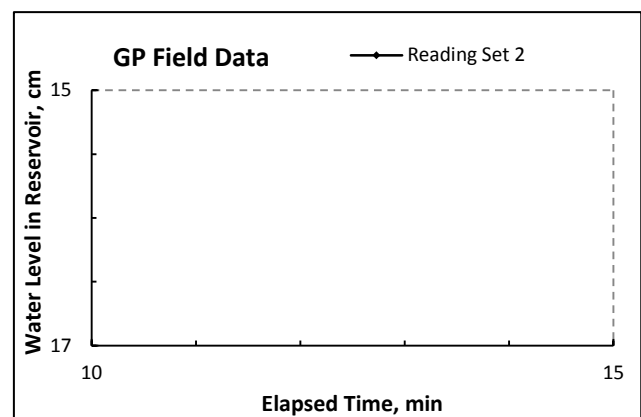
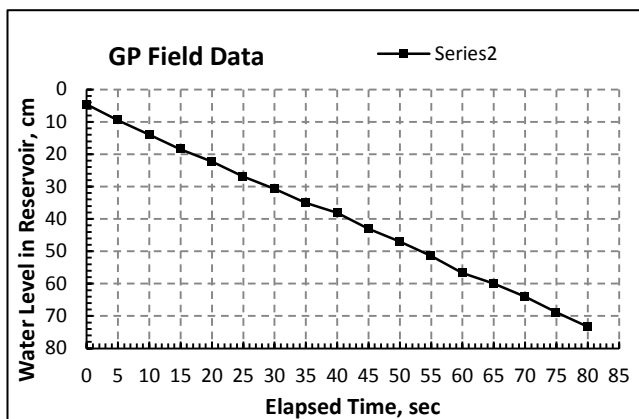
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; orangish brown, loose, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse
	<i>Test in broad uniform primary channel, central gravel bar separates primary from secondary channel</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.014	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.9	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	7	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	
0	0.083	4.6	0	0						
5	0.083	9.5	4.9	58.8						
10	0.083	14	4.5	54						
15	0.083	18.5	4.5	54						
20	0.083	22.3	3.8	45.6						
25	0.083	26.9	4.6	55.2						
30	0.083	30.7	3.8	45.6						
35	0.083	35	4.3	51.6						
40	0.083	38.1	3.1	37.2						
45	0.083	43.1	5	60						
50	0.083	47	3.9	46.8						
55	0.083	51.5	4.5	54						
60	0.083	56.7	5.2	62.4						
65	0.083	60	3.3	39.6						
70	0.083	64	4	48						
75	0.083	68.9	4.9	58.8						
80	0.083	73.3	4.4	52.8						
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				52.1	Steady Rate for 3 readings R <sub>2</sub> (cm/min)					



Field Saturated Hydraulic Conductivity

$K_{fs} = 6.76E-02 \text{ cm/s OR } 6.76E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	9/07/2019	
<b>Project Number</b>	045560				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S7-PM01A	
<b>Test Location</b>	Easting	688288	Northing	7483198	RL	NA	

### Subsurface Profile

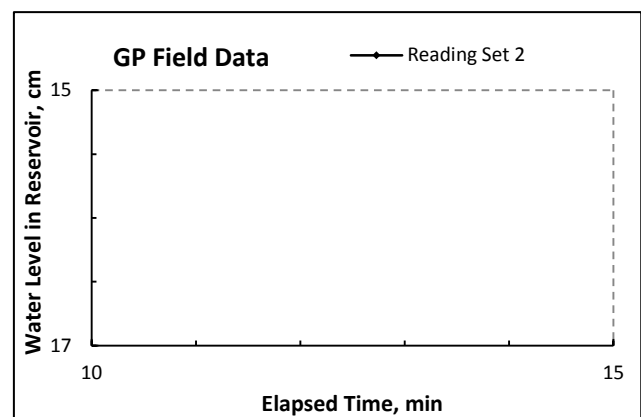
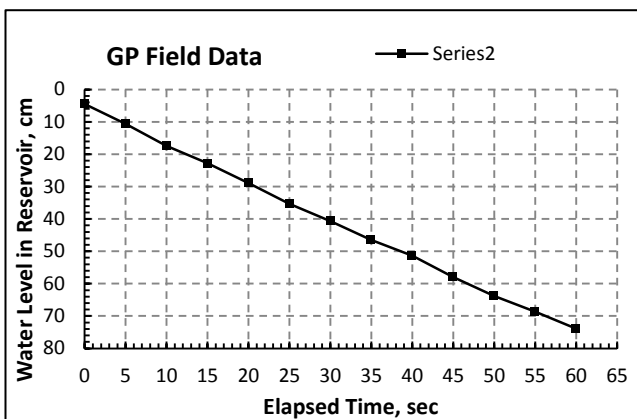
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace silt; orangish brown, loose, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse
	<i>Test in broad uniform primary channel, central gravel bar</i>
	<i>seperates primary from secondary channel</i>

Soil Type	$\alpha^*$	C1	C2
4	0.36	1.288	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.9	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	10	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)
0	0.083	4.5	0	0					
5	0.083	10.6	6.1	73.2					
10	0.083	17.4	6.8	81.6					
15	0.083	22.8	5.4	64.8					
20	0.083	28.9	6.1	73.2					
25	0.083	35.3	6.4	76.8					
30	0.083	40.7	5.4	64.8					
35	0.083	46.5	5.8	69.6					
40	0.083	51.4	4.9	58.8					
45	0.083	58	6.6	79.2					
50	0.083	63.8	5.8	69.6					
55	0.083	68.7	4.9	58.8					
60	0.083	73.9	5.2	62.4					
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				72	Steady Rate for 3 readings R <sub>2</sub> (cm/min)				



Field Saturated Hydraulic Conductivity

$K_{fs} = 6.48E-02 \text{ cm/s OR } 6.48E-04 \text{ m/s}$



## Guelph Permeameter Field Data Field Saturated Hydraulic Conductivity

<b>Client</b>	BHP				<b>Test Date</b>	11/07/2019	
<b>Project Number</b>	045500				<b>Tested By</b>	MAS/VBJ	
<b>Project</b>	Marillana Creek Infiltration Project				<b>Checked By</b>	IHL	
<b>Site Location</b>	Yandi				<b>Test ID Number</b>	S9-PM01	
<b>Test Location</b>	Easting	683702	Northing	7476910	RL	NA	

### Subsurface Profile

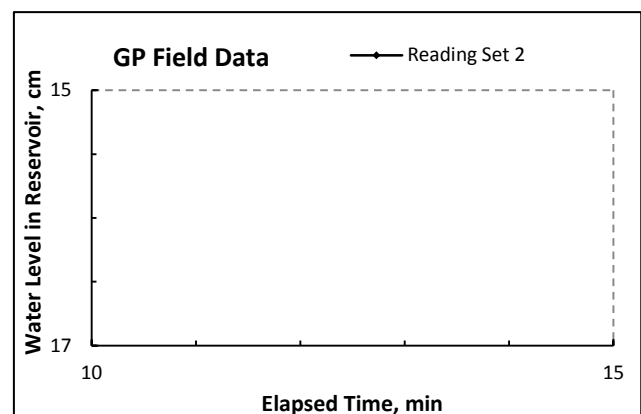
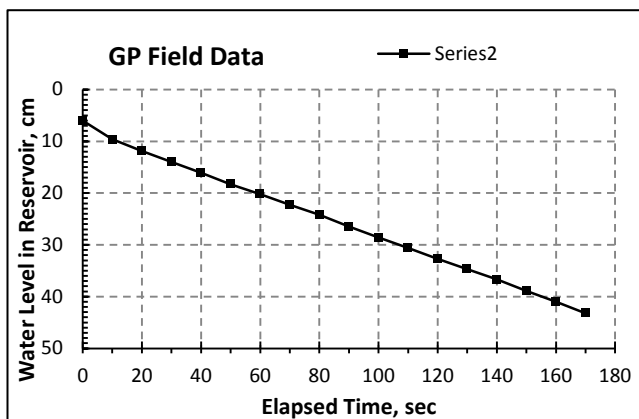
Depth	Description
0 - 0.10 m	Sandy GRAVEL, trace cobbles, trace silt; reddish brown, loose, dry, well graded; gravel, fine to coarse, rounded to sub angular; sand, fine to coarse
Site Description	Test conducted in low flow of broad vegetated riffled channel

Soil Type	$\alpha^*$	C1	C2
4	0.36	0.686	0

### GP Field Data

<b>Type of Reservoir used</b>	<u>Inner</u>			<b>Reservoir Constant</b>	RC	35.22	cm <sup>2</sup>
<b>Radius of Well hole</b>	r	3	cm	<b>Depth of Well hole</b>	d	0.9	m
<b>Well water height - Set 1</b>	H <sub>1</sub>	4	cm	<b>Well Water Height - Set 2</b>	H <sub>2</sub>		cm

Elapsed time (sec)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	Elapsed time (min)	Time interval (min)	Water level in reservoir (cm)	Change in Water level	Rate of water level change, R (cm/min)	
0	0.167	6	0	0						
10	0.167	9.6	3.6	21.6						
20	0.167	11.9	2.3	13.8						
30	0.167	14	2.1	12.6						
40	0.167	16.1	2.1	12.6						
50	0.167	18.3	2.2	13.2						
60	0.167	20.2	1.9	11.4						
70	0.167	22.3	2.1	12.6						
80	0.167	24.2	1.9	11.4						
90	0.167	26.5	2.3	13.8						
100	0.167	28.6	2.1	12.6						
110	0.167	30.6	2	12						
120	0.167	32.7	2.1	12.6						
130	0.167	34.7	2	12						
140	0.167	36.7	2	12						
150	0.167	38.9	2.2	13.2						
160	0.167	41	2.1	12.6						
170	0.167	43.2	2.2	13.2						
Steady Rate for 3 readings R <sub>1</sub> (cm/min)				12.6	Steady Rate for 3 readings R <sub>2</sub> (cm/min)					



Field Saturated Hydraulic Conductivity

$K_{fs} = 2.67E-02 \text{ cm/s OR } 2.67E-04 \text{ m/s}$

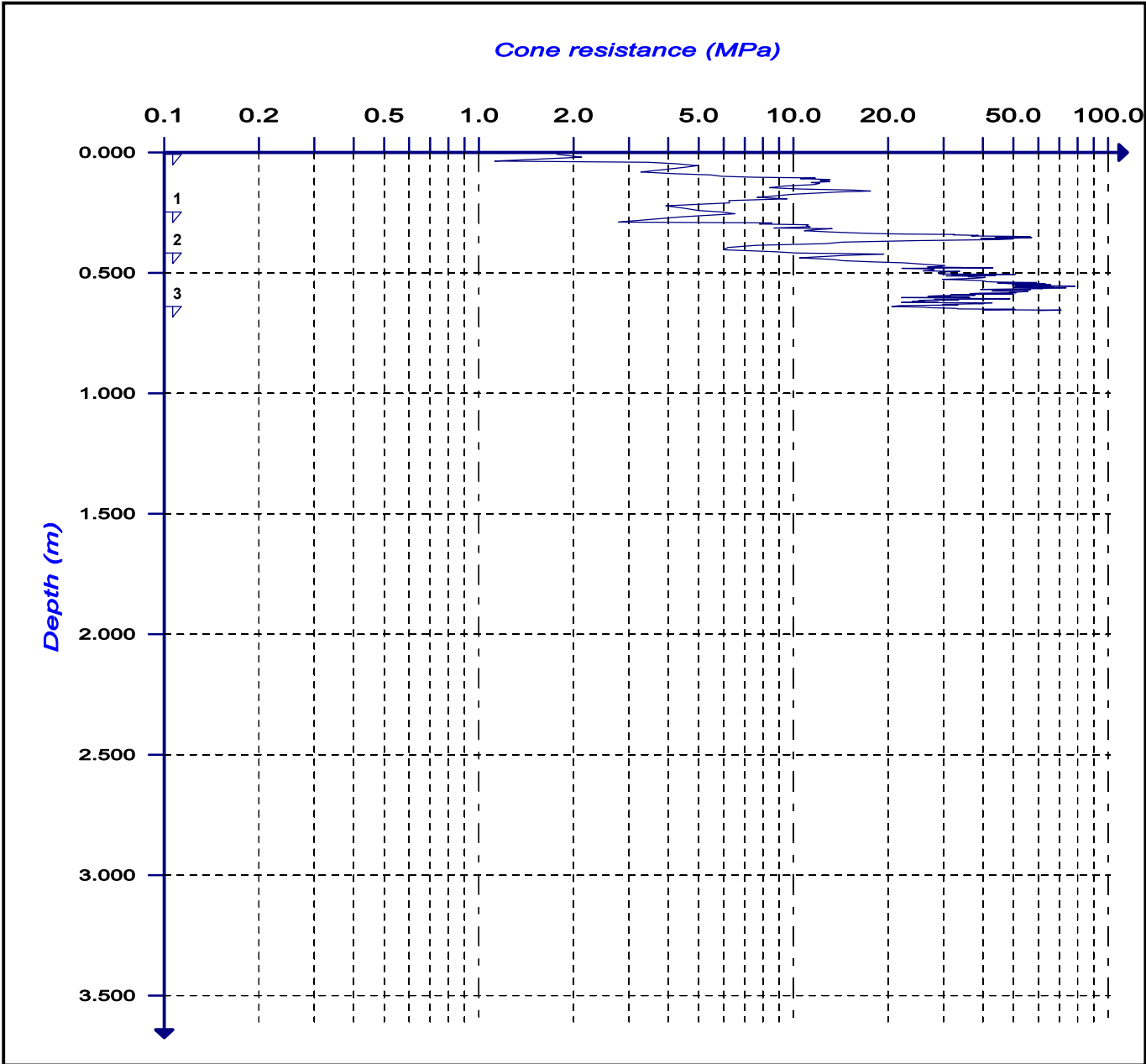


**APPENDIX G**  
**PANDA DCP LOGS**



# Ground investigation with variable energy dynamic penetrometer

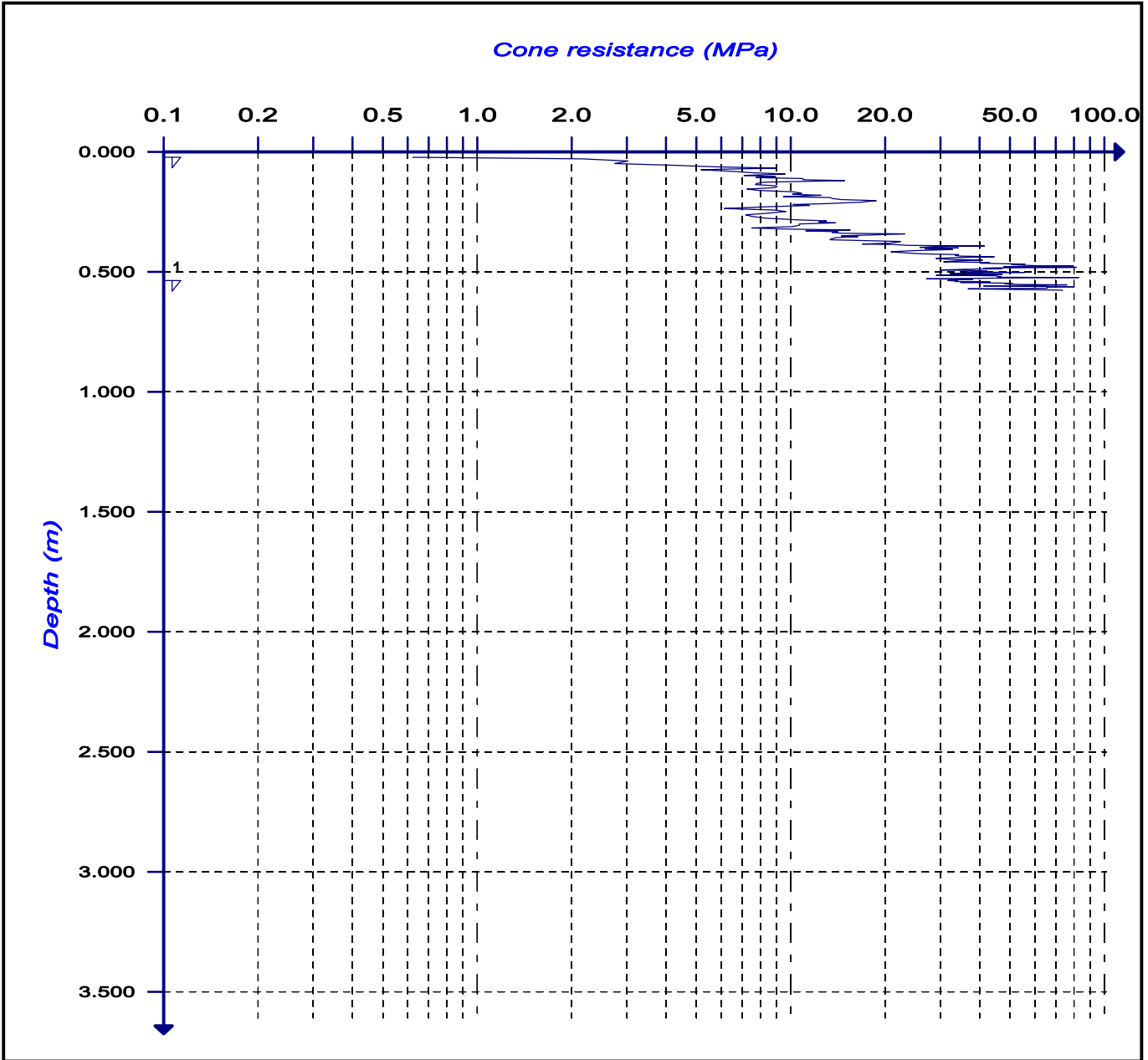
Marillana Creek Infiltration Project			
Sounding : S1-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 9/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

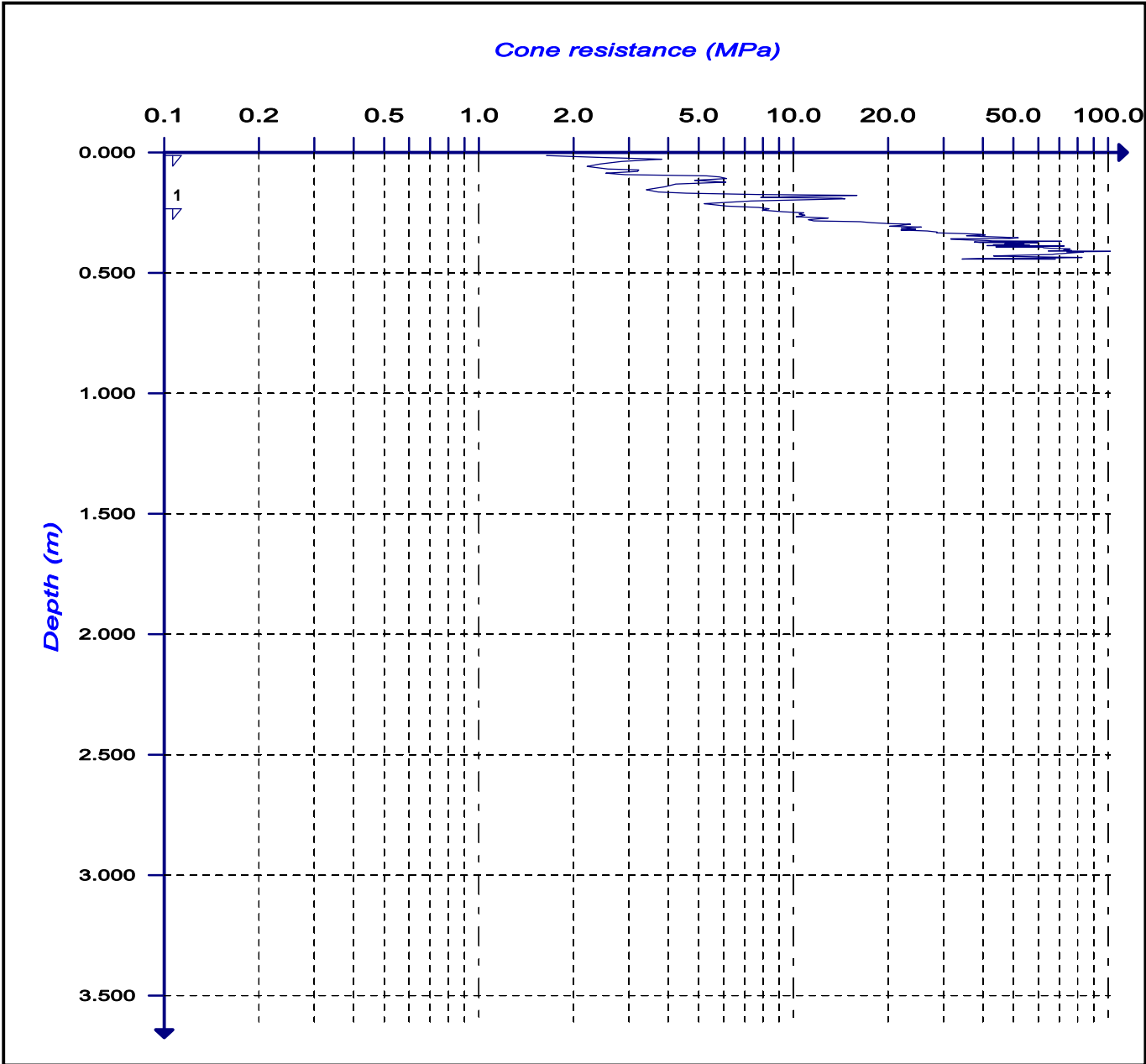
Marillana Creek Infiltration Project			
Sounding : S1-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 9/07/2019	





# Ground investigation with variable energy dynamic penetrometer

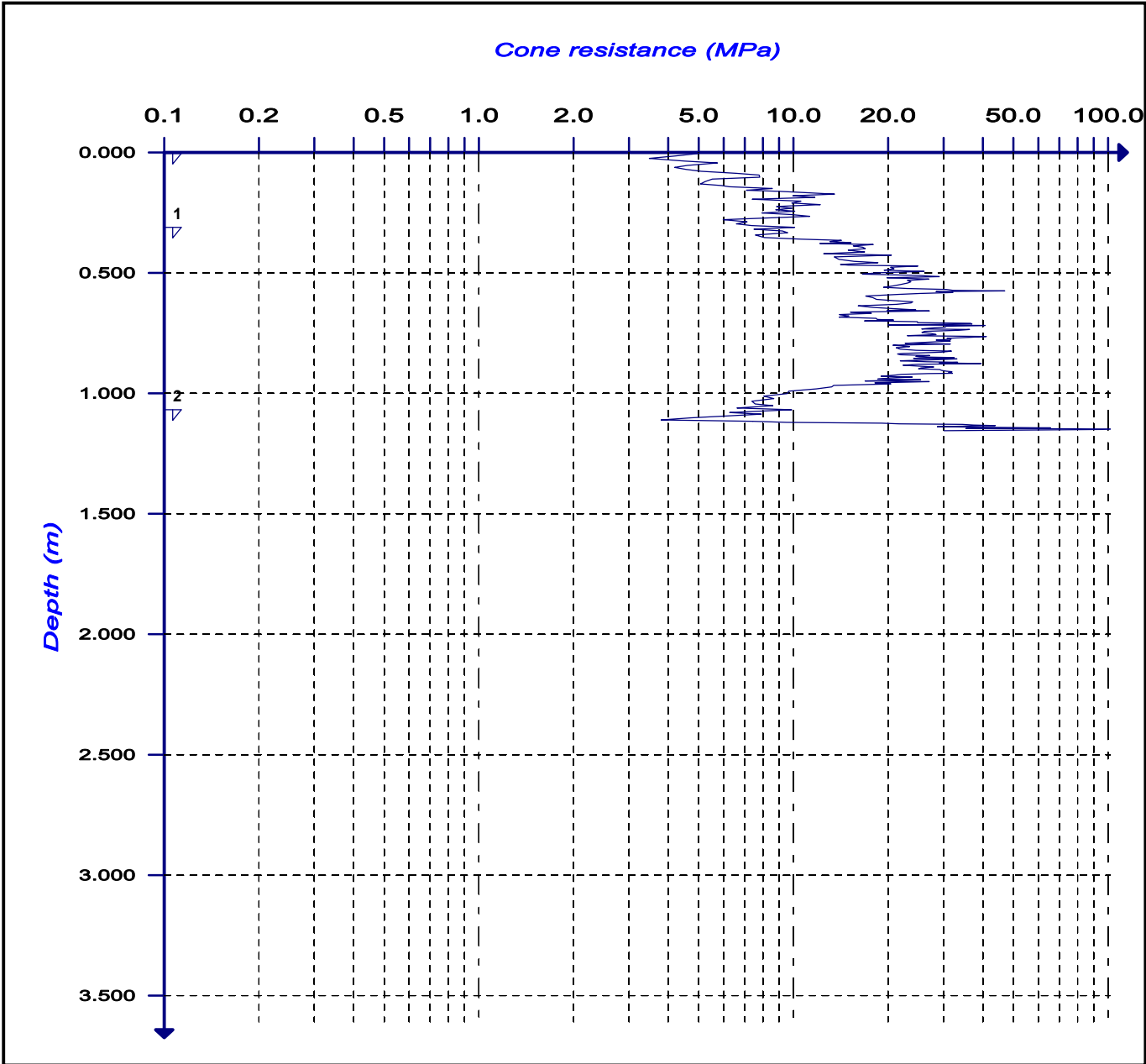
Marillana Creek Infiltration Project			
Sounding : S1-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 9/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

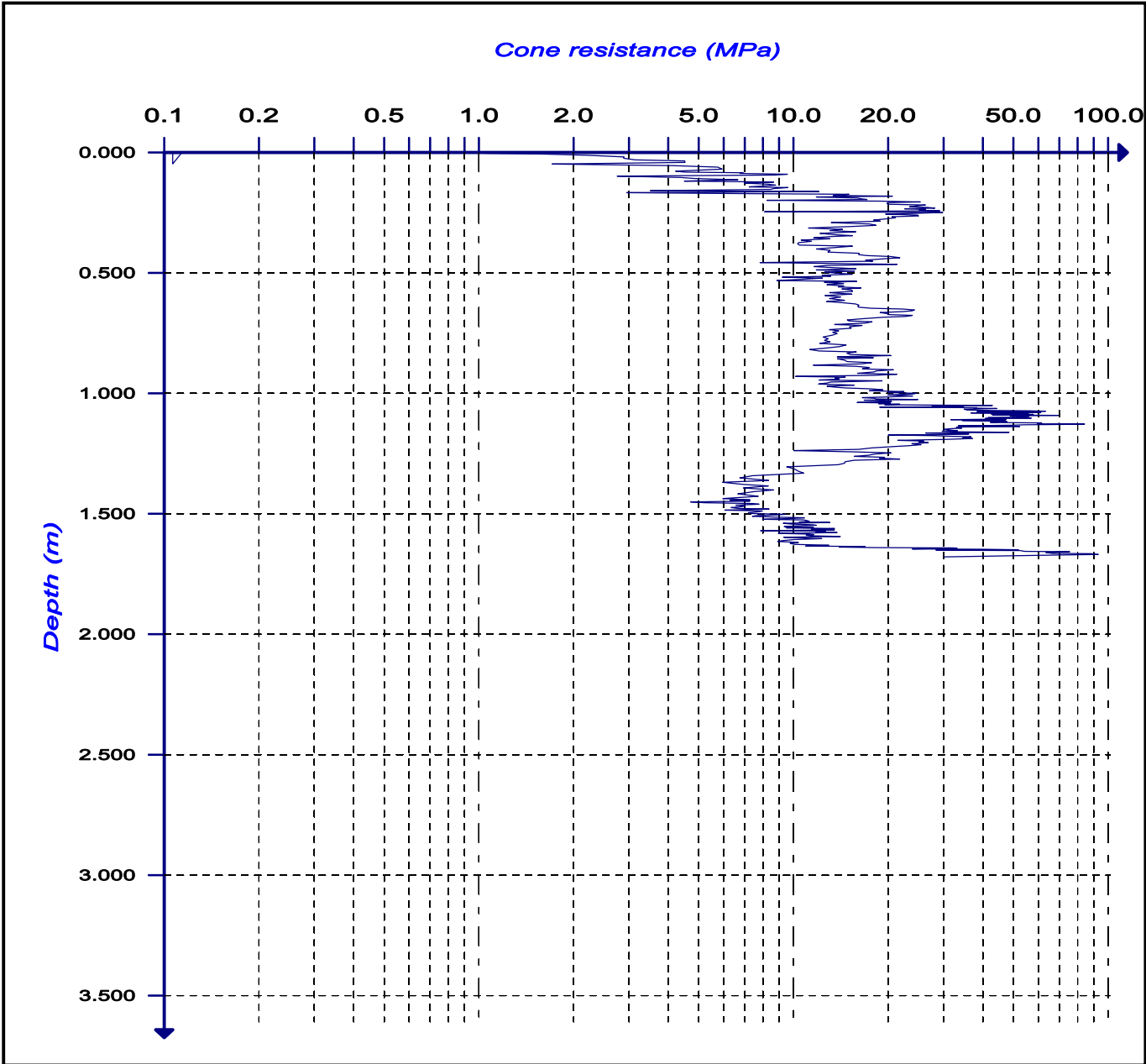
Marillana Creek Infiltration Project			
Sounding : S1-PM01 PDCP04			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 9/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

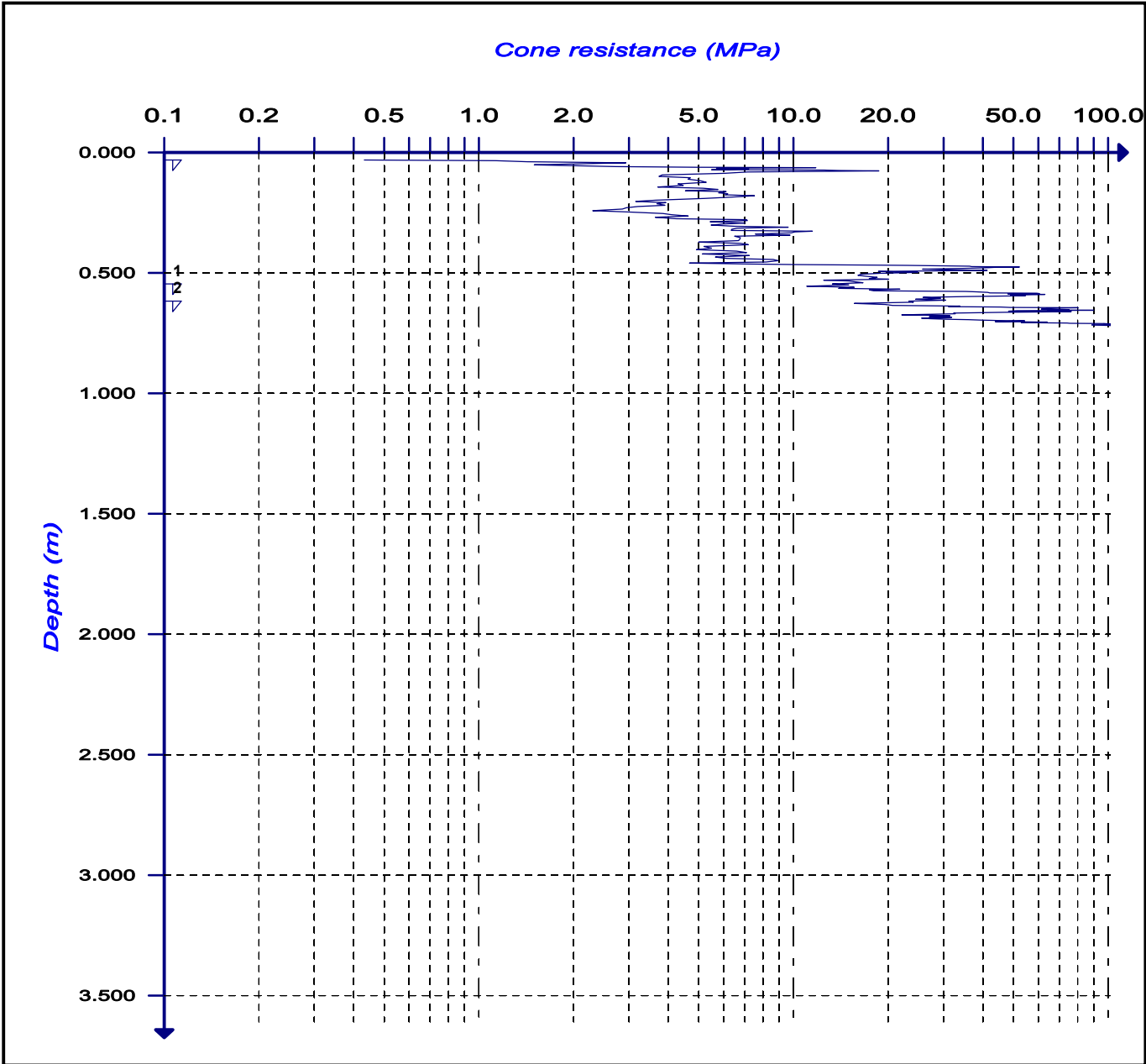
Marillana Creek Infiltration Project			
Sounding : S2-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 7/07/2019	





# Ground investigation with variable energy dynamic penetrometer

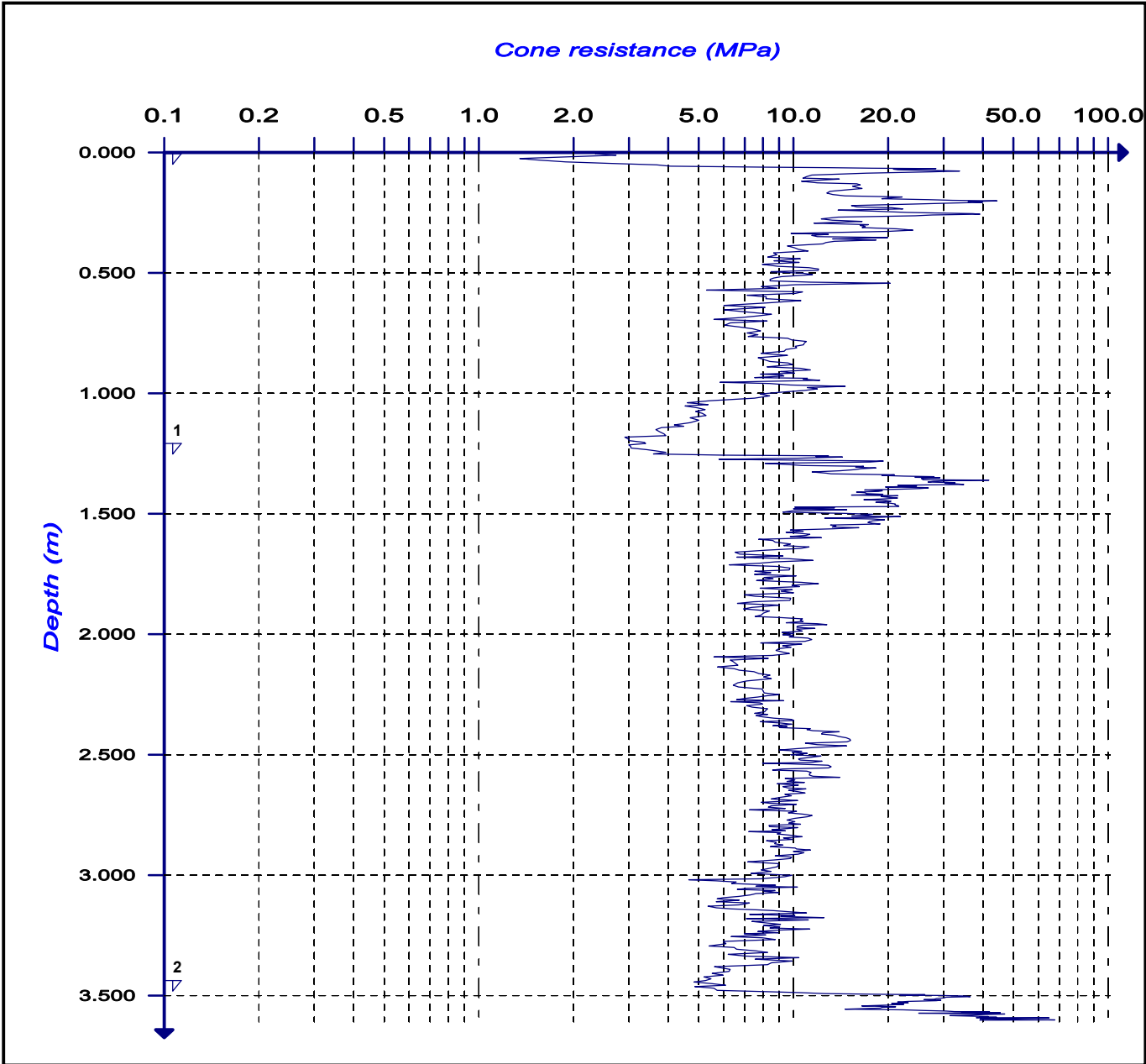
Marillana Creek Infiltration Project			
Sounding : S2-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 7/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

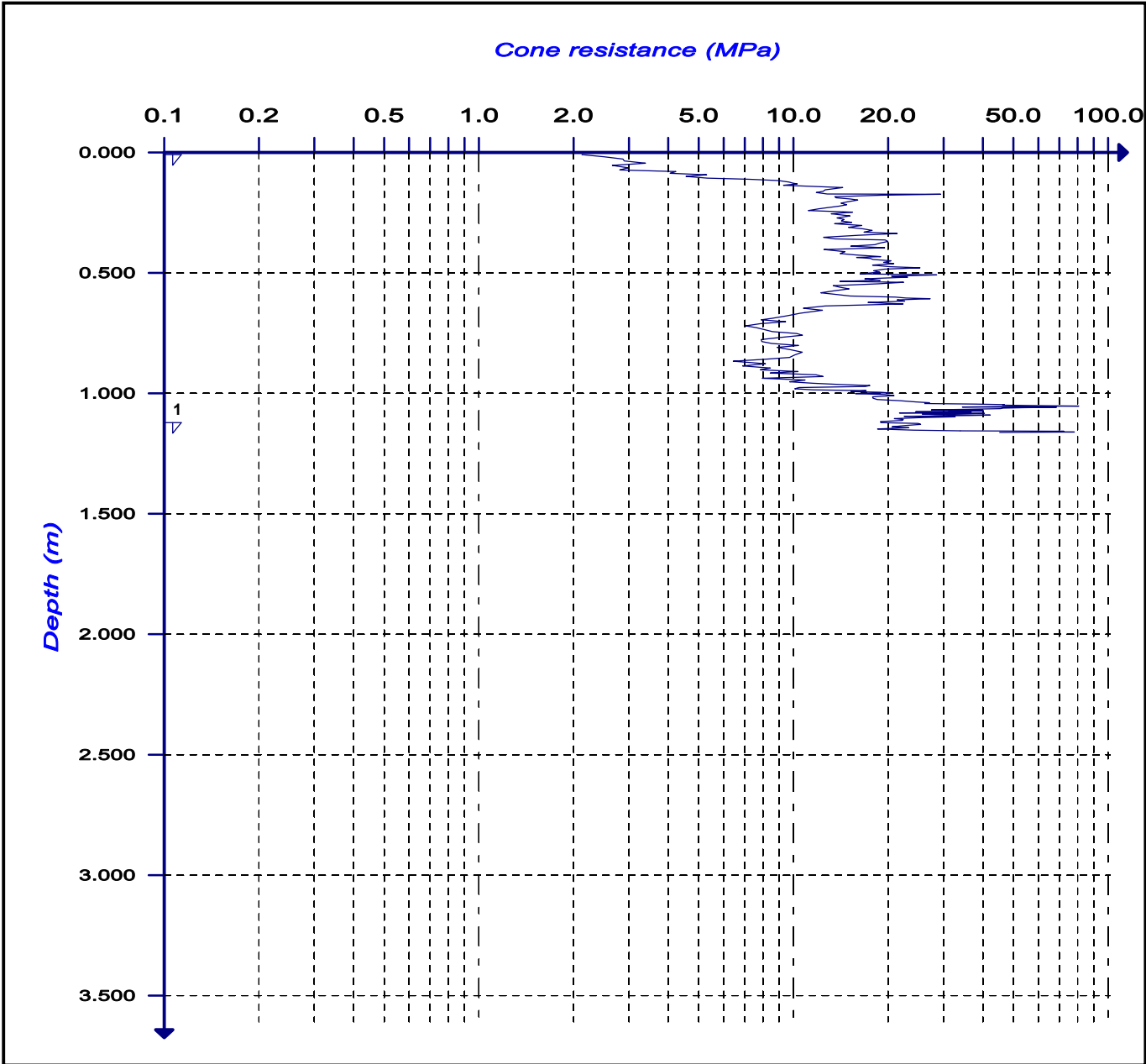
Marillana Creek Infiltration Project			
Sounding : S2-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 7/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

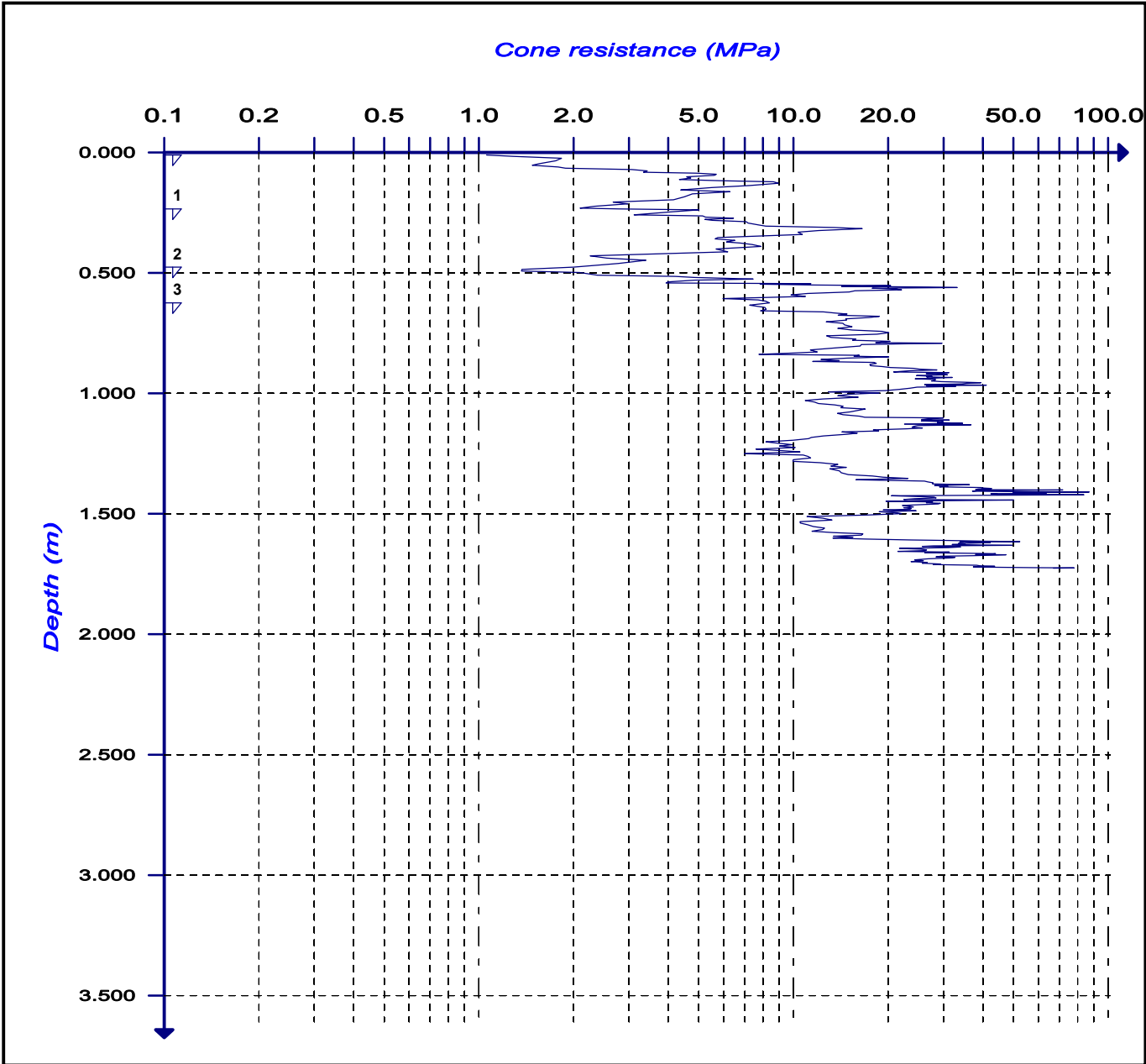
Marillana Creek Infiltration Project			
Sounding : S2-PM02 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 11/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

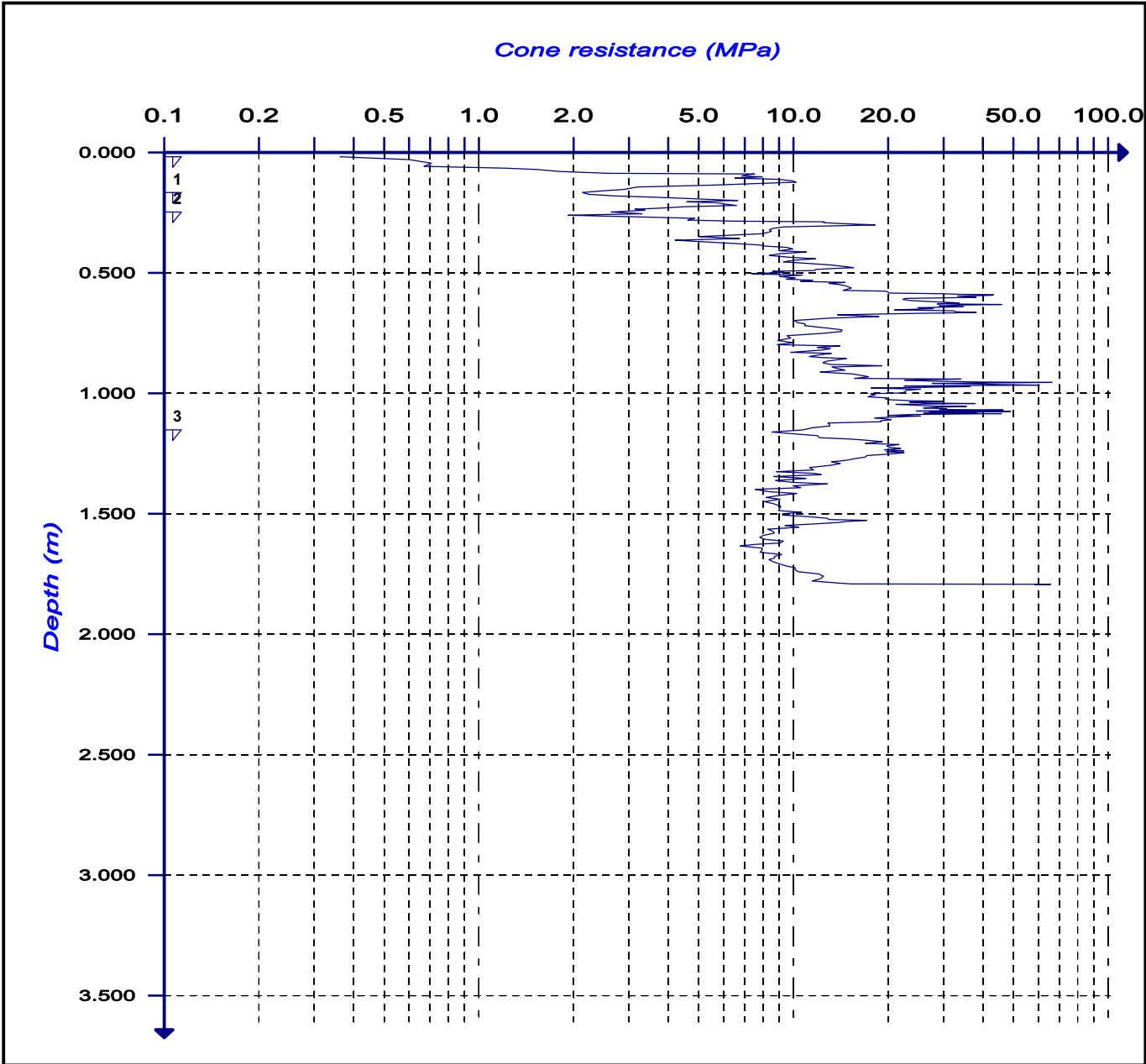
Marillana Creek Infiltration Project			
Sounding : S2-PM02 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 11/07/2019	





# Ground investigation with variable energy dynamic penetrometer

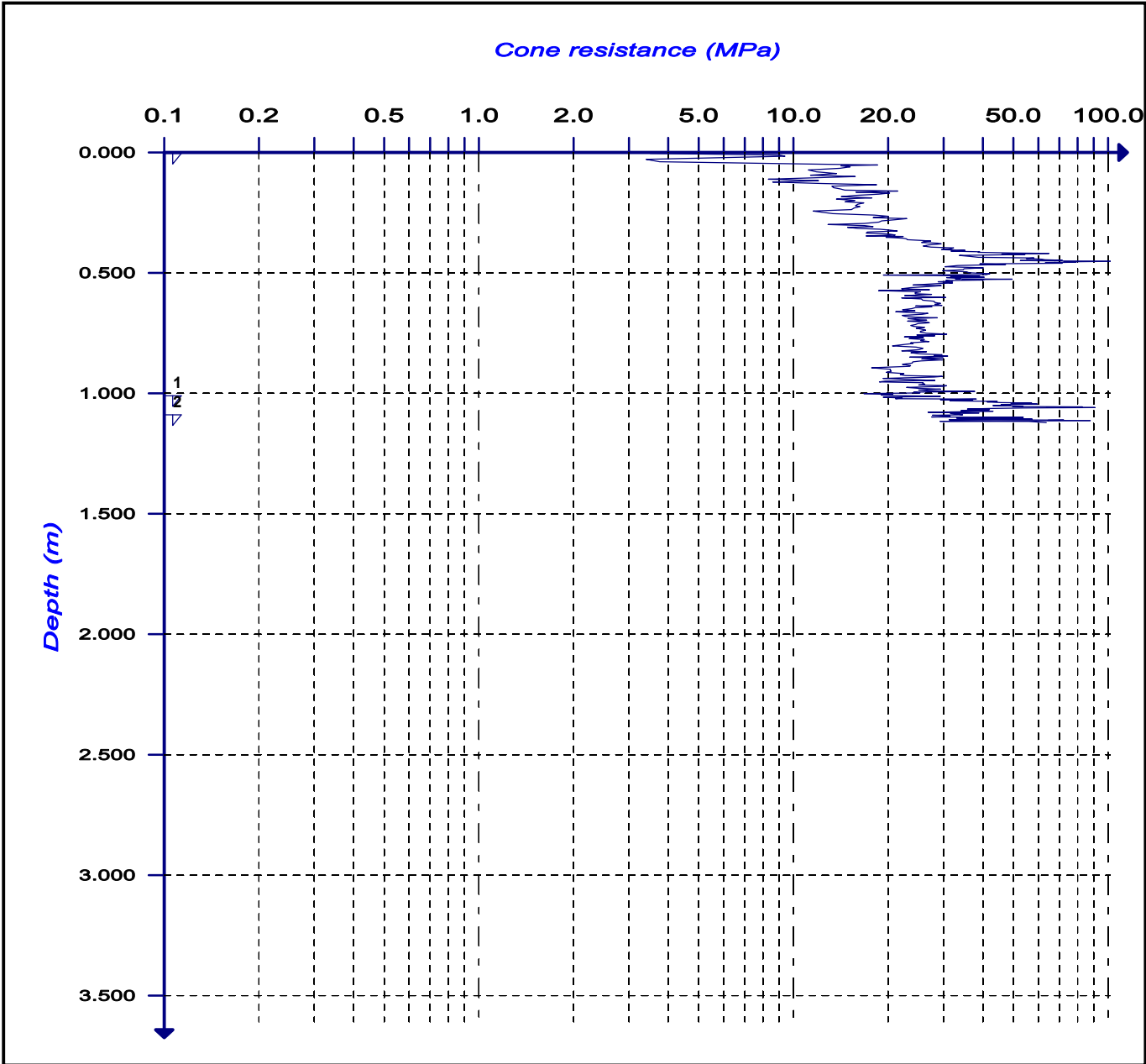
Marillana Creek Infiltration Project			
Sounding : S2-PM02 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 11/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

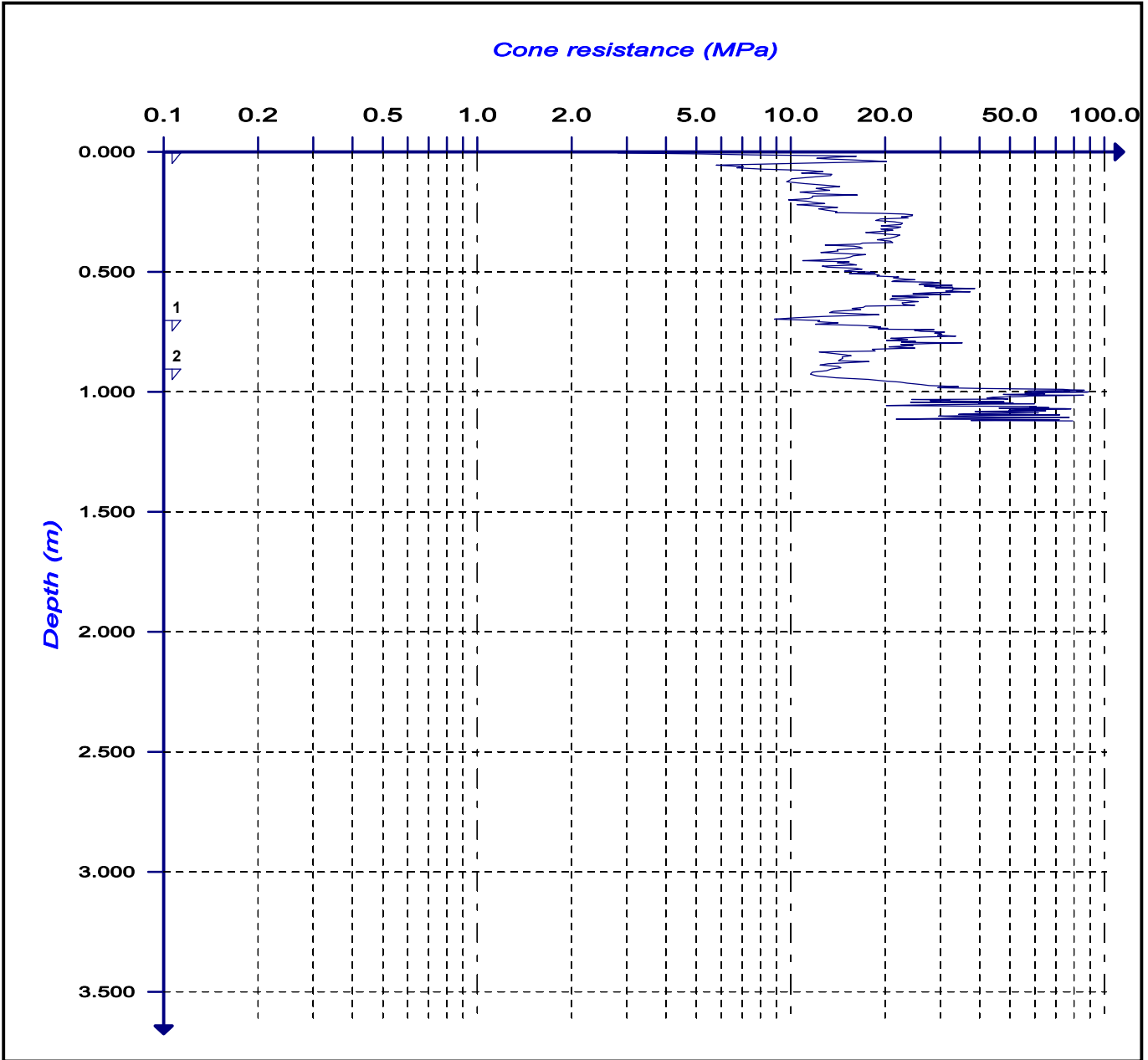
Marillana Creek Infiltration Project			
Sounding : S3-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 7/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

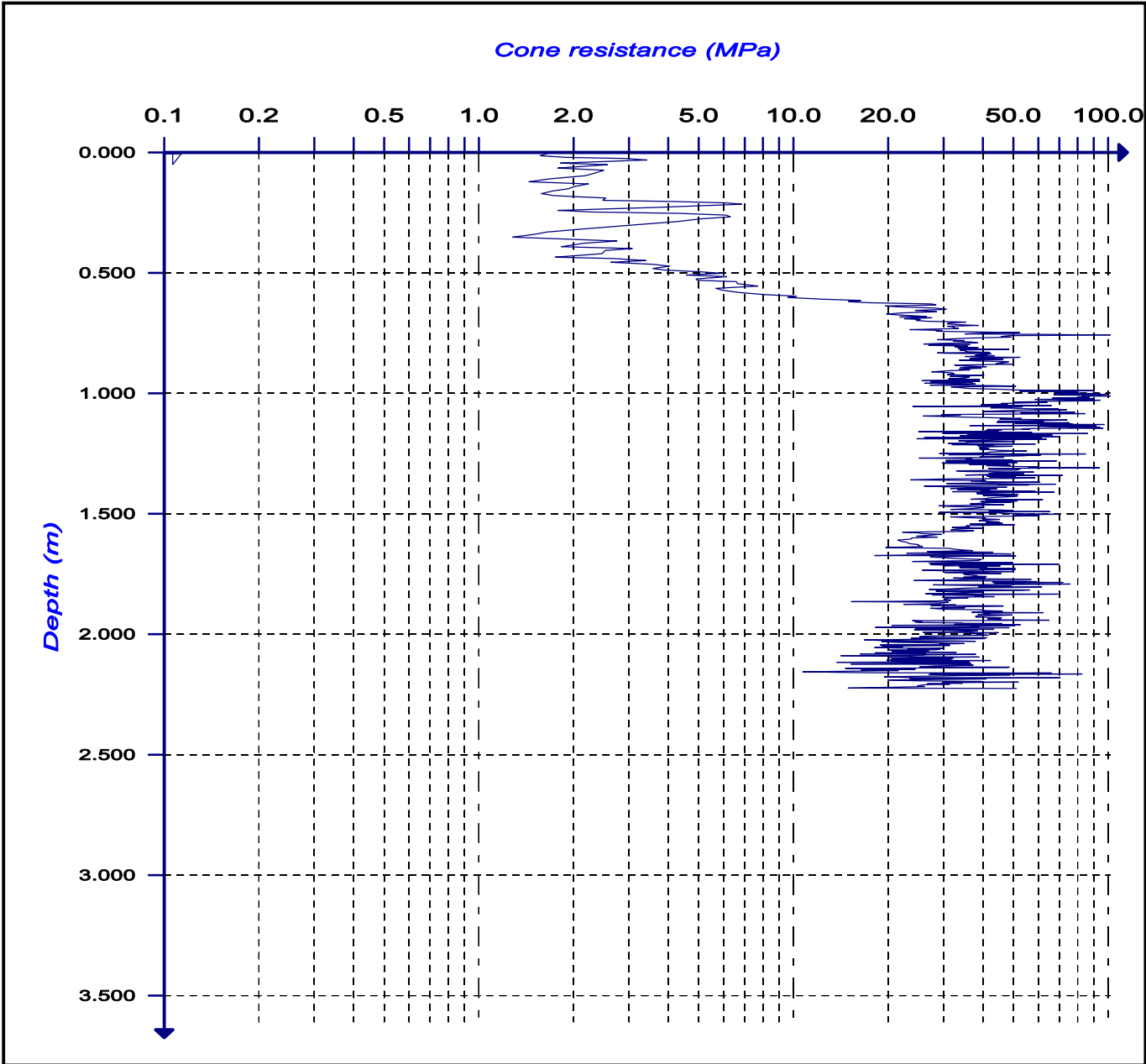
Marillana Creek Infiltration Project			
Sounding : S3-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 7/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

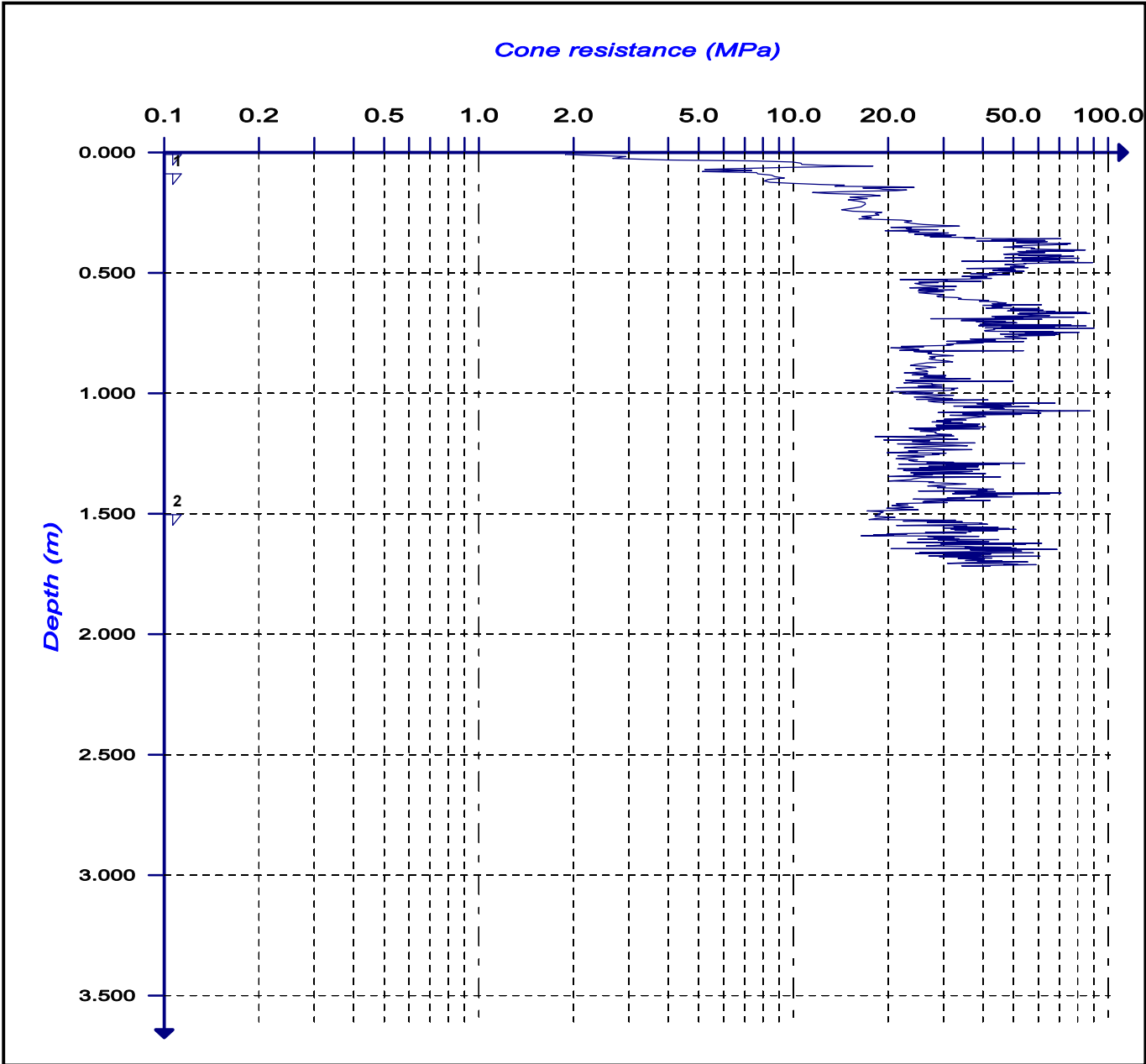
Marillana Creek Infiltration Project			
Sounding : S3-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 7/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

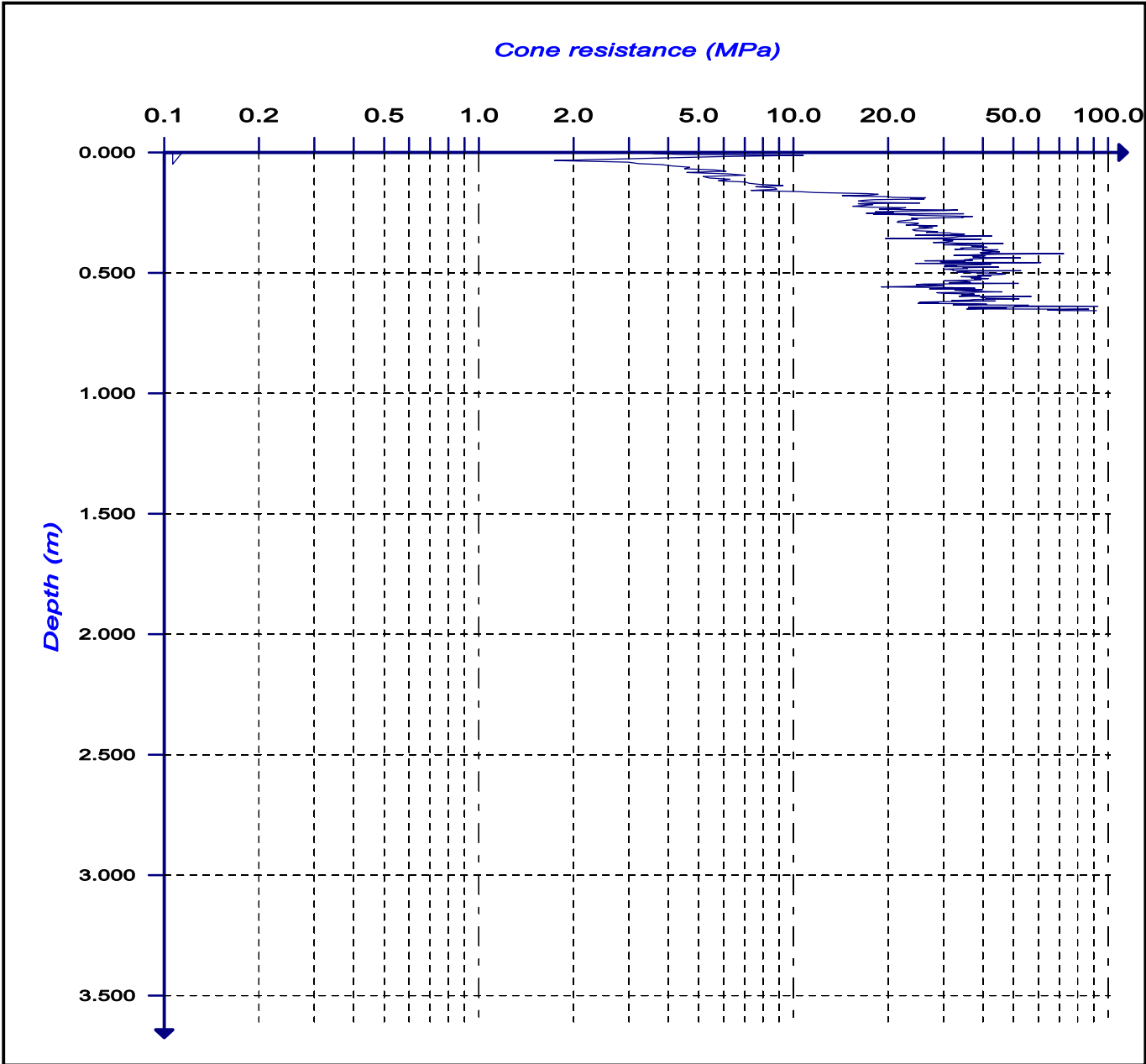
Marillana Creek Infiltration Project			
Sounding : S4-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





# Ground investigation with variable energy dynamic penetrometer

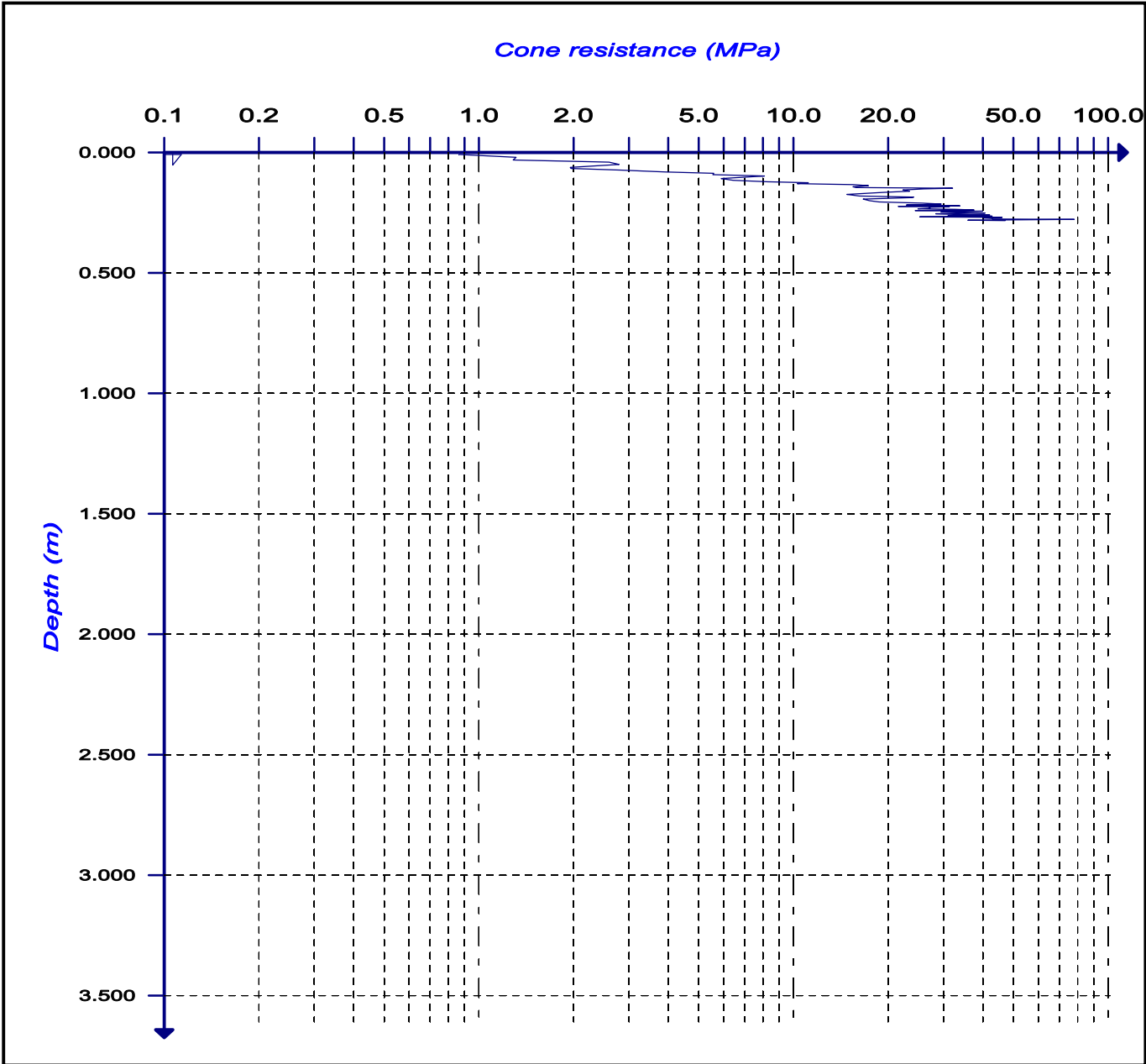
Marillana Creek Infiltration Project			
Sounding : S4-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

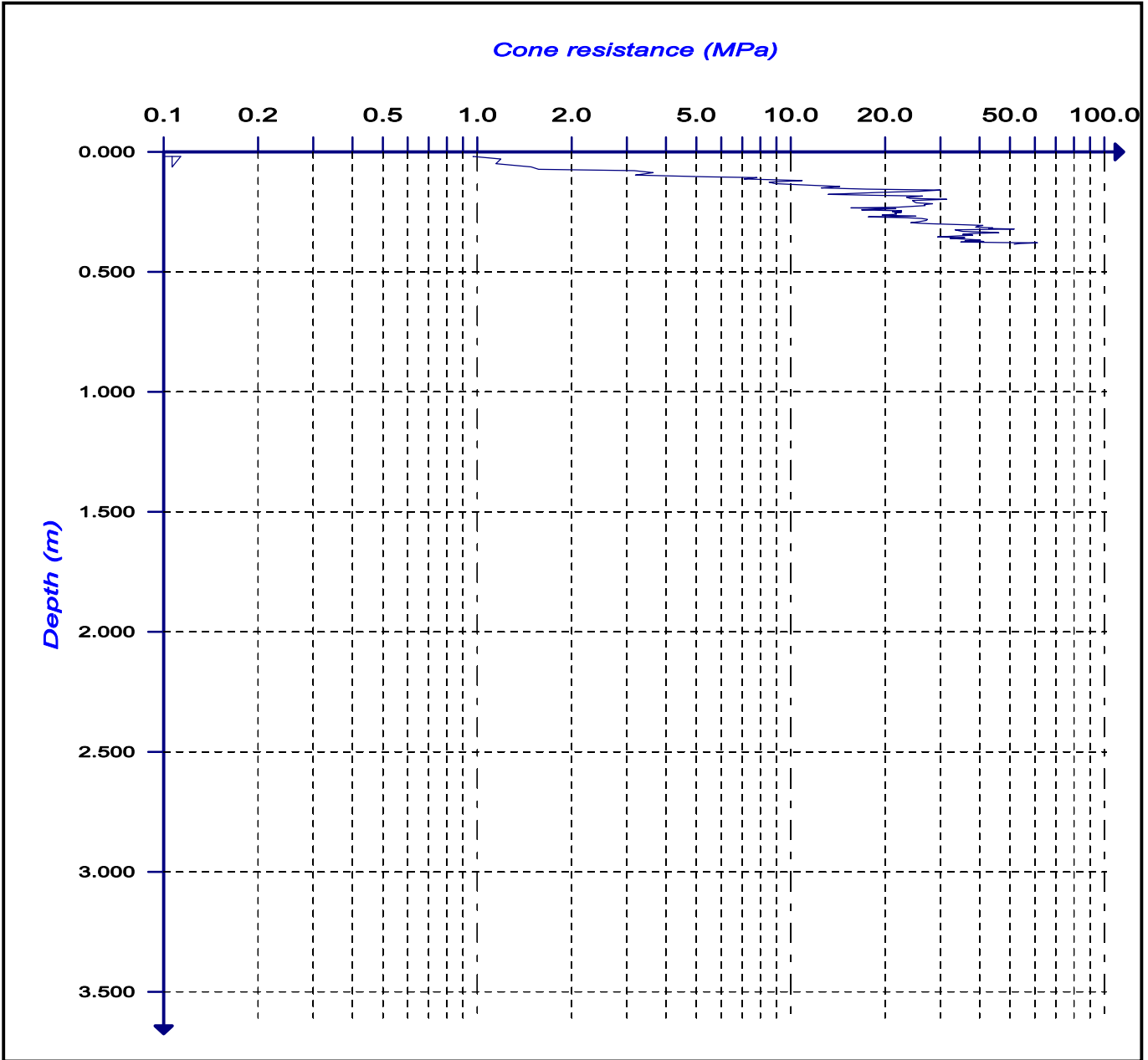
Marillana Creek Infiltration Project			
Sounding : S4-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

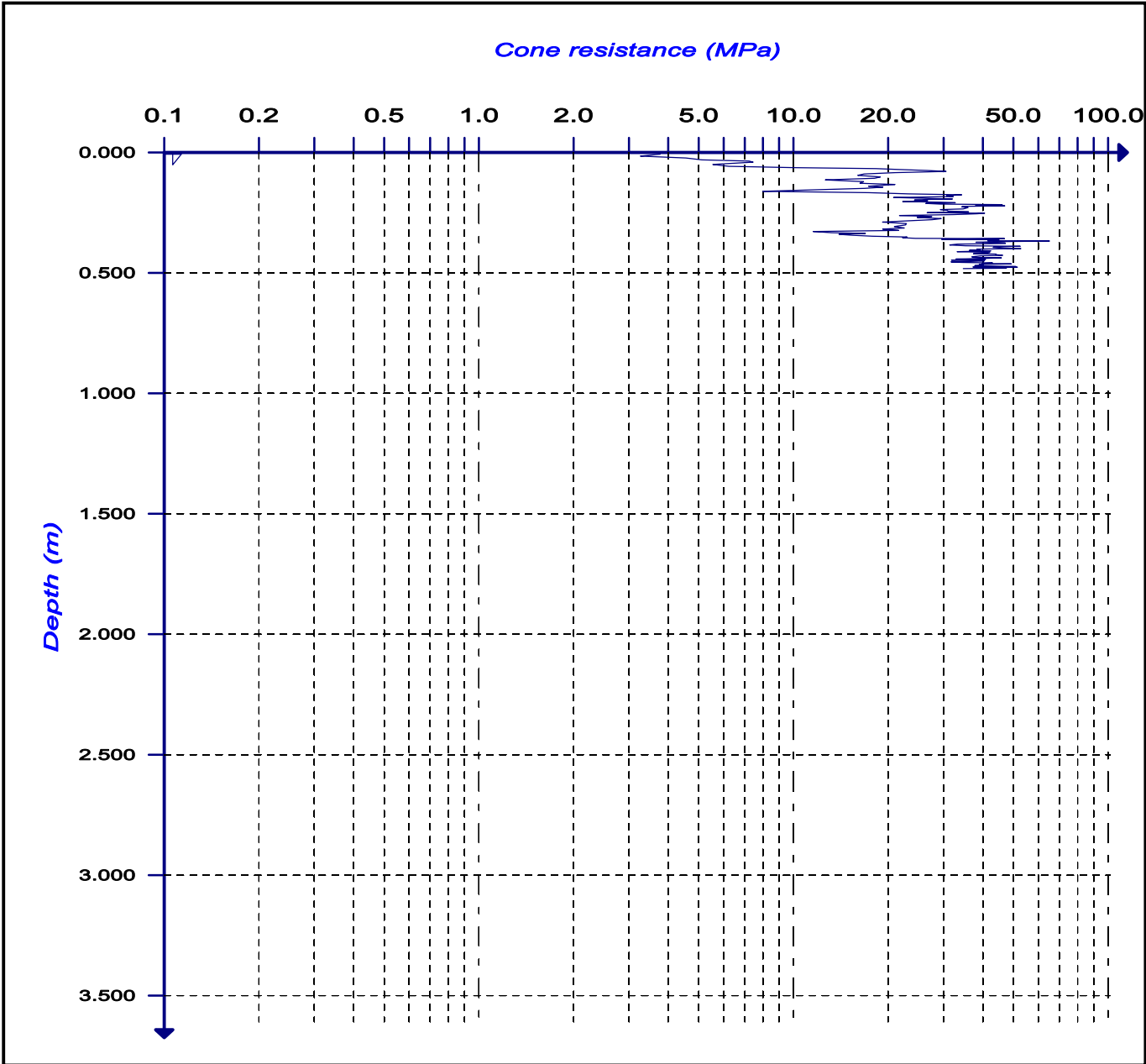
Marillana Creek Infiltration Project			
Sounding : S4-PM01 PDCP03A			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

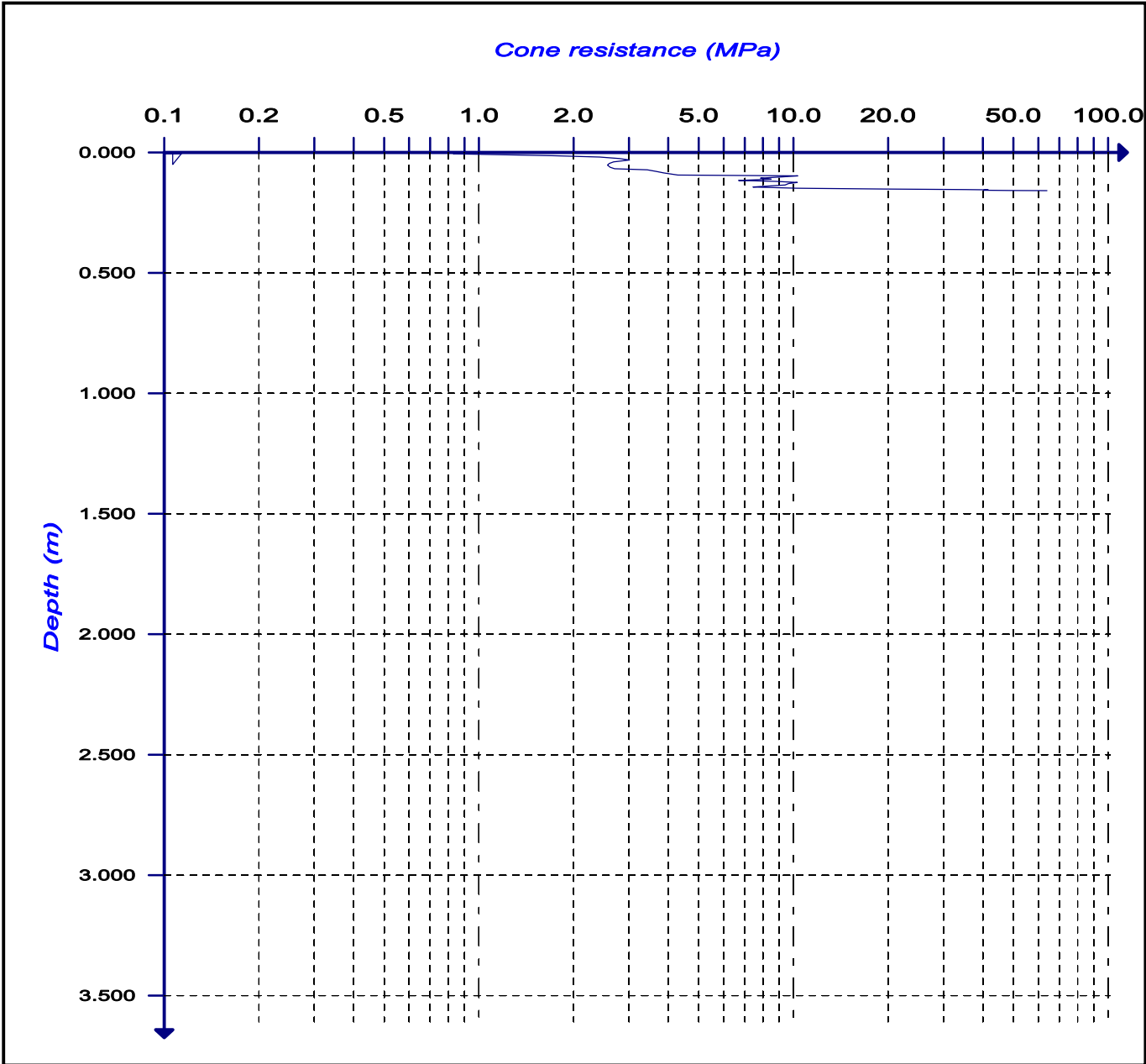
Marillana Creek Infiltration Project			
Sounding : S4-PM01 PDCP04			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

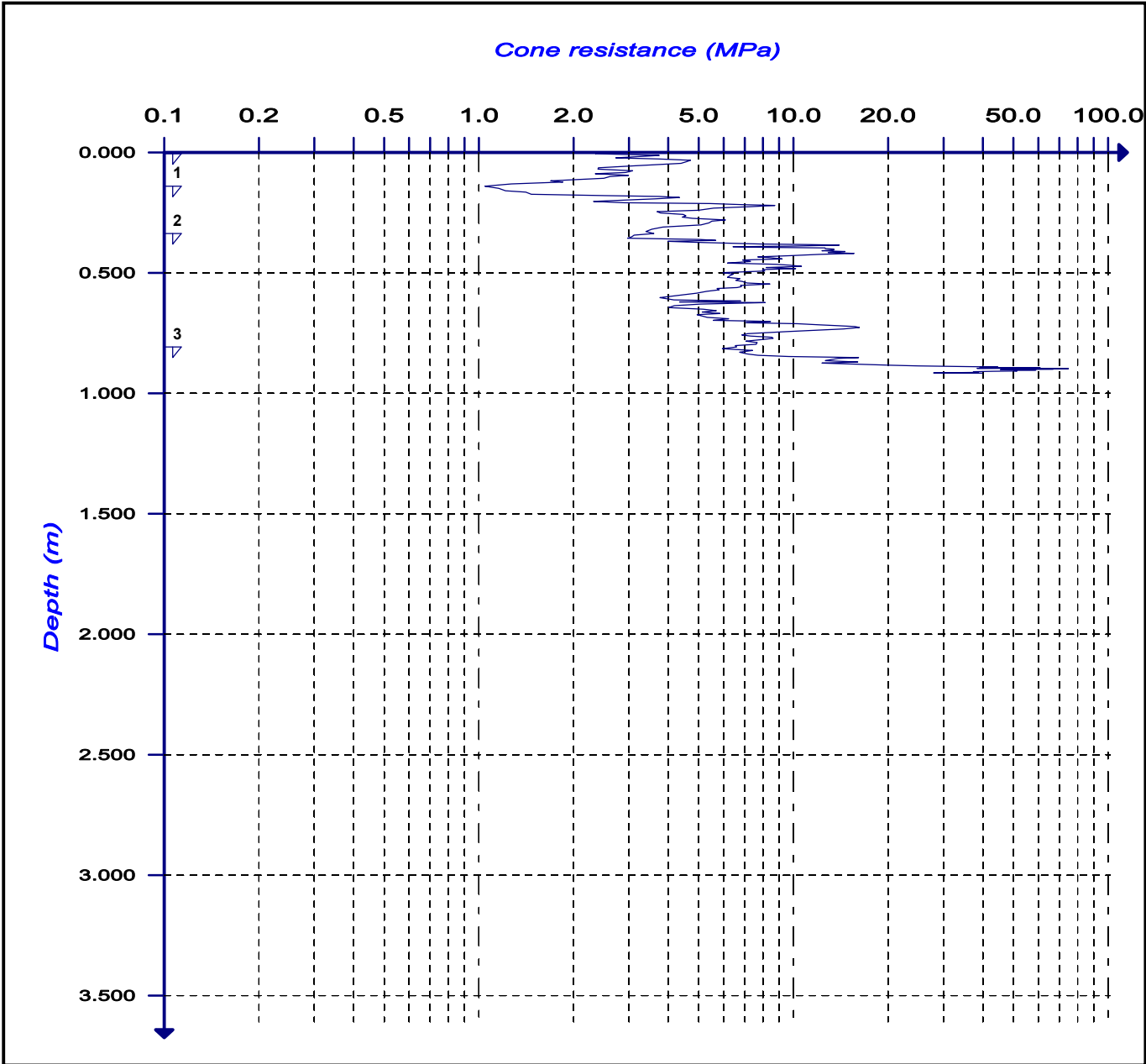
Marillana Creek Infiltration Project			
Sounding : S5-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

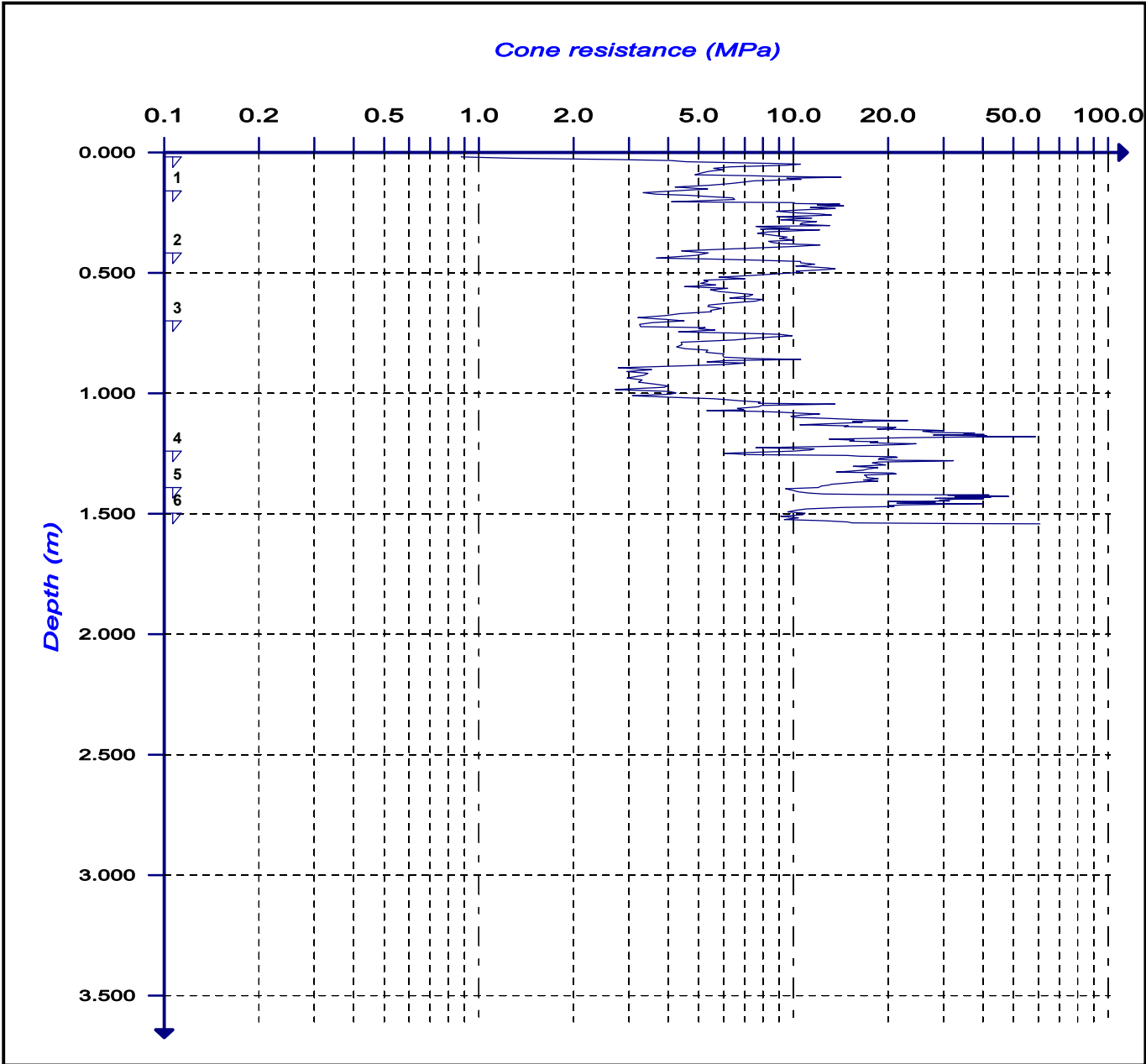
Marillana Creek Infiltration Project			
Sounding : S5-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	





# Ground investigation with variable energy dynamic penetrometer

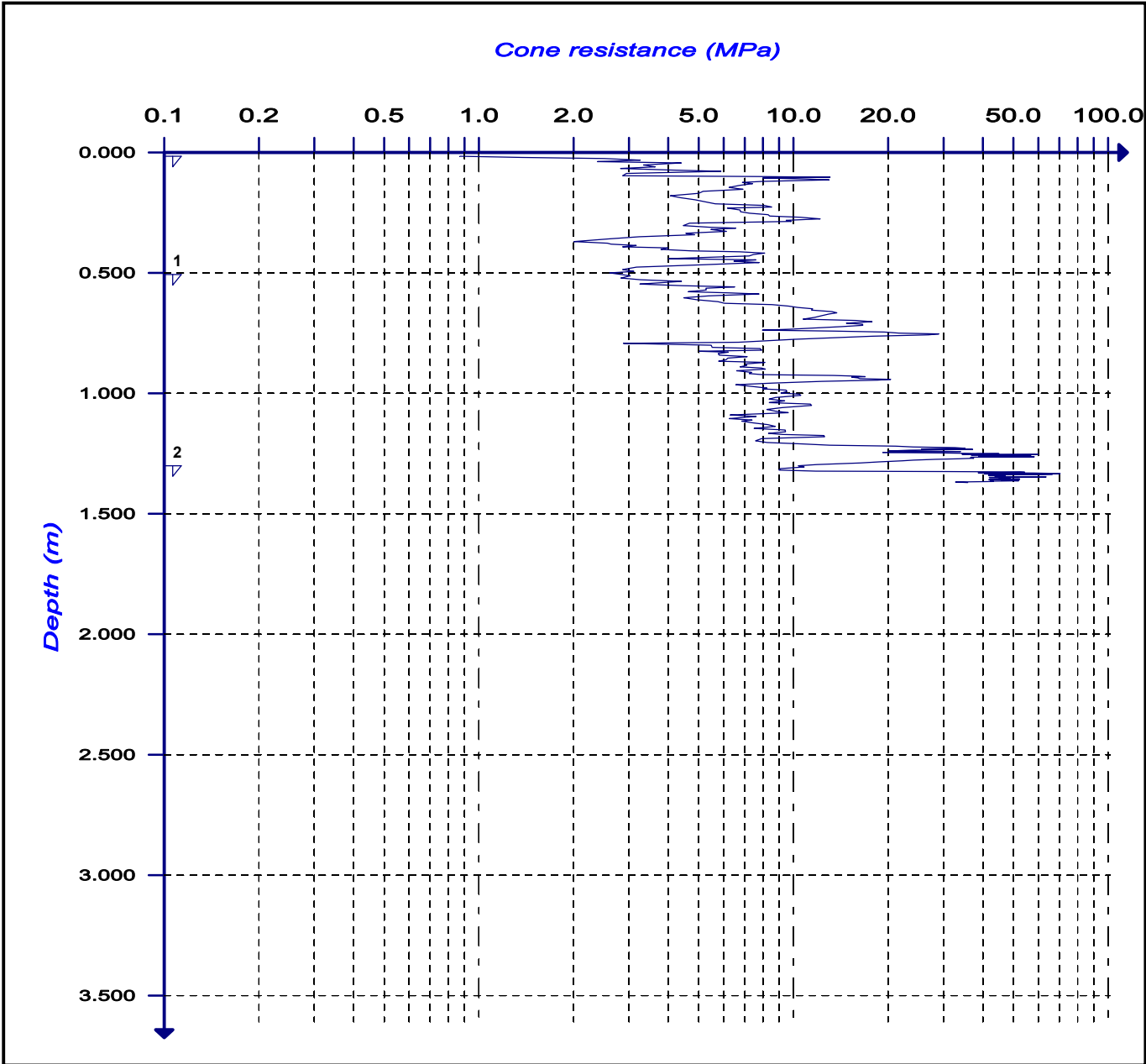
Marillana Creek Infiltration Project			
Sounding : S5-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	





# Ground investigation with variable energy dynamic penetrometer

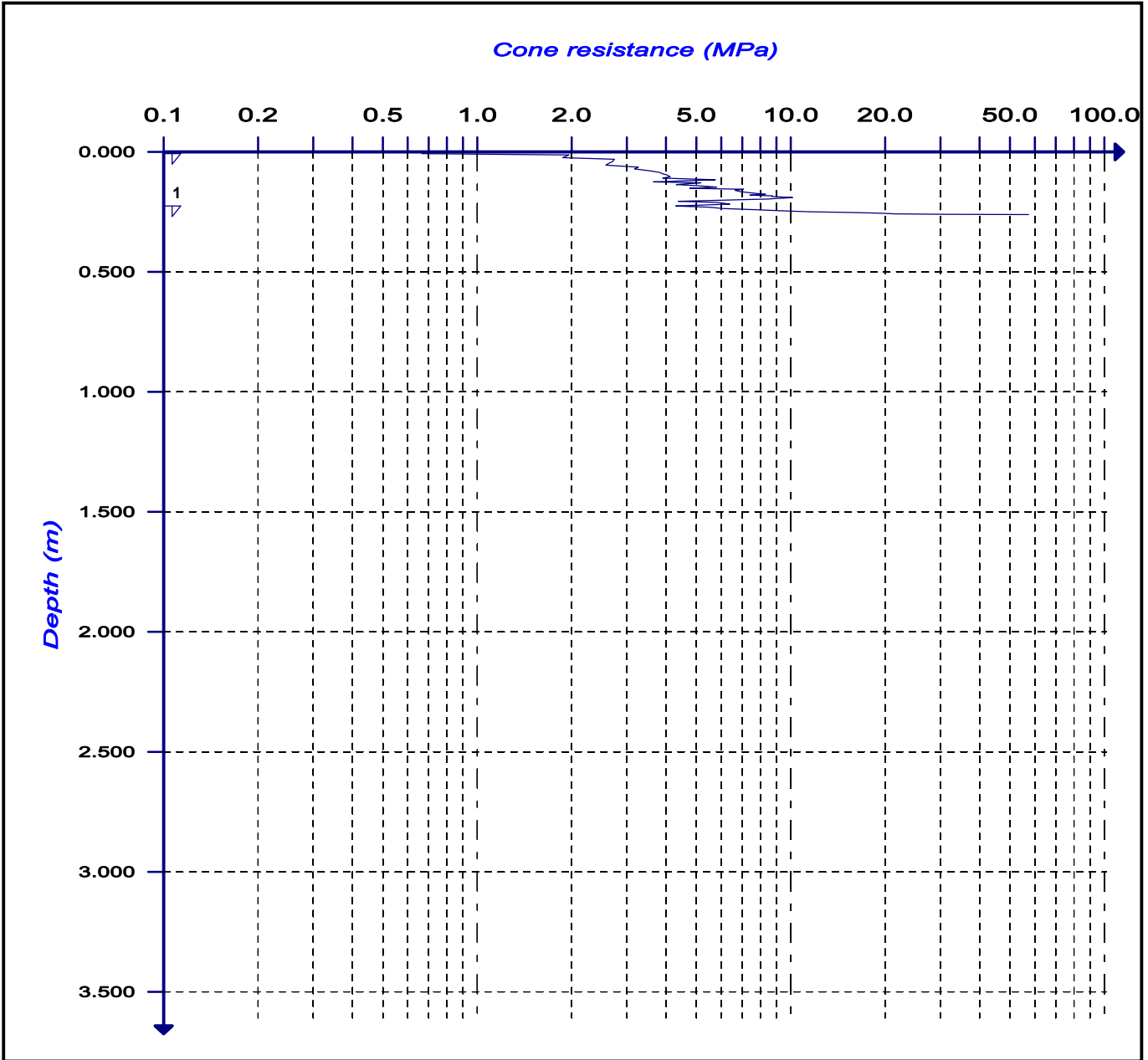
Marillana Creek Infiltration Project			
Sounding : S5-PM01 PDCP04			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	





# Ground investigation with variable energy dynamic penetrometer

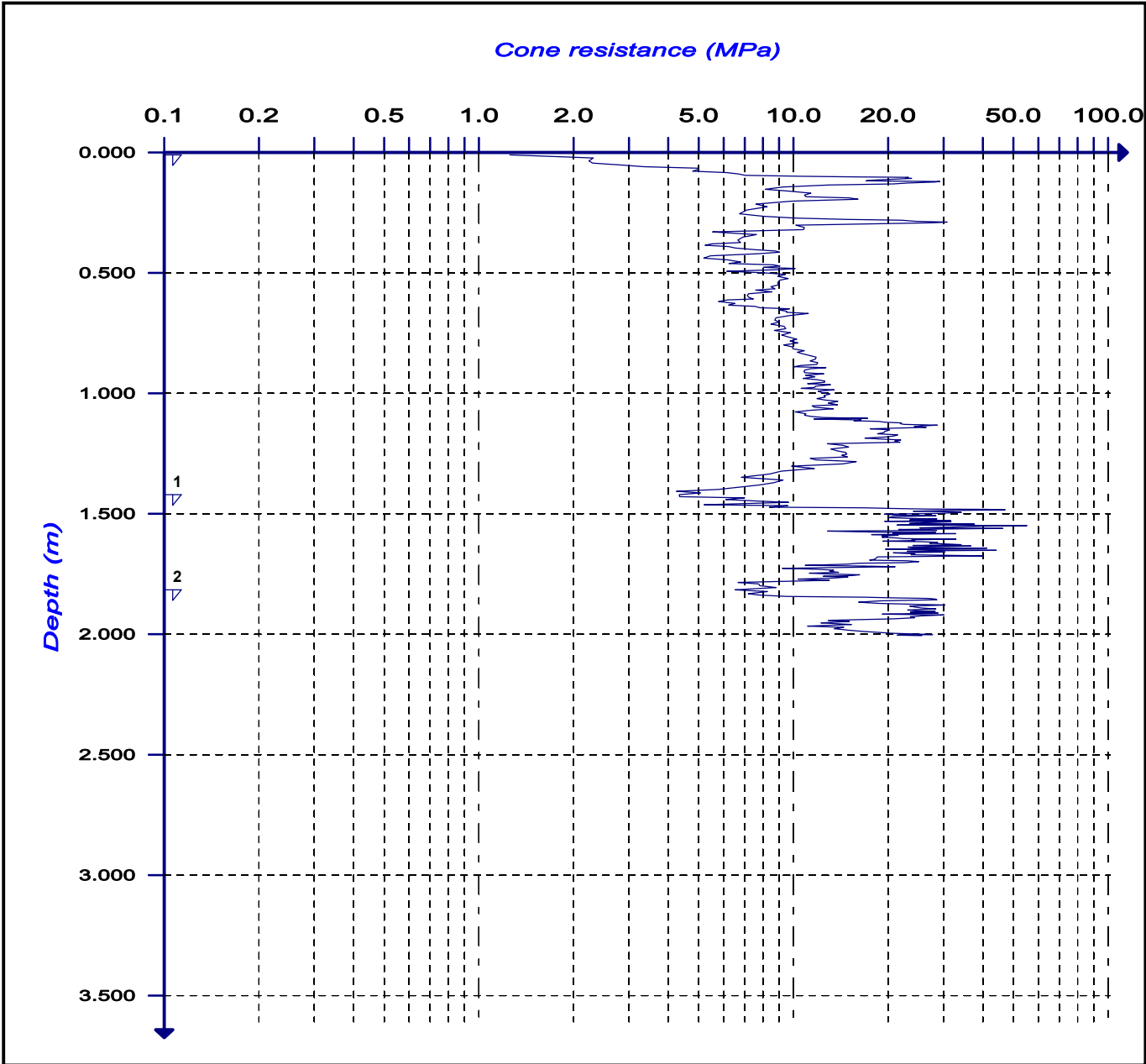
Marillana Creek Infiltration Project			
Sounding : S5-PM01 PDCP05			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

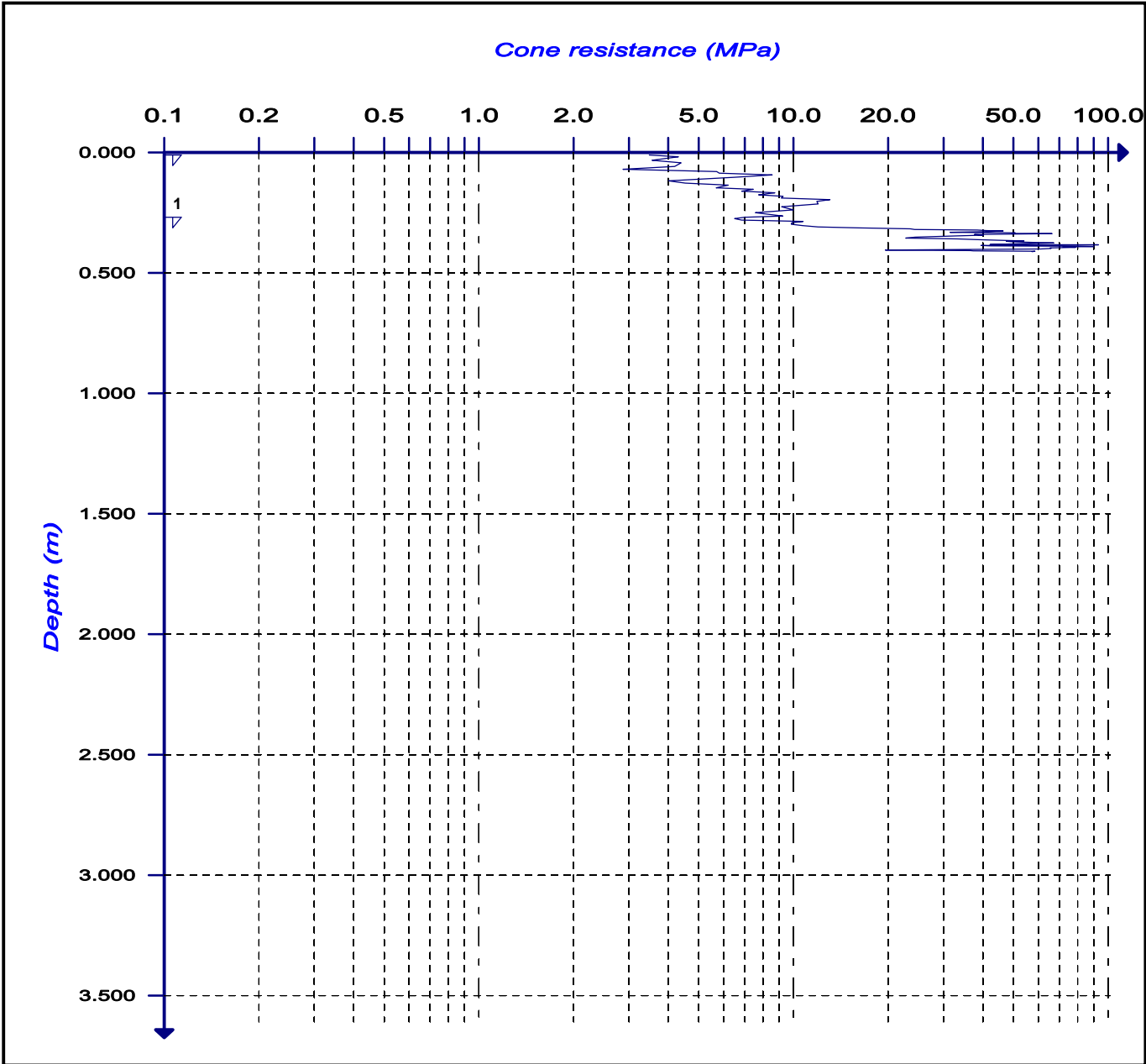
Marillana Creek Infiltration Project			
Sounding : S6-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

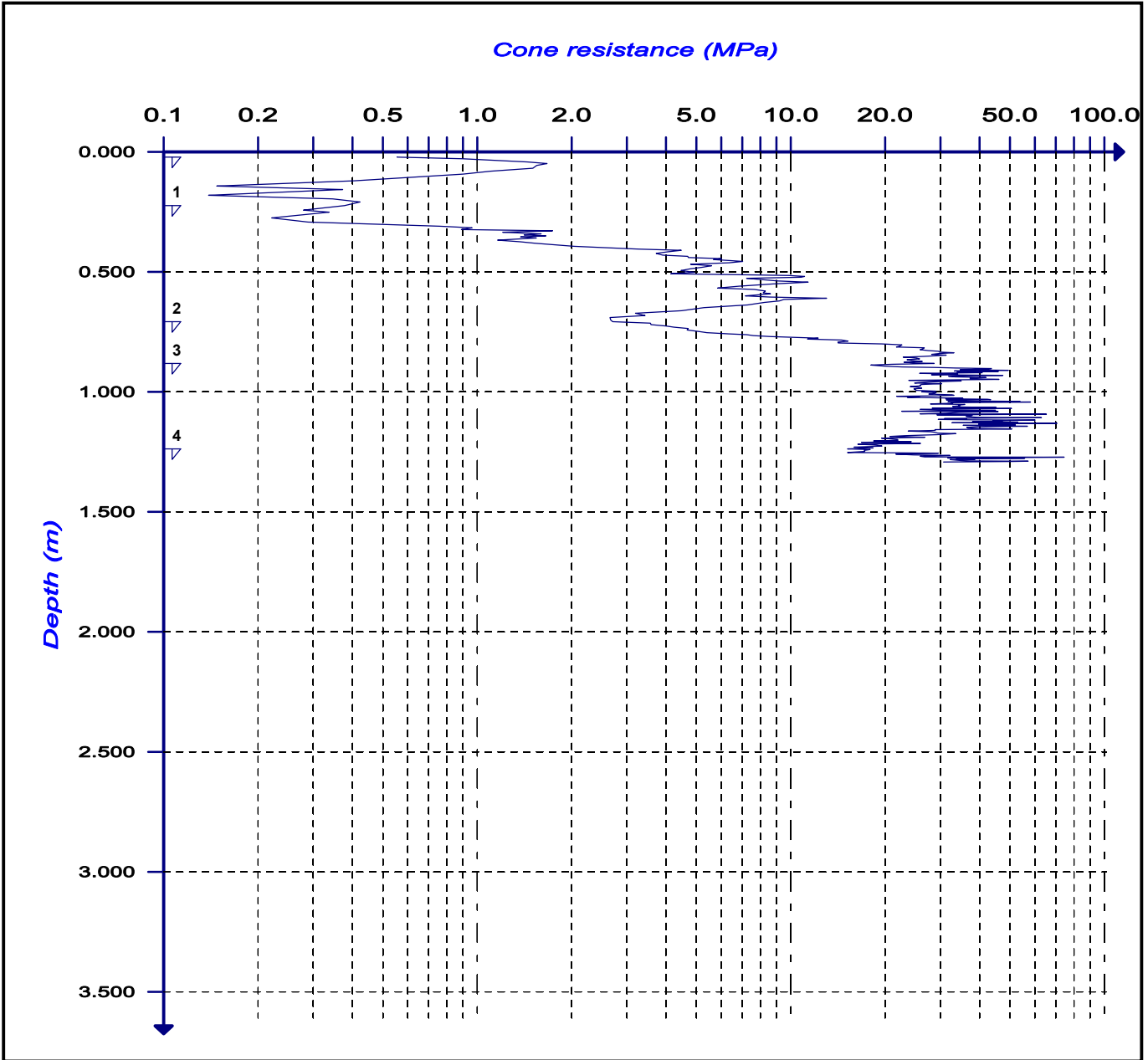
Marillana Creek Infiltration Project			
Sounding : S6-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

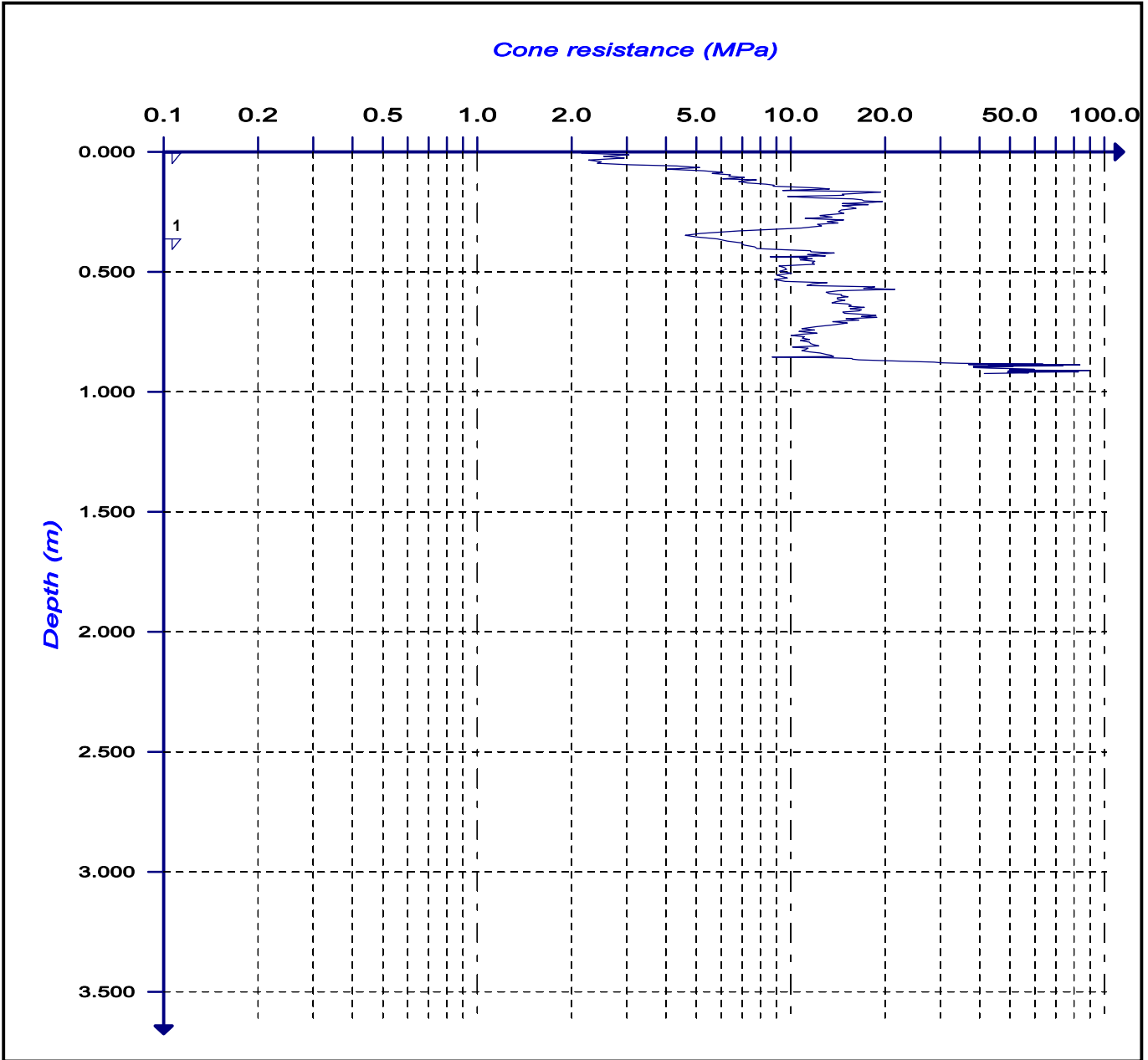
Marillana Creek Infiltration Project			
Sounding : S6-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 8/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

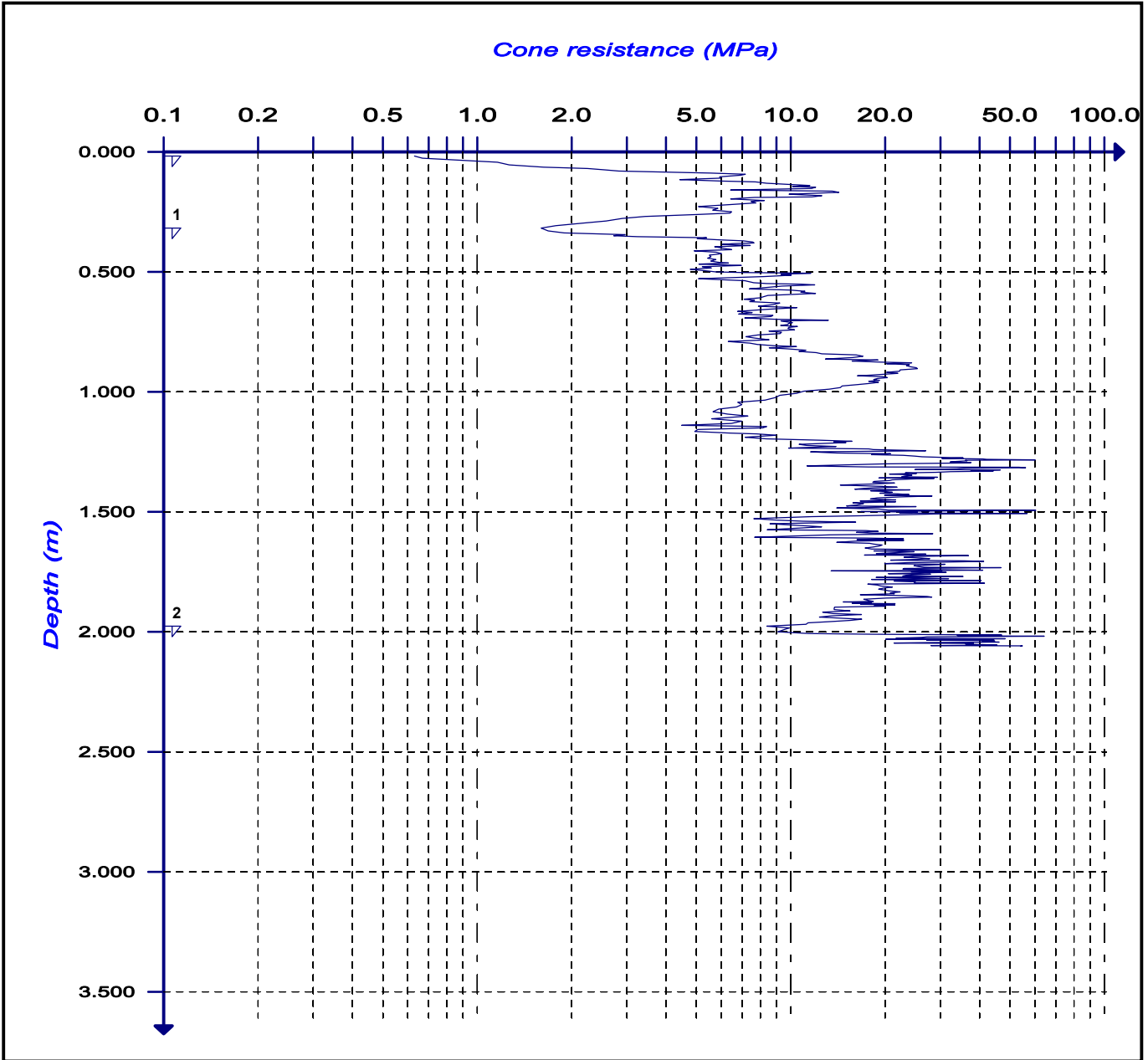
Marillana Creek Infiltration Project			
Sounding : S7-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 9/07/2019	





**Ground investigation with variable  
energy dynamic penetrometer**

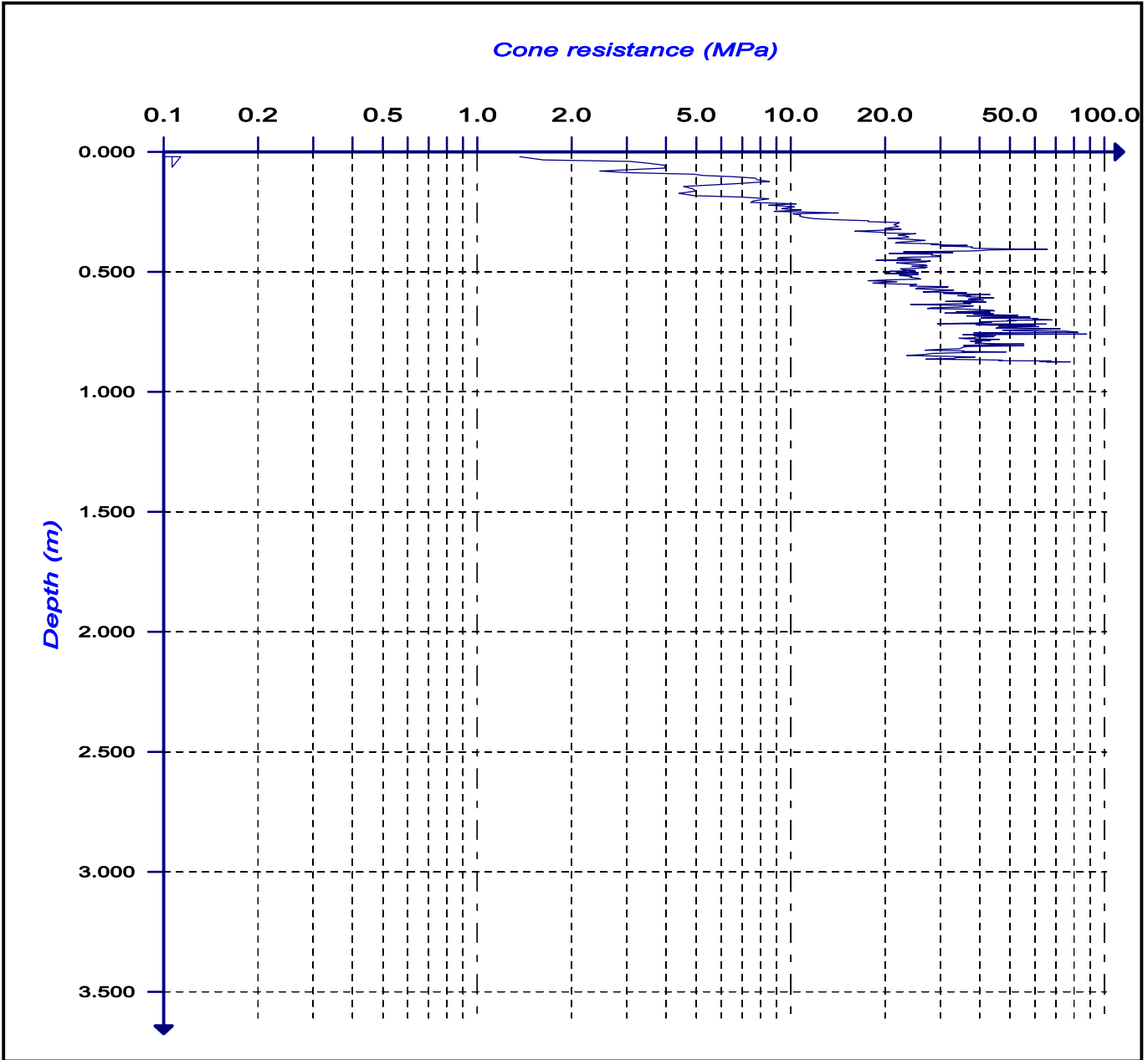
Marillana Creek Infiltration Project			
Sounding : S7-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 9/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

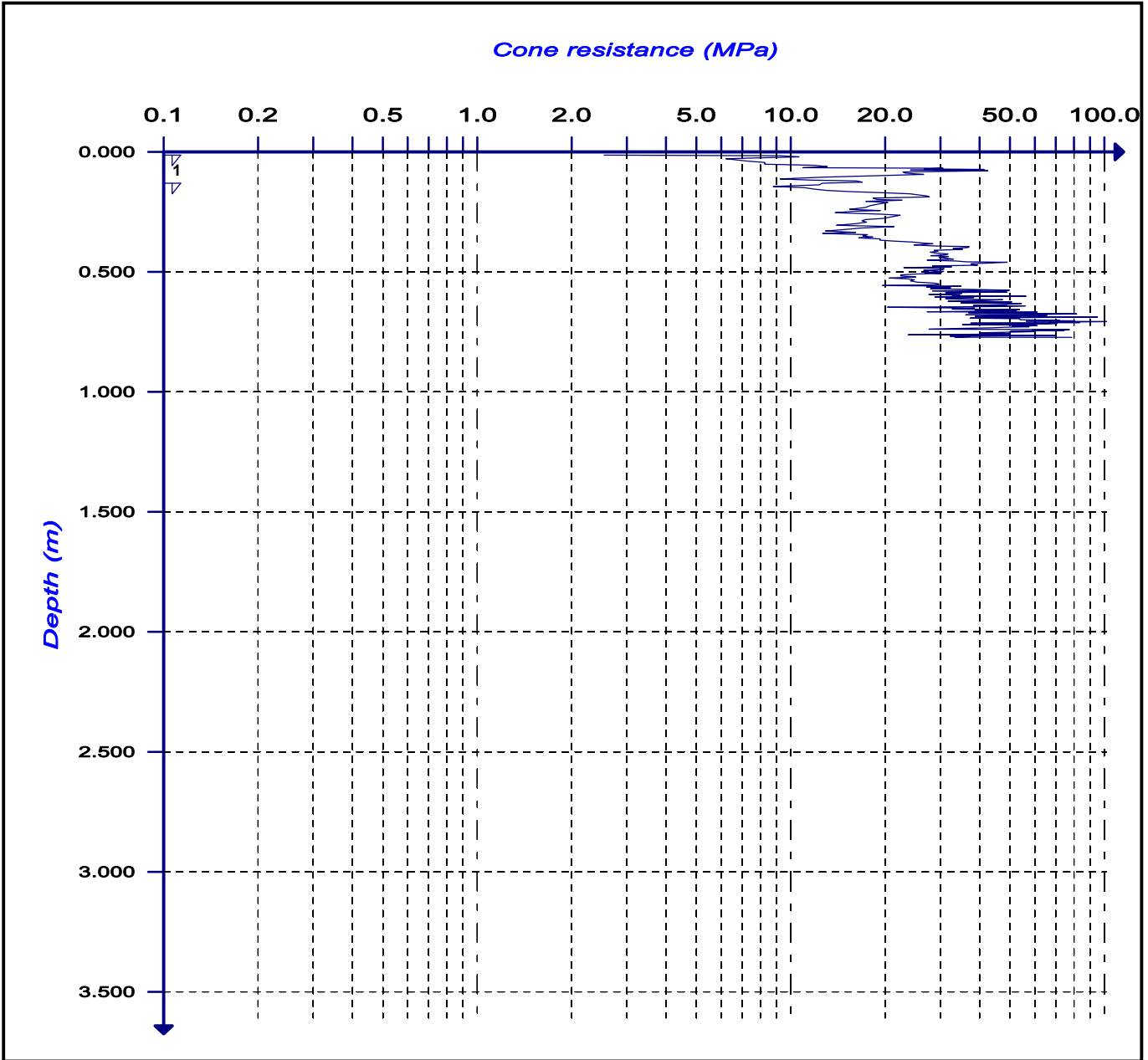
Marillana Creek Infiltration Project			
Sounding : S7-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 9/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

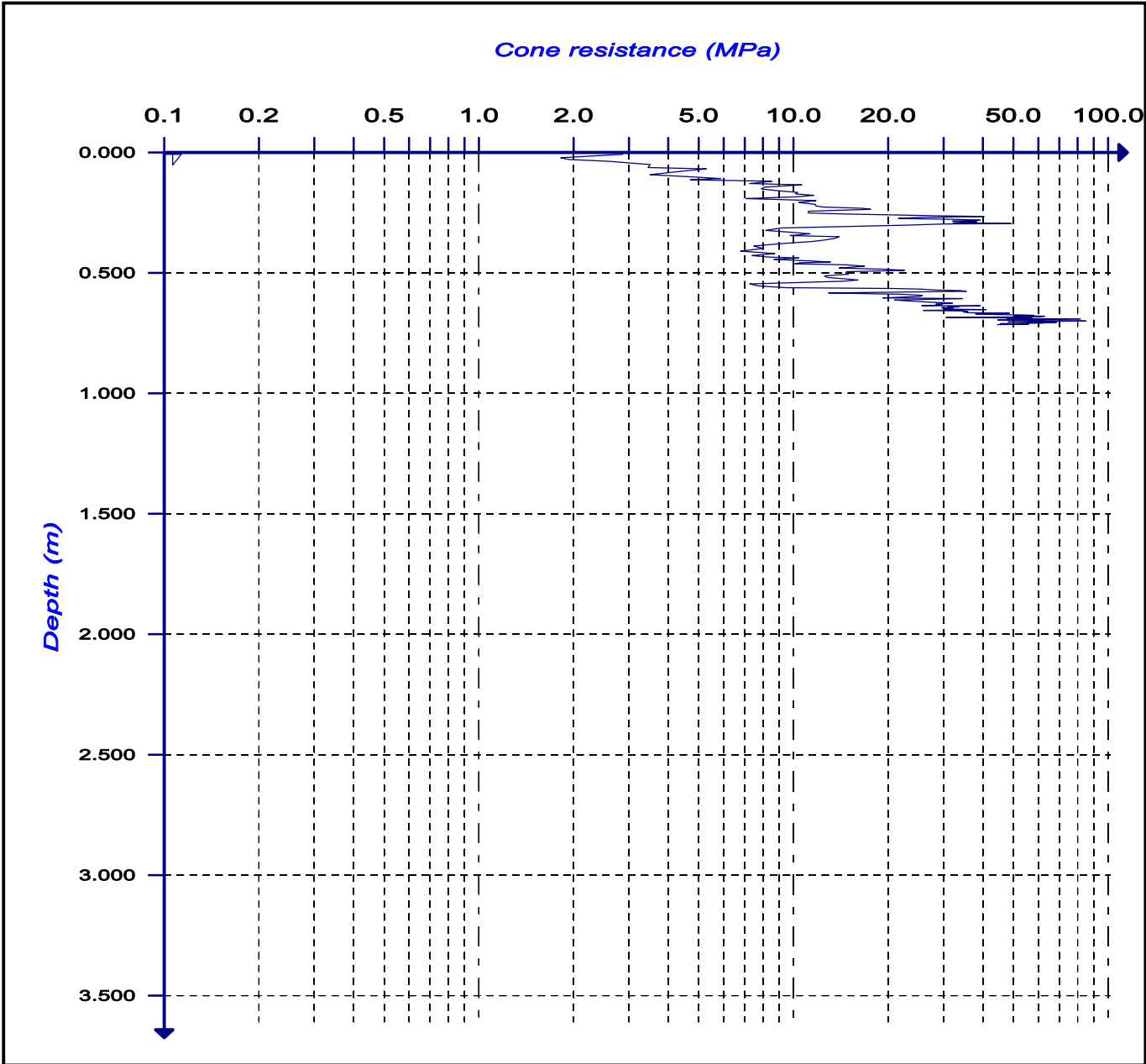
Marillana Creek Infiltration Project			
Sounding : S9-PM01 PDCP01			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	





# Ground investigation with variable energy dynamic penetrometer

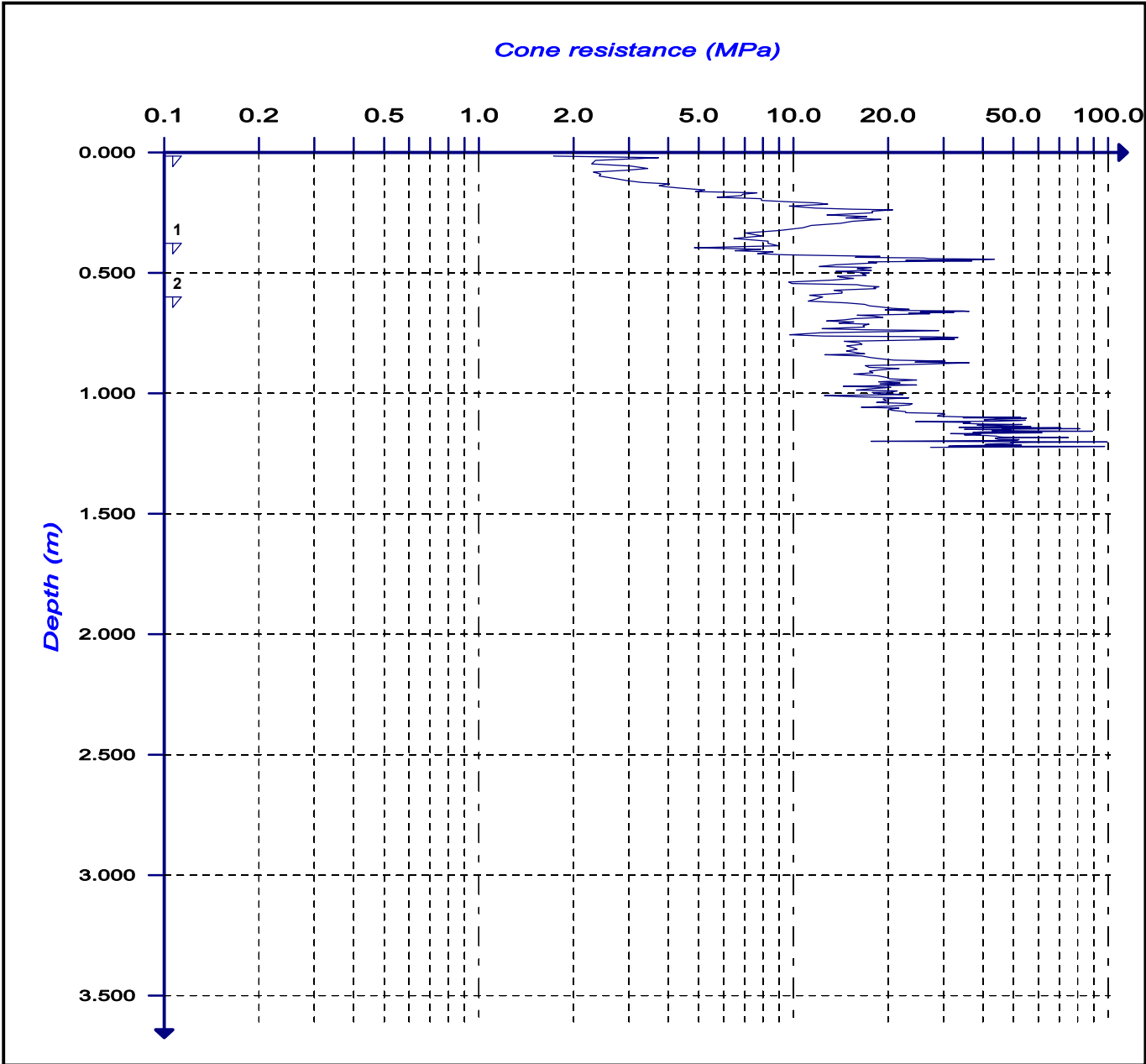
Marillana Creek Infiltration Project			
Sounding : S9-PM01 PDCP02			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	





**Ground investigation with variable energy dynamic penetrometer**

Marillana Creek Infiltration Project			
Sounding : S9-PM01 PDCP03			
Weight : Panda 2 hammer	Pre-sounding depth : 0.000 m	Area : 2 cm <sup>2</sup>	Water table : Not encountered
Operator : MAS	Breaking cond. : Refusal	Date : 10/07/2019	



## APPENDIX H LABORATORY TEST RESULTS – SOIL TESTING

# TEST REPORT

Client: **BHP**

SAMPLE ID: **S1-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

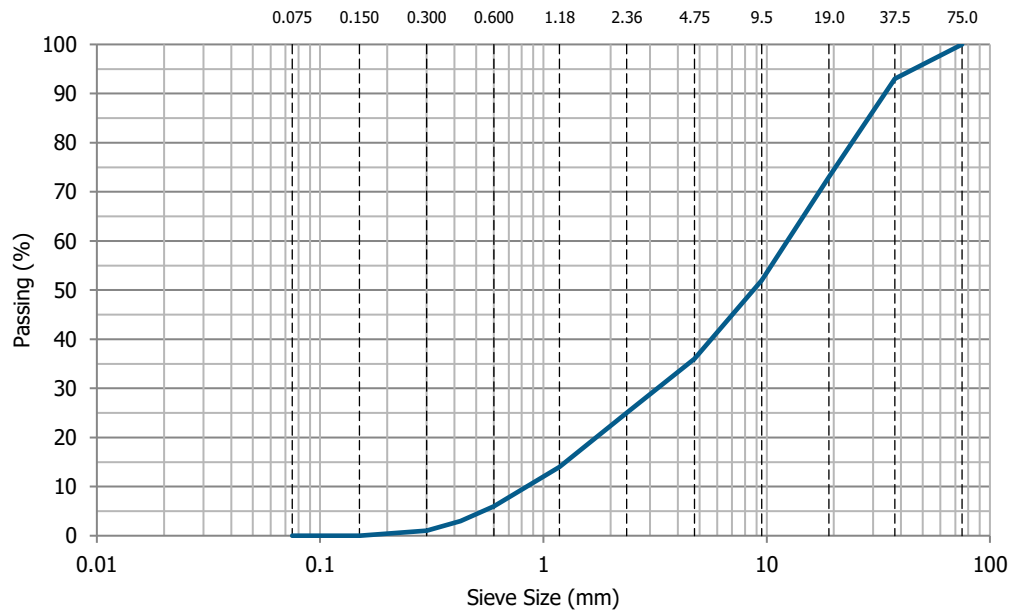
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0556



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Wangara, WA 6065

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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	14
37.5	93	0.600	6
19.0	73	0.425	3
9.5	52	0.300	1
4.75	36	0.150	0
2.36	25	0.075	0

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0556 PSD  
Page: 1 of 1

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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S1-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

Lab test request: 19-032  
Lab location: Wangara  
Date tested: 25/07/2019  
Lab sample ID: WG19-0556

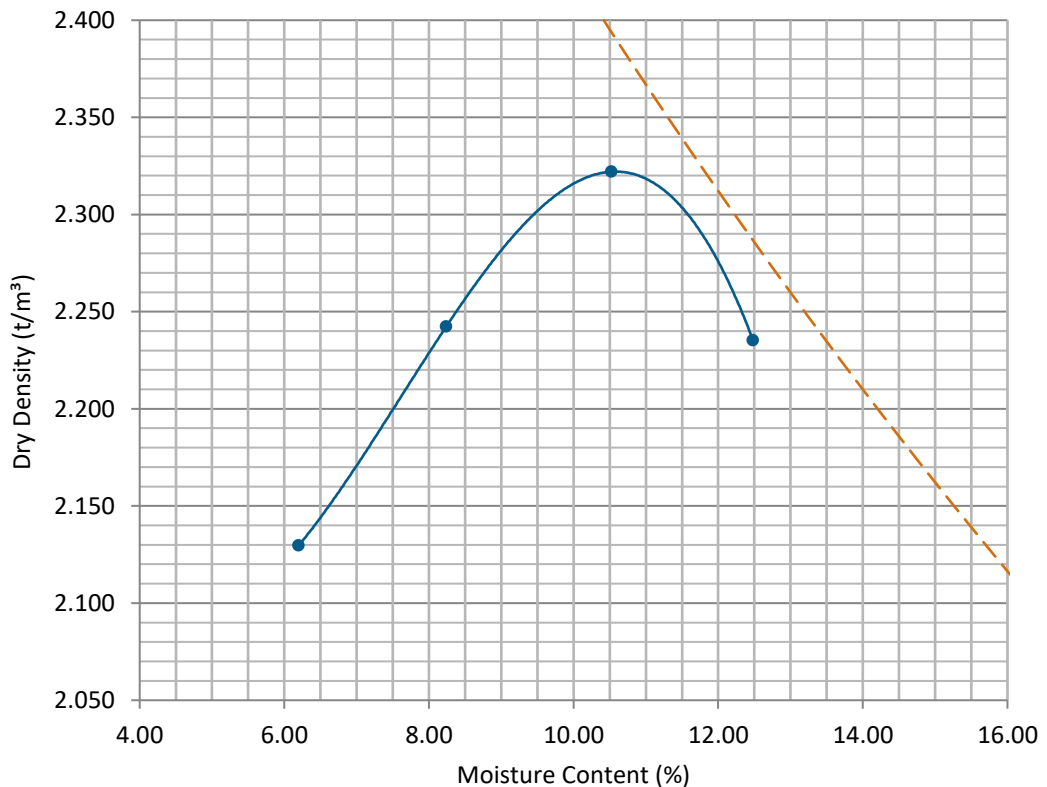


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## Dry Density / Moisture Content Relationship

Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.32** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **10.5** (%)


RETAINED ON 19.0mm SIEVE: 27 (%)

RETAINED ON 37.5mm SIEVE: 7 (%)

AIR VOIDS LINE: 0 (%)

### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.20
2. Where further analysis is required, MDD and OMC values of 2.322t/m<sup>3</sup> and 10.6% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 48 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 26/07/2019  
Test Report No.: 19-032-0556 SMDD  
Page: 1 of 1

Deviation from Standard: Insufficient material to perform the required 'Mould B' compaction. 'Mould A' was used and only material <19mm was tested, as directed by the Client.

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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S1-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019  
Lab sample ID: WG19-0556



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## WA915.1 Calcium Carbonate Content

CALCIUM CARBONATE CONTENT (CaCO<sub>3</sub>): **50.3** (%)

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 1/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0556 CCC  
Page: 1 of 1

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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S2-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

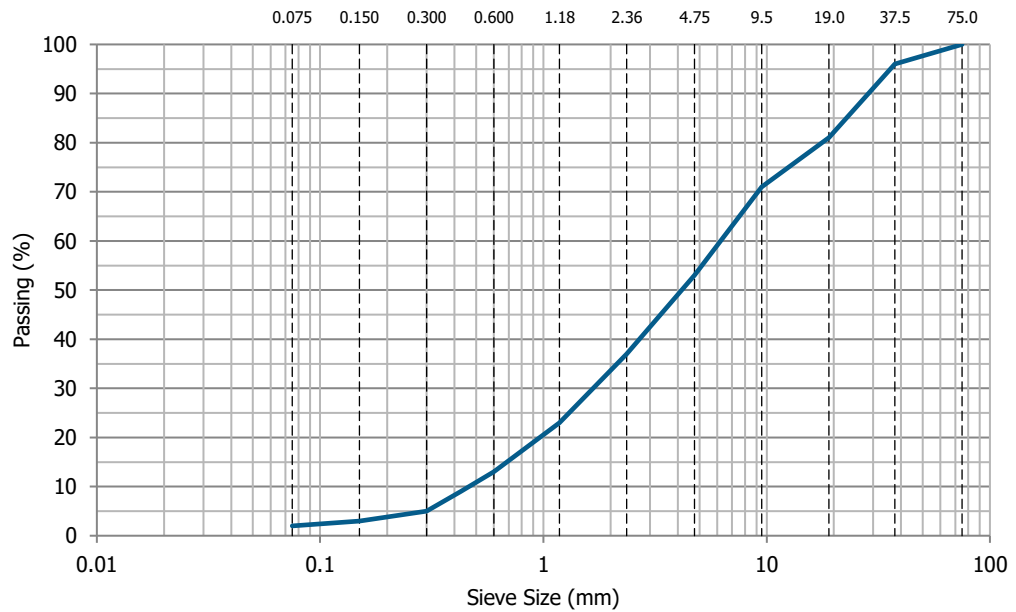
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0557



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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	23
37.5	96	0.600	13
19.0	81	0.425	9
9.5	71	0.300	5
4.75	53	0.150	3
2.36	37	0.075	2

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0557 PSD  
Page: 1 of 1

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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S2-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

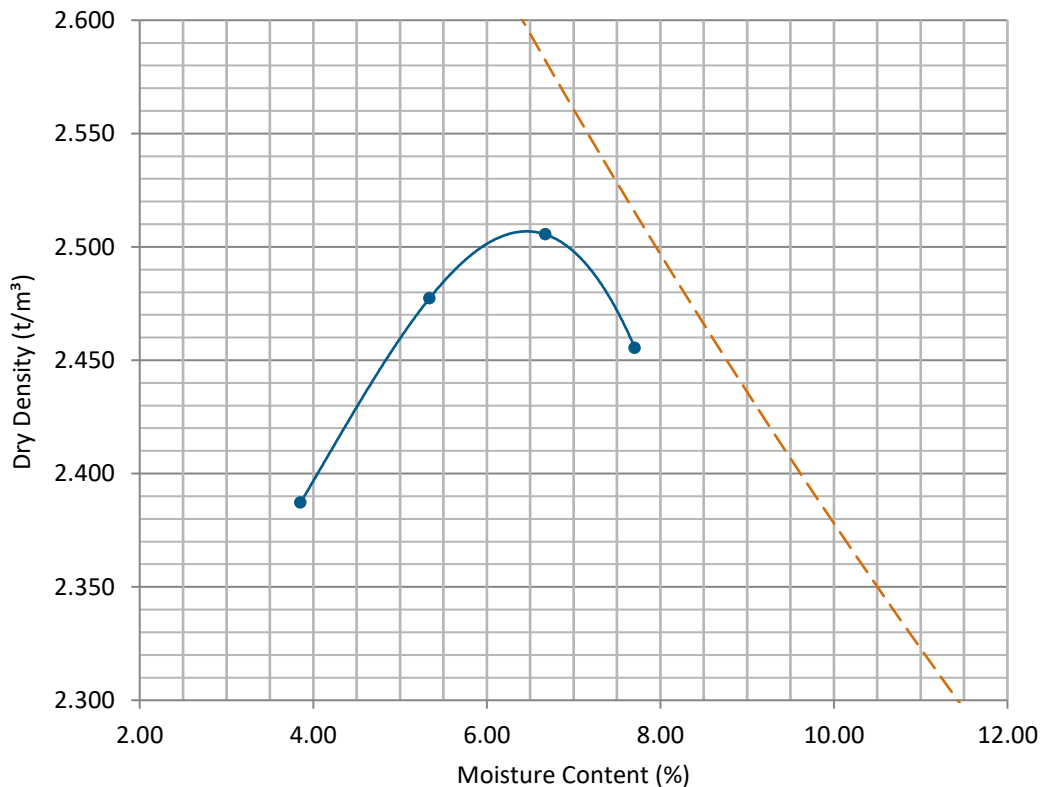
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 24/07/2019  
Lab sample ID: WG19-0557



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### AS1289.5.1.1 Dry Density / Moisture Content Relationship Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.51** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **6.5** (%)

RETAINED ON 19.0mm SIEVE: 19 (%)

RETAINED ON 37.5mm SIEVE: 4 (%)

AIR VOIDS LINE: 0 (%)

#### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.12
2. Where further analysis is required, MDD and OMC values of 2.507t/m<sup>3</sup> and 6.5% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 25/07/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0557 SMDD  
Page: 1 of 1

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## TEST REPORT

Client: **BHP**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

Lab test request: 19-032  
Lab location: Wangara  
Date tested: 22/07/2019  
Lab sample ID: WG19-0557



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### AS1289.2.1.1 Moisture Content

Oven Method

CLIENT SAMPLE ID	DEPTH	MOISTURE CONTENT (%)	LAB SAMPLE ID
S2-PM01	0.05m - 0.25m	<b>5.0</b>	WG19-0557

#### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0557 MC  
Page: 1 of 1

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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S2-PM02**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

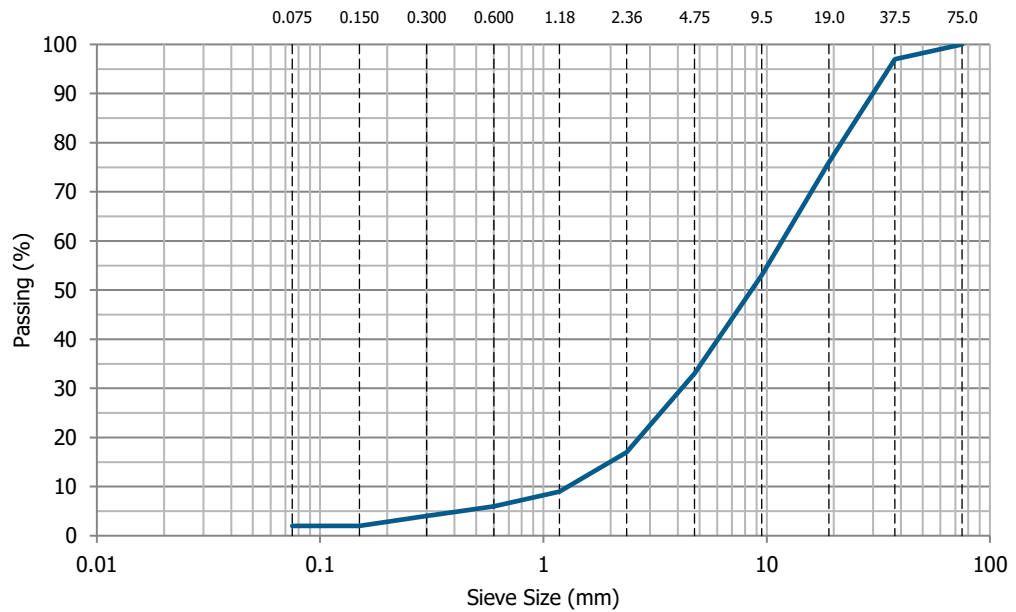
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0558



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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	9
37.5	97	0.600	6
19.0	76	0.425	5
9.5	53	0.300	4
4.75	33	0.150	2
2.36	17	0.075	2

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0558 PSD  
Page: 1 of 1

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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S2-PM02**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

Lab test request: 19-032  
Lab location: Wangara  
Date tested: 25/07/2019  
Lab sample ID: WG19-0558

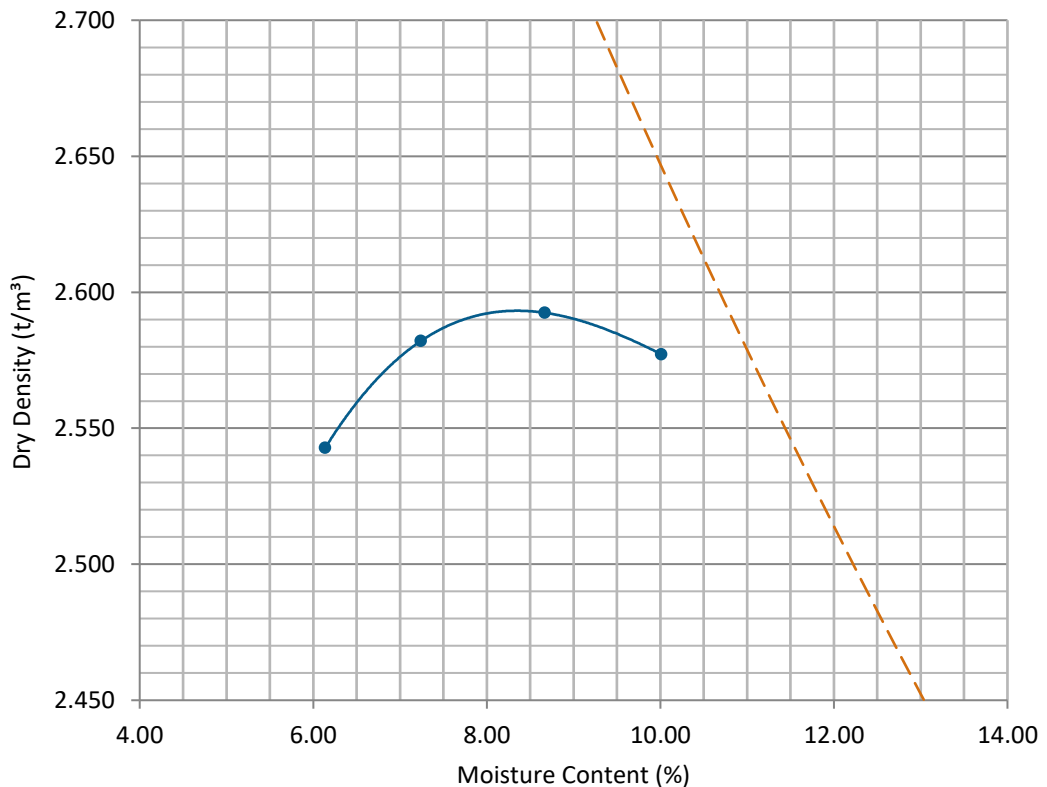


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## Dry Density / Moisture Content Relationship

Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.59** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **8.5** (%)


RETAINED ON 19.0mm SIEVE: 24 (%)

RETAINED ON 37.5mm SIEVE: 3 (%)

AIR VOIDS LINE: 0 (%)

### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.60
2. Where further analysis is required, MDD and OMC values of 2.593t/m<sup>3</sup> and 8.4% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 26/07/2019  
Test Report No.: 19-032-0558 SMDD  
Page: 1 of 1

Deviation from Standard: Insufficient material to perform the required 'Mould B' compaction. 'Mould A' was used and only material <19mm was tested, as directed by the Client.

These results apply only to the material tested, as received. Please refer to 'EMC Terms and Conditions'.

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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S3-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

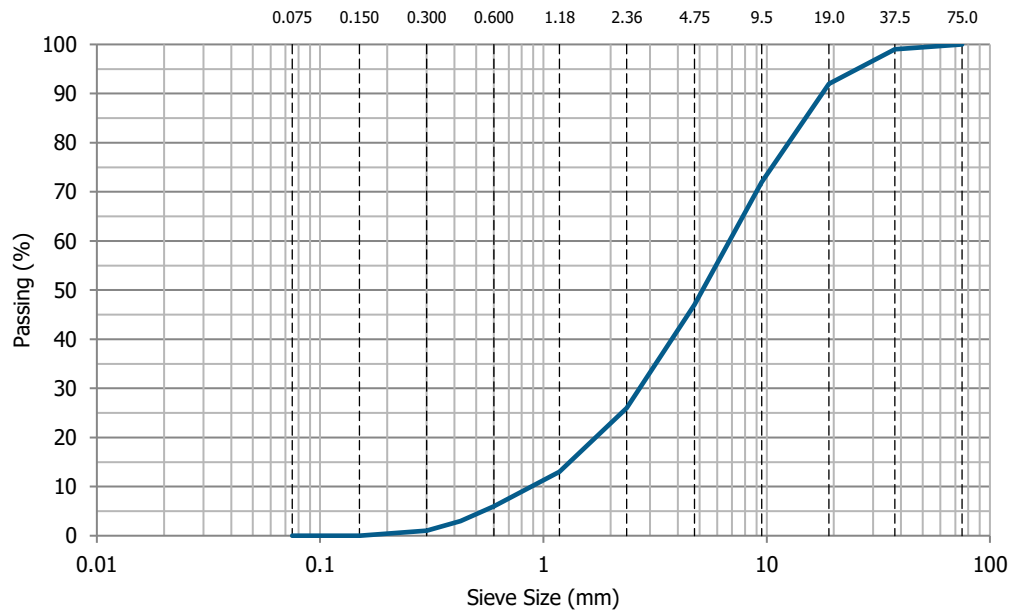
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0559



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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	13
37.5	99	0.600	6
19.0	92	0.425	3
9.5	72	0.300	1
4.75	47	0.150	0
2.36	26	0.075	0

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0559 PSD  
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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S3-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

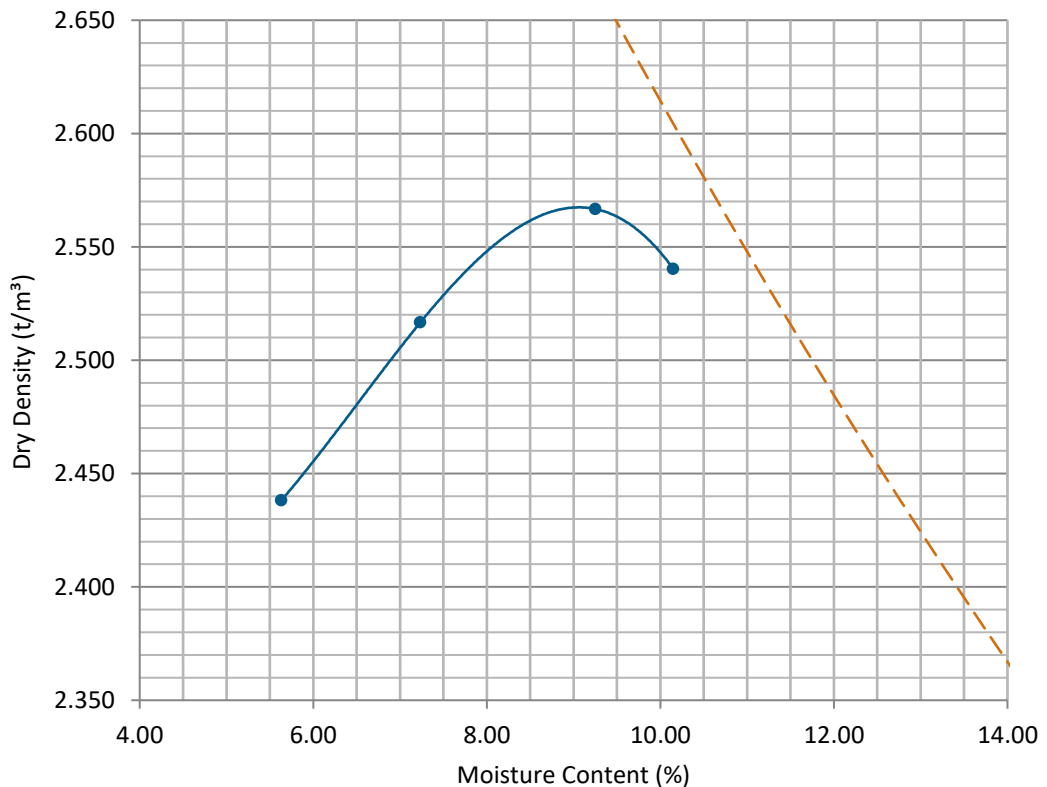
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 24/07/2019  
Lab sample ID: WG19-0559



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### AS1289.5.1.1 Dry Density / Moisture Content Relationship Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.57** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **9.0** (%)

RETAINED ON 19.0mm SIEVE: 8 (%)

RETAINED ON 37.5mm SIEVE: 1 (%)

AIR VOIDS LINE: 0 (%)

#### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.54
2. Where further analysis is required, MDD and OMC values of 2.567t/m<sup>3</sup> and 9.1% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 25/07/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0559 SMDD  
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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S4-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

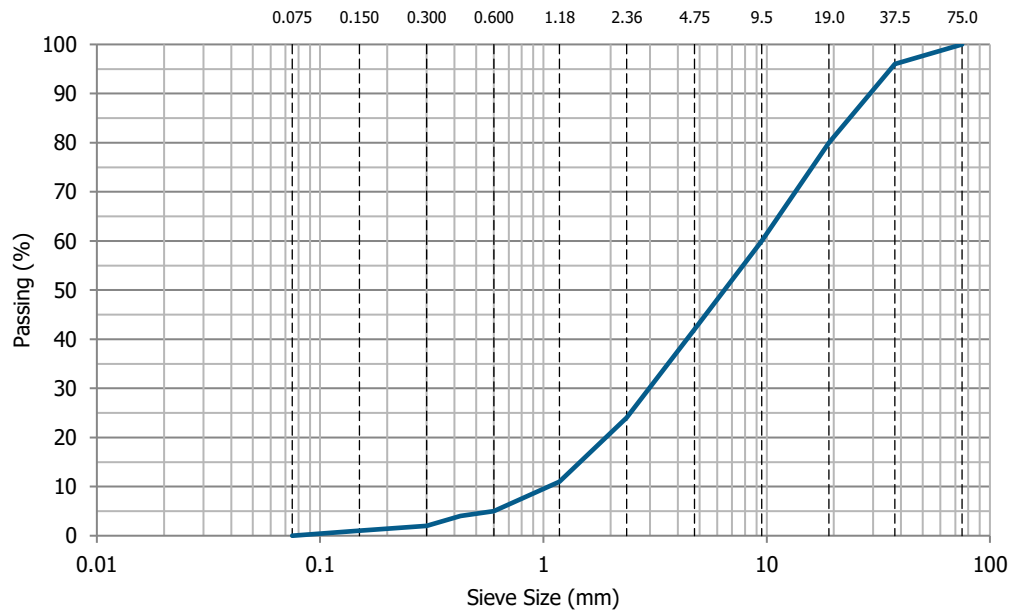
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0560



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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	11
37.5	96	0.600	5
19.0	80	0.425	4
9.5	60	0.300	2
4.75	42	0.150	1
2.36	24	0.075	0

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0560 PSD  
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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S4-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

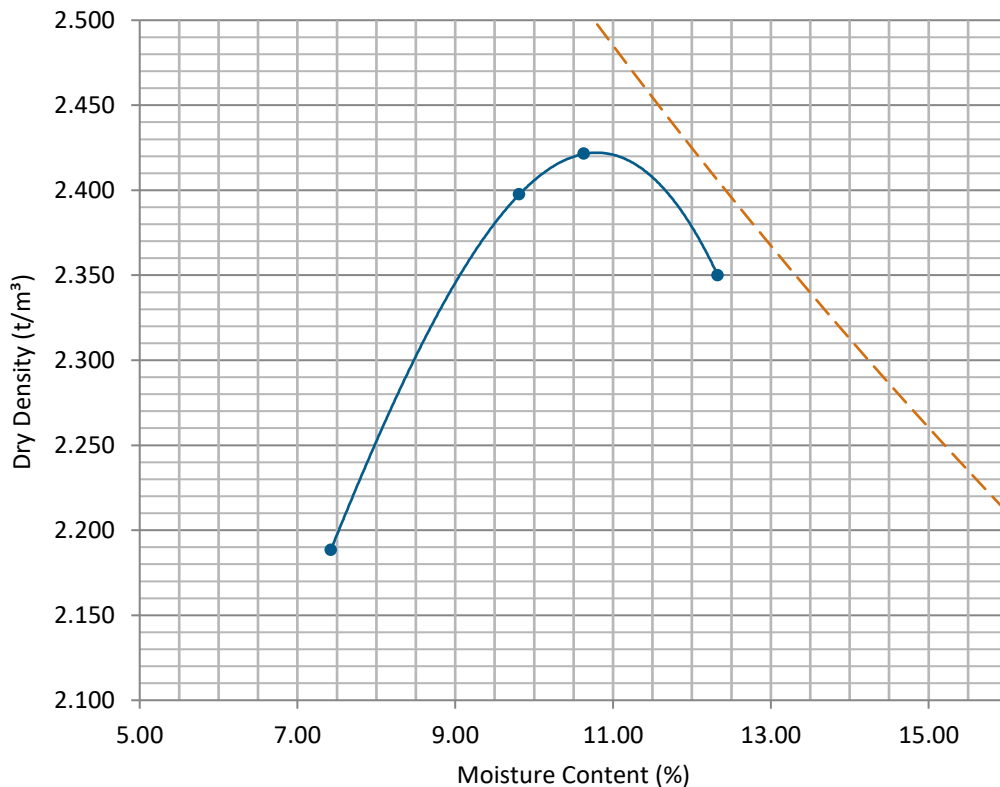
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 24/07/2019  
Lab sample ID: WG19-0560



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### AS1289.5.1.1 Dry Density / Moisture Content Relationship Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.42** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **11.0** (%)

RETAINED ON 19.0mm SIEVE: 20 (%)

RETAINED ON 37.5mm SIEVE: 4 (%)

AIR VOIDS LINE: 0 (%)

#### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.42
2. Where further analysis is required, MDD and OMC values of 2.422t/m<sup>3</sup> and 10.8% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 25/07/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0560 SMDD  
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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S5-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

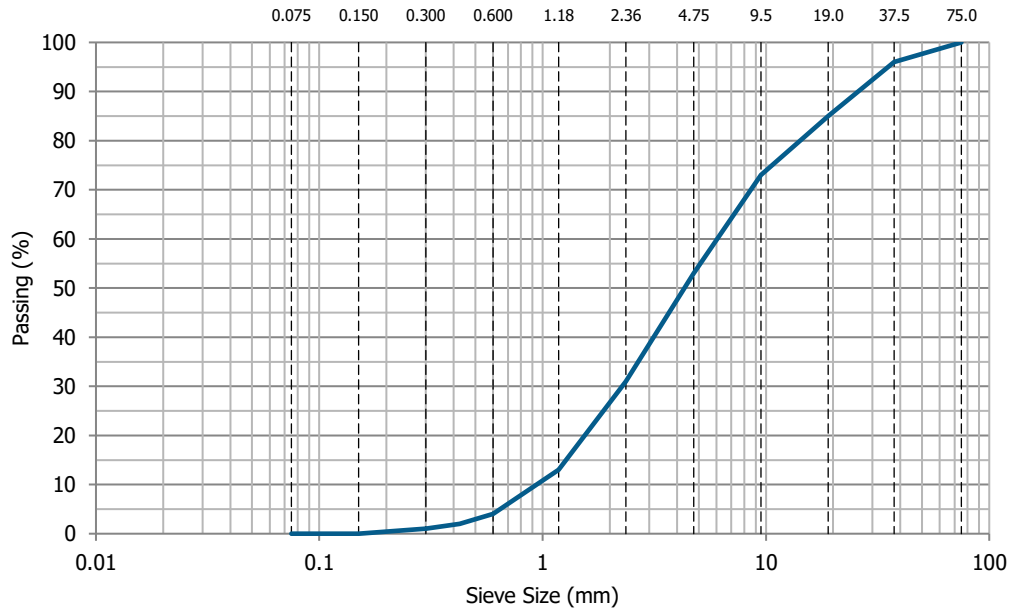
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0561



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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	13
37.5	96	0.600	4
19.0	85	0.425	2
9.5	73	0.300	1
4.75	53	0.150	0
2.36	31	0.075	0

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 1/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0561 PSD  
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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S5-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

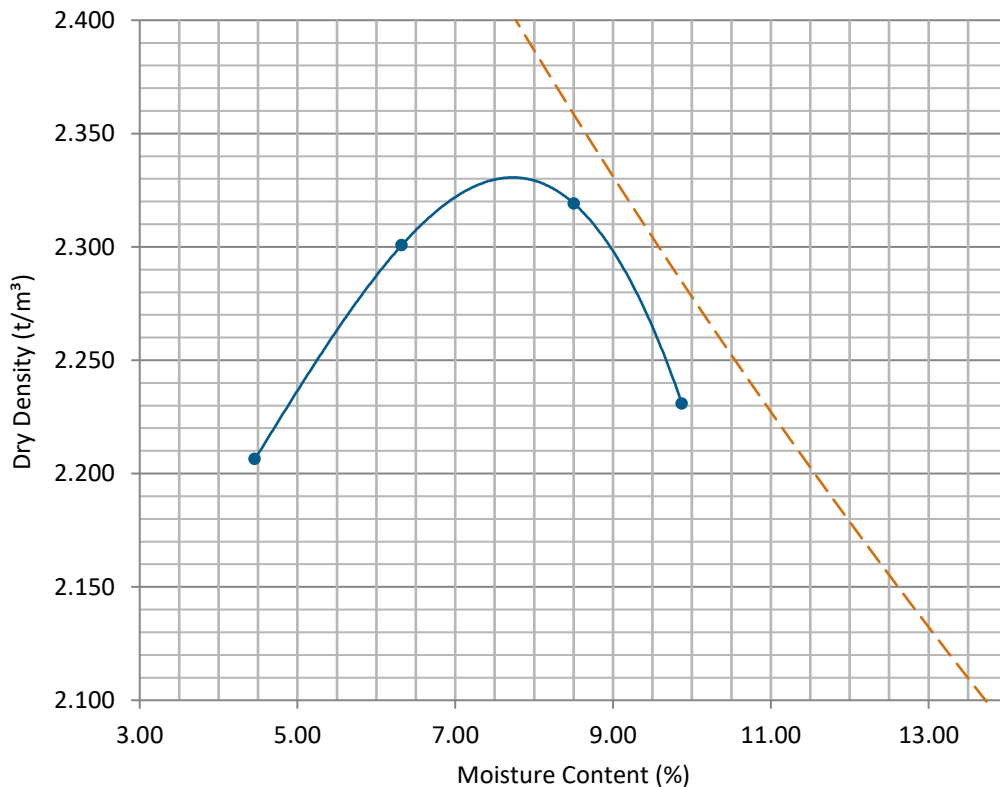
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 25/07/2019  
Lab sample ID: WG19-0561



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### AS1289.5.1.1 Dry Density / Moisture Content Relationship Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.33** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **7.5** (%)


RETAINED ON 19.0mm SIEVE: 15 (%)

RETAINED ON 37.5mm SIEVE: 4 (%)

AIR VOIDS LINE: 0 (%)

#### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 2.95
2. Where further analysis is required, MDD and OMC values of 2.331t/m<sup>3</sup> and 7.7% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 26/07/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0561 SMDD  
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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S5-PM02**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

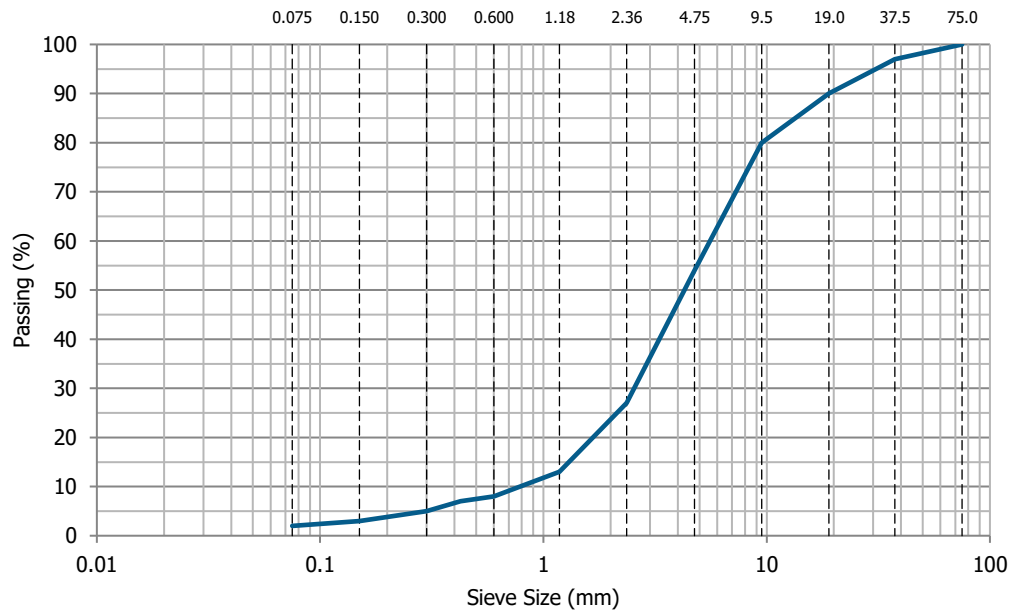
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0562



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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	13
37.5	97	0.600	8
19.0	90	0.425	7
9.5	80	0.300	5
4.75	54	0.150	3
2.36	27	0.075	2

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0562 PSD  
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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S5-PM02**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

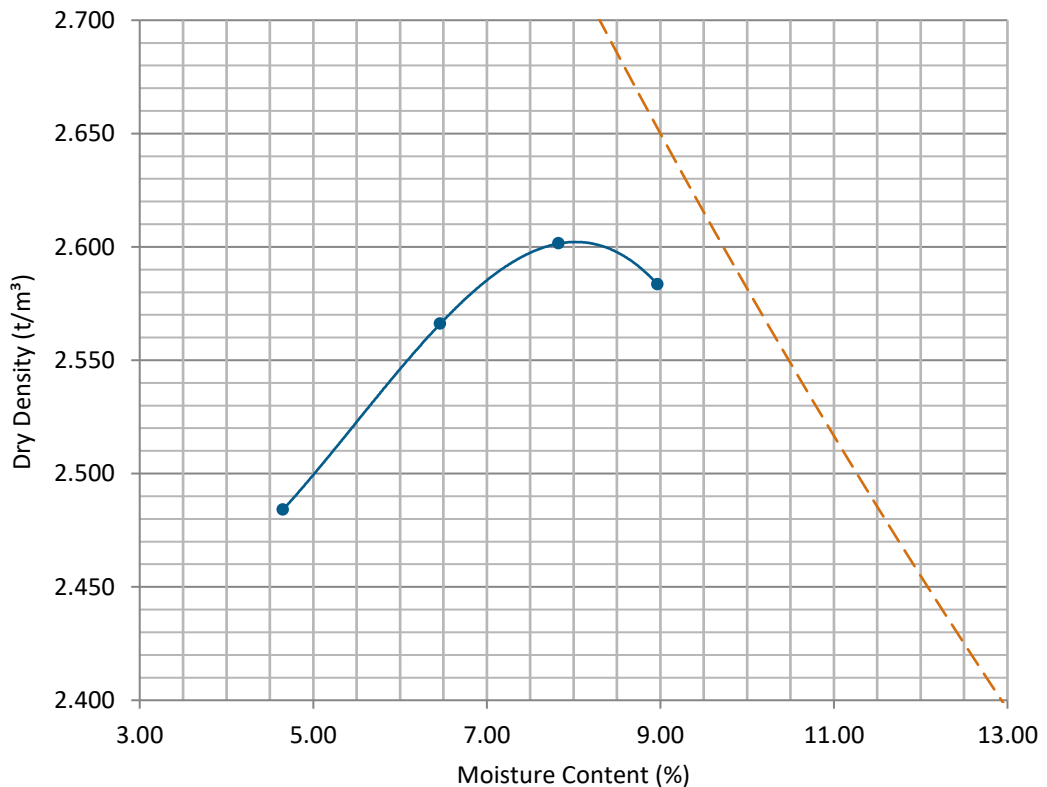
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 25/07/2019  
Lab sample ID: WG19-0562



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### AS1289.5.1.1 Dry Density / Moisture Content Relationship Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.60** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **8.0** (%)

RETAINED ON 19.0mm SIEVE: 10 (%)

RETAINED ON 37.5mm SIEVE: 3 (%)

AIR VOIDS LINE: 0 (%)

#### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.48
2. Where further analysis is required, MDD and OMC values of 2.60t/m<sup>3</sup> and 8.0% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 26/07/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0562 SMDD  
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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S5-PM02**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019  
Lab sample ID: WG19-0562



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## WA915.1 Calcium Carbonate Content

CALCIUM CARBONATE CONTENT (CaCO<sub>3</sub>): **58.6** (%)

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 1/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0562 CCC  
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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S6-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

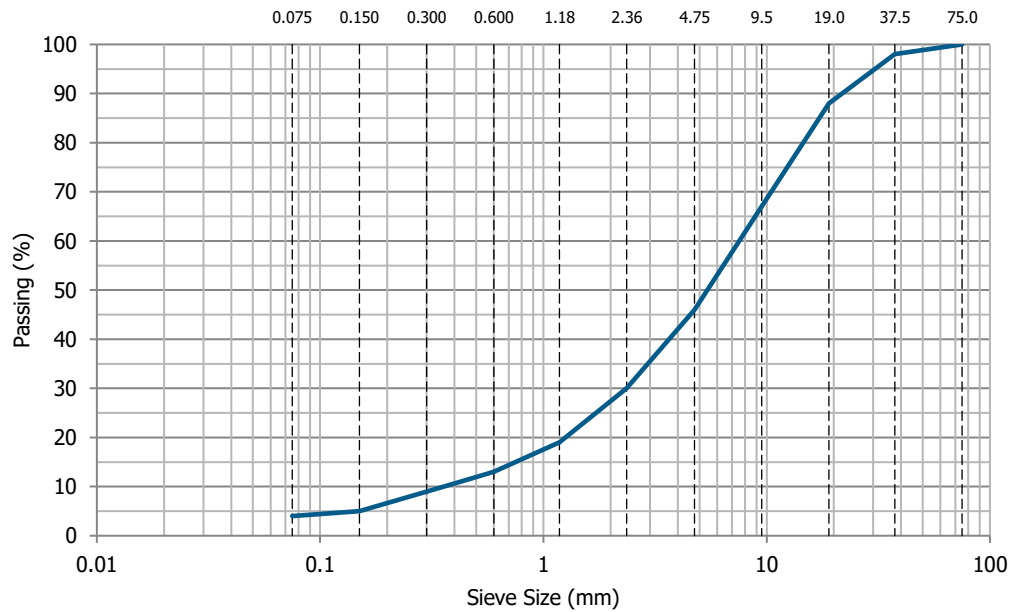
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0563



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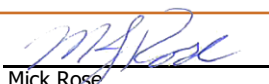
## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	19
37.5	98	0.600	13
19.0	88	0.425	11
9.5	67	0.300	9
4.75	46	0.150	5
2.36	30	0.075	4

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0563 PSD  
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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S6-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

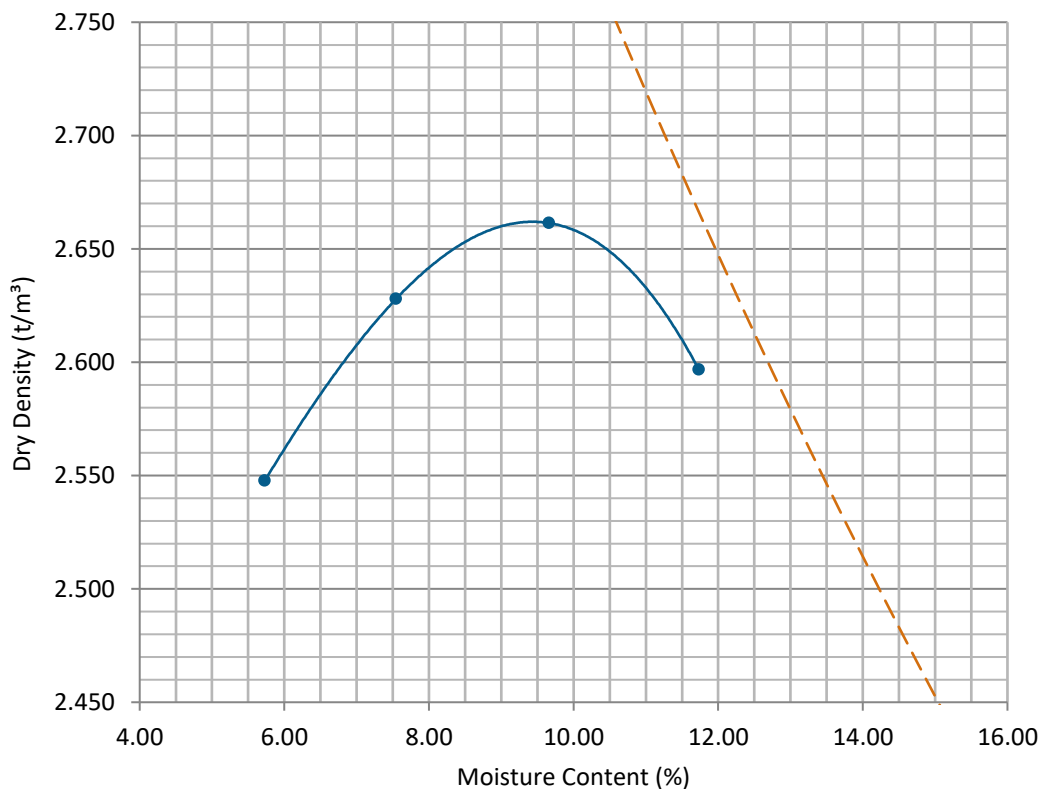
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 24/07/2019  
Lab sample ID: WG19-0563



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### AS1289.5.1.1 Dry Density / Moisture Content Relationship Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.66** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **9.5** (%)

RETAINED ON 19.0mm SIEVE: 12 (%)

RETAINED ON 37.5mm SIEVE: 2 (%)

AIR VOIDS LINE: 0 (%)

#### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.88
2. Where further analysis is required, MDD and OMC values of 2.66t/m<sup>3</sup> and 9.4% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 25/07/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0563 SMDD  
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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S7-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

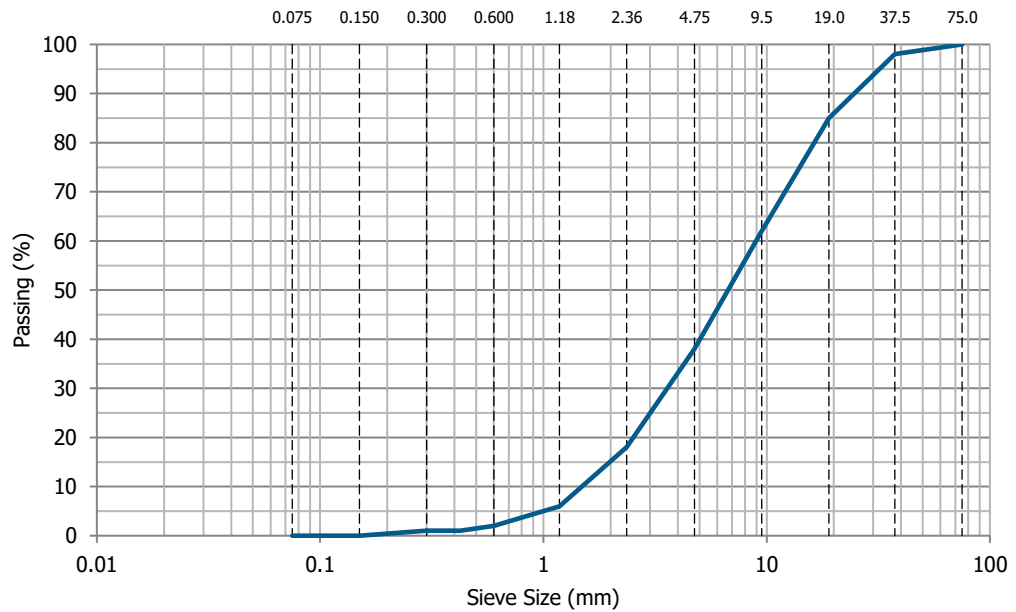
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0564



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## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	6
37.5	98	0.600	2
19.0	85	0.425	1
9.5	62	0.300	1
4.75	38	0.150	0
2.36	18	0.075	0

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0564 PSD  
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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S7-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

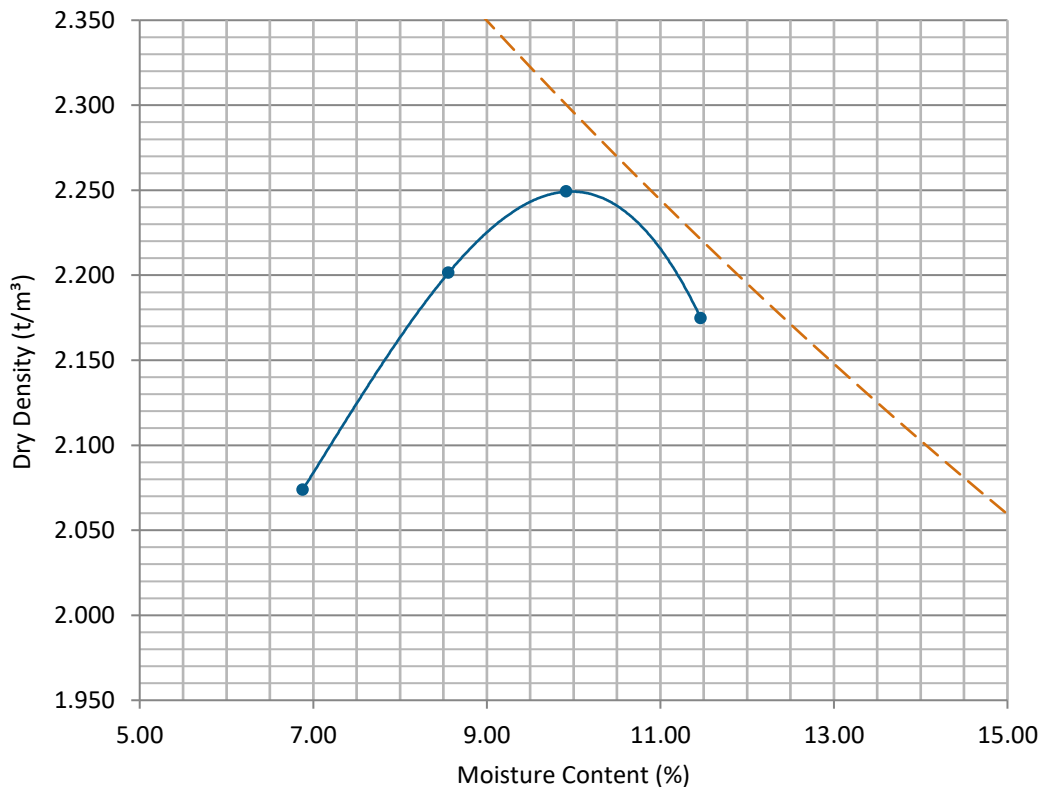
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 25/07/2019  
Lab sample ID: WG19-0564



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### AS1289.5.1.1 Dry Density / Moisture Content Relationship Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.25** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **10.0** (%)

RETAINED ON 19.0mm SIEVE: 15 (%)

RETAINED ON 37.5mm SIEVE: 2 (%)

AIR VOIDS LINE: 0 (%)

#### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 2.98
2. Where further analysis is required, MDD and OMC values of 2.249t/m<sup>3</sup> and 10.0% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 26/07/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0564 SMDD  
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# TEST REPORT

Client: **BHP**

SAMPLE ID: **S9-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

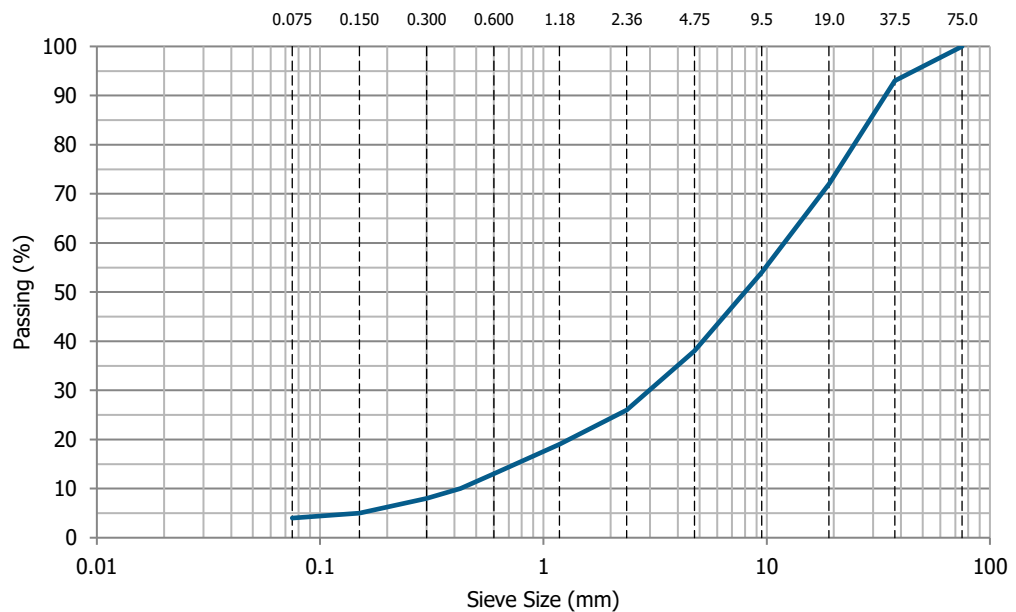
Lab test request: 19-032  
Lab location: Wangara  
Date tested: 31/07/2019 to 01/08/2019  
Lab sample ID: WG19-0565



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E lab@emclabs.com.au


## AS1289.3.6.1 Particle Size Distribution



SIEVE SIZE (mm)	PASSING (%)	SIEVE SIZE (mm)	PASSING (%)
75.0	100	1.18	19
37.5	93	0.600	13
19.0	72	0.425	10
9.5	54	0.300	8
4.75	38	0.150	5
2.36	26	0.075	4

### TEST NOTES:

1. Material was sampled and supplied by the client.

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 01/08/2019  
Accreditation number: 18548  
Test Report No.: 19-032-0565 PSD  
Page: 1 of 1

The results of the tests and/or measurements included in this document are traceable to Australian/National standards. Accredited for compliance with ISO/IEC 17025 – Testing

These results apply only to the material tested, as received. Please refer to 'EMC Terms and Conditions'.

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## TEST REPORT

Client: **BHP**

SAMPLE ID: **S9-PM01**  
DEPTH: **0.05m - 0.25m**

Project: Marillana Creek Infiltration GBI  
Location: Pilbara, WA

Lab test request: 19-032  
Lab location: Wangara  
Date tested: 24/07/2019  
Lab sample ID: WG19-0565

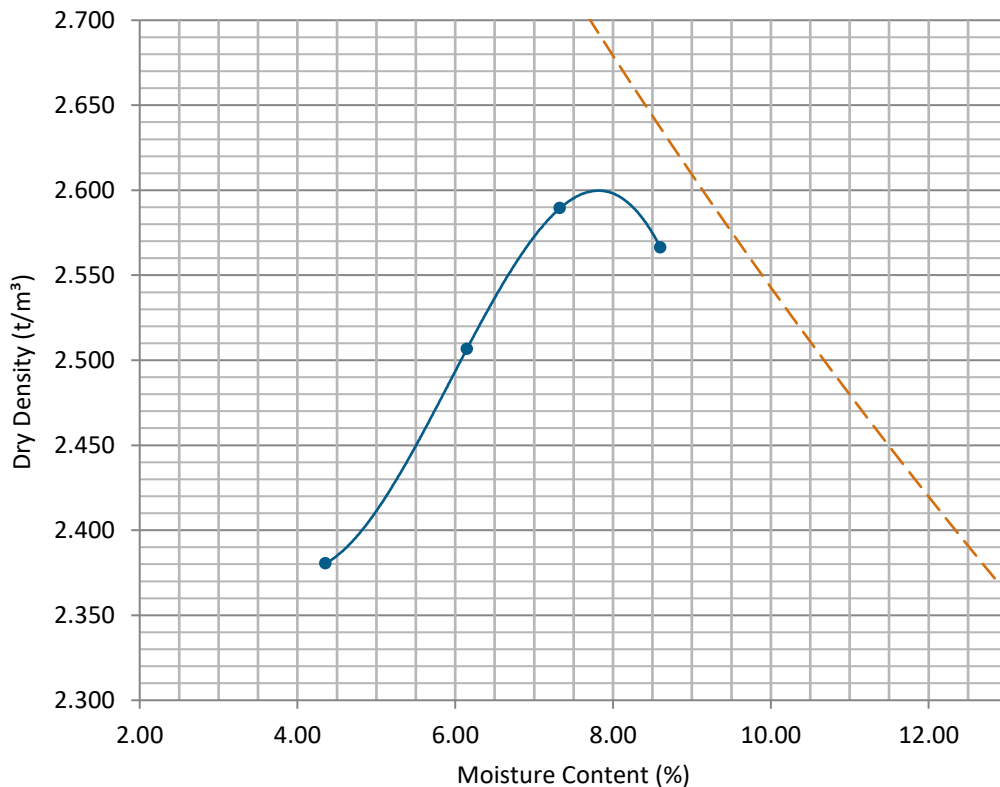


**Earth Materials Classification Trust**  
Unit 6 / 5 Beneficial Way  
Wangara, WA 6065

P (08) 9303 2775  
W www.emclabs.com.au  
E lab@emclabs.com.au

## Dry Density / Moisture Content Relationship

Standard Compactive Effort (Mould A)



STANDARD MAXIMUM DRY DENSITY: **2.60** (t/m<sup>3</sup>)

STANDARD OPTIMUM MOISTURE CONTENT: **8.0** (%)


RETAINED ON 19.0mm SIEVE: 28 (%)

RETAINED ON 37.5mm SIEVE: 7 (%)

AIR VOIDS LINE: 0 (%)

### TEST NOTES:

1. Zero air void line calculated from an assumed particle density of 3.41
2. Where further analysis is required, MDD and OMC values of 2.60t/m<sup>3</sup> and 7.8% may be used.
3. Material was sampled and supplied by the client.
4. The sample was cured for 2 hours. Liquid Limit was determined by visual assessment

Approved signatory:   
Name: Mick Rose  
Position: Laboratory Manager  
Date: 25/07/2019  
Test Report No.: 19-032-0565 SMDD  
Page: 1 of 1

Deviation from Standard: Insufficient material to perform the required 'Mould B' compaction. 'Mould A' was used and only material <19mm was tested, as directed by the Client.

These results apply only to the material tested, as received. Please refer to 'EMC Terms and Conditions'.

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**APPENDIX I**  
**LABORATORY TEST RESULTS – PERMEABILITY TESTING**

SOIL | AGGREGATE | CONCRETE | CRUSHING

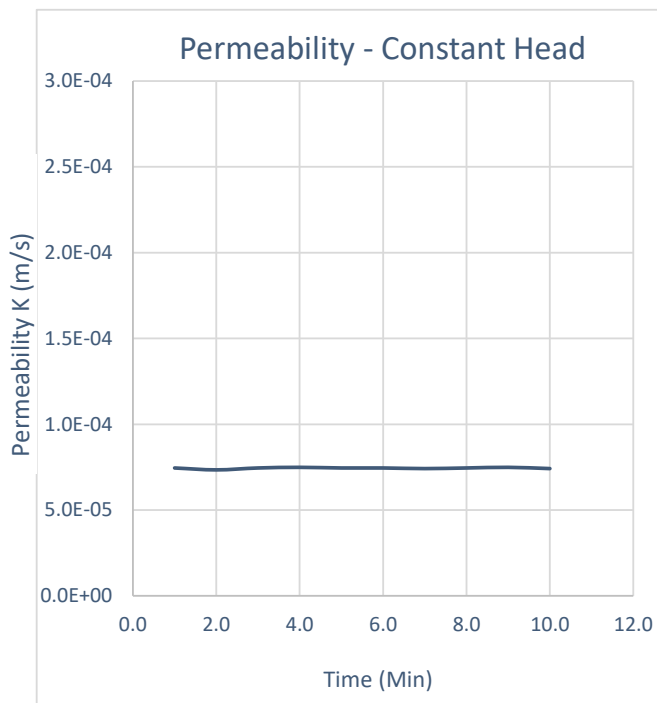
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2140_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2140
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S1-PM01 (WG19-0556) 0.05-0.25m	<b>Date Tested:</b>	29-07-2019

TEST RESULTS - CONSTANT HEAD PERMEABILITY

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	27
Maximum Dry Density (t/m <sup>3</sup> )	2.322
Optimum Moisture (%)	10.6
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	92.1
Laboratory Moisture Ratio (%)	99.7
Surcharge (kPa)	3.0
Hydraulic Gradient	0.6


**Coefficient of Permeability  $K_{20}$  (m/s): 7.44E-05**

Comments:

Approved Signatory:



Name: Matt van Herk  
Function: General Manager  
Date: 31-July-2019



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with ISO/IEC 17025 - Testing  
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SOIL | AGGREGATE | CONCRETE | CRUSHING

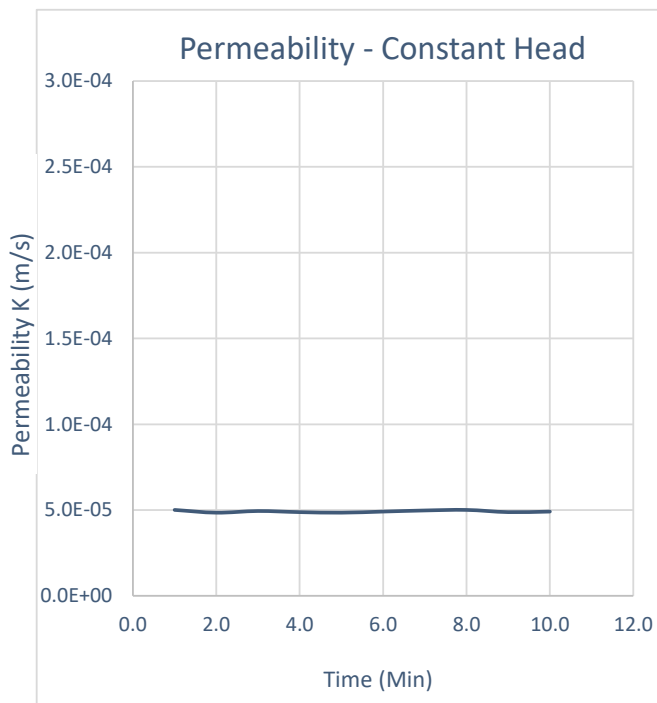
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2141_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2141
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S2-PM01 (WG19-0557) 0.05-0.25m	<b>Date Tested:</b>	27-07-2019

TEST RESULTS - CONSTANT HEAD PERMEABILITY

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	18.5
Maximum Dry Density (t/m3)	2.507
Optimum Moisture (%)	6.5
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	92.0
Laboratory Moisture Ratio (%)	99.4
Surcharge (kPa)	3.0
Hydraulic Gradient	0.7


**Coefficient of Permeability  $K_{20}$  (m/s): 4.91E-05**

Comments:

Approved Signatory:



Name: Matt van Herk  
Function: General Manager  
Date: 31-July-2019



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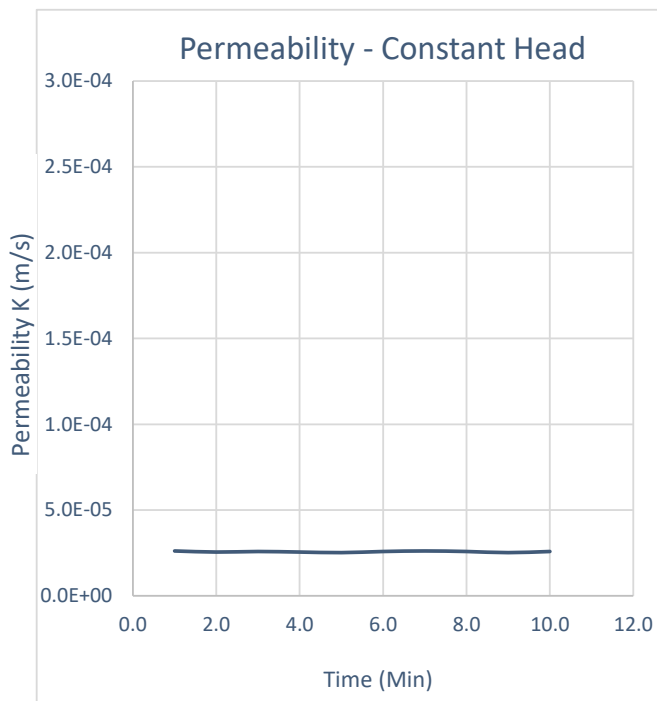
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2142_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2142
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S2-PM02 (WG19-0558) 0.05-0.25m	<b>Date Tested:</b>	31-07-2019

TEST RESULTS - CONSTANT HEAD PERMEABILITY

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
<b>Compaction Method</b>	AS 1289.5.1.1
<b>Hammer Type</b>	Standard
<b>% Retained on 19.0mm</b>	23.5
<b>Maximum Dry Density (t/m3)</b>	2.593
<b>Optimum Moisture (%)</b>	8.4
<b>Target Dry Density Ratio</b>	92
<b>Target Moisture Ratio</b>	100

Specimen Conditions at Compaction	
<b>Laboratory Density Ratio (%)</b>	91.9
<b>Laboratory Moisture Ratio (%)</b>	99.5
<b>Surcharge (kPa)</b>	3.0
<b>Hydraulic Gradient</b>	0.7

**Coefficient of Permeability  $K_{20}$  (m/s): 2.56E-05**

Comments:

Approved Signatory:



Name: Matt van Herk

Function: General Manager

Date: 01-August-2019



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with ISO/IEC 17025 - Testing

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SOIL | AGGREGATE | CONCRETE | CRUSHING

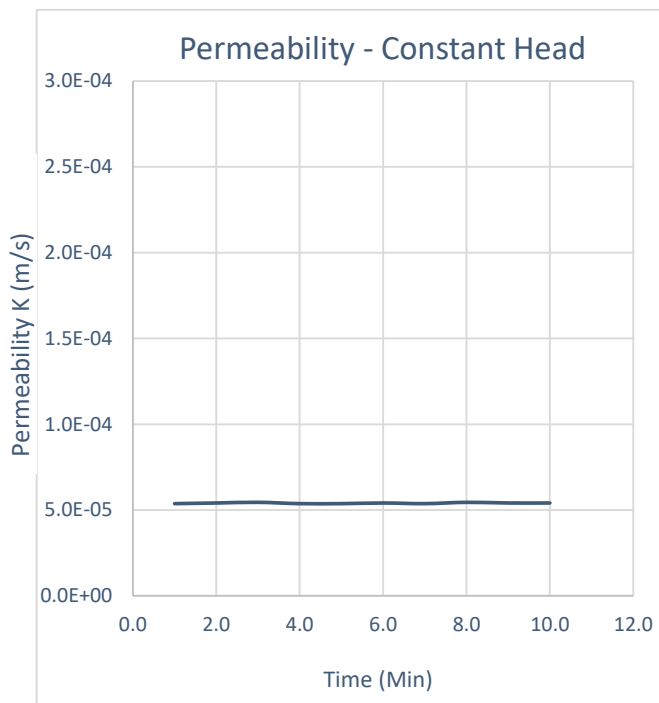
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2143_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2143
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S3-PM01 (WG19-0559) 0.05-0.25m	<b>Date Tested:</b>	26-07-2019

**TEST RESULTS - CONSTANT HEAD PERMEABILITY**

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	8.3
Maximum Dry Density (t/m3)	2.567
Optimum Moisture (%)	9.1
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	92.2
Laboratory Moisture Ratio (%)	98.7
Surcharge (kPa)	3.0
Hydraulic Gradient	0.6

**Coefficient of Permeability  $K_{20}$  (m/s): 5.41E-05**

Comments:

Approved Signatory:

Name: Matt van Herk  
Function: General Manager  
Date: 31-July-2019

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SOIL | AGGREGATE | CONCRETE | CRUSHING

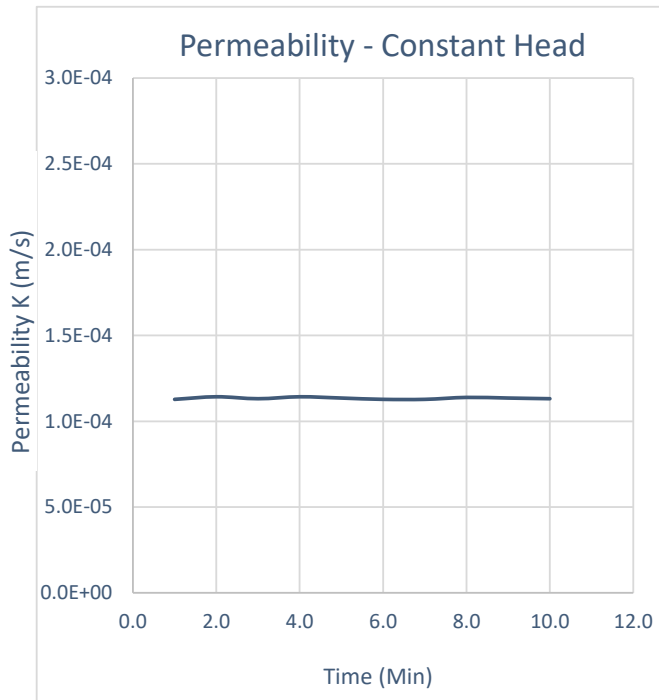
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2144_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2144
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S4-PM01 (WG19-0560) 0.05-0.25m	<b>Date Tested:</b>	30-07-2019

**TEST RESULTS - CONSTANT HEAD PERMEABILITY**

Sampling Method:

Sampled by Client, Tested as Received




Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	0
Maximum Dry Density (t/m3)	2.422
Optimum Moisture (%)	10.8
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	92.1
Laboratory Moisture Ratio (%)	99.0
Surcharge (kPa)	3.0
Hydraulic Gradient	0.6

**Coefficient of Permeability  $K_{20}$  (m/s): 1.13E-04**

Comments:

Approved Signatory:  
  
 Name: Matt van Herk  
 Function: General Manager  
 Date: 31-July-2019

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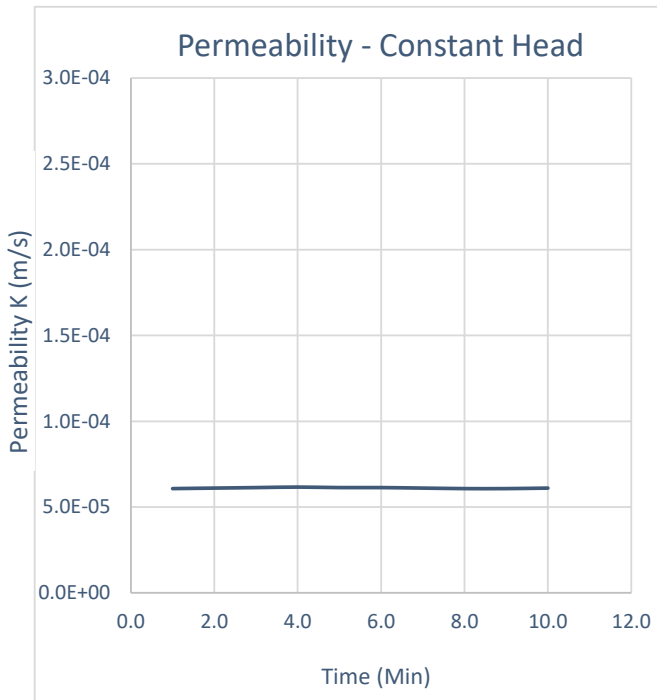
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2145_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2145
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S5-PM01 (WG19-0561) 0.05-0.25m	<b>Date Tested:</b>	1-08-2019

**TEST RESULTS - CONSTANT HEAD PERMEABILITY**

Sampling Method:

Sampled by Client, Tested as Received




Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	14.7
Maximum Dry Density (t/m3)	2.331
Optimum Moisture (%)	7.7
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	91.9
Laboratory Moisture Ratio (%)	101.5
Surcharge (kPa)	3.0
Hydraulic Gradient	0.7

**Coefficient of Permeability  $K_{20}$  (m/s): 6.11E-05**

Comments:

Approved Signatory:  
  
 Name: Matt van Herk  
 Function: General Manager  
 Date: 01-August-2019

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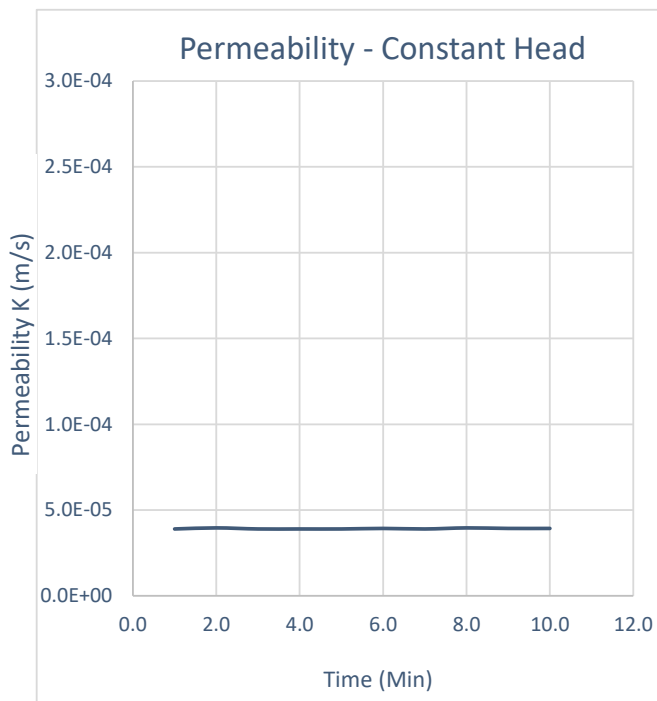
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2146_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2146
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S5-PM02 (WG19-0562) 0.05-0.25m	<b>Date Tested:</b>	31-07-2019

TEST RESULTS - CONSTANT HEAD PERMEABILITY

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
<b>Compaction Method</b>	AS 1289.5.1.1
<b>Hammer Type</b>	Standard
<b>% Retained on 19.0mm</b>	10
<b>Maximum Dry Density (t/m<sup>3</sup>)</b>	2.602
<b>Optimum Moisture (%)</b>	8
<b>Target Dry Density Ratio</b>	92
<b>Target Moisture Ratio</b>	100

Specimen Conditions at Compaction	
<b>Laboratory Density Ratio (%)</b>	92.1
<b>Laboratory Moisture Ratio (%)</b>	100.3
<b>Surcharge (kPa)</b>	3.0
<b>Hydraulic Gradient</b>	0.7


**Coefficient of Permeability  $K_{20}$  (m/s): 3.92E-05**

Comments:

Approved Signatory:



Name: Matt van Herk  
Function: General Manager  
Date: 01-August-2019



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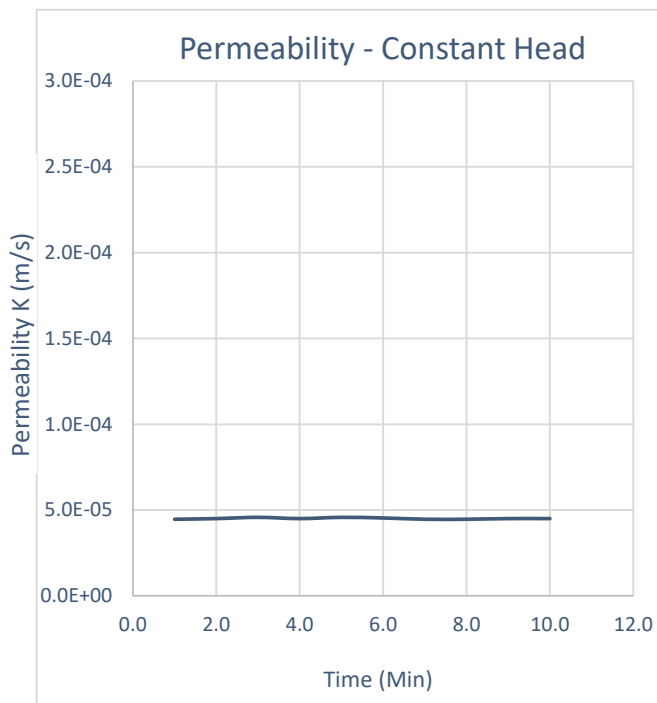
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2147_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2147
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S6-PM01 (WG19-0563) 0.05-0.25m	<b>Date Tested:</b>	29-07-2019

TEST RESULTS - CONSTANT HEAD PERMEABILITY

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	11.8
Maximum Dry Density (t/m <sup>3</sup> )	2.662
Optimum Moisture (%)	9.4
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	92.2
Laboratory Moisture Ratio (%)	102.1
Surcharge (kPa)	3.0
Hydraulic Gradient	0.6


**Coefficient of Permeability  $K_{20}$  (m/s): 4.51E-05**

Comments:

Approved Signatory:



Name: Matt van Herk  
Function: General Manager  
Date: 31-July-2019



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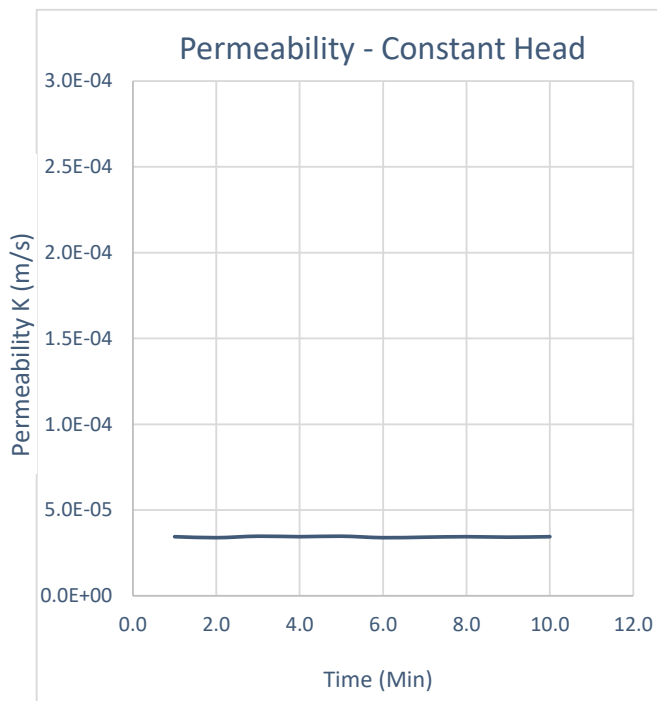
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2148_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2148
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S7-PM01 (WG19-0564) 0.05-0.25m	<b>Date Tested:</b>	31-07-2019

TEST RESULTS - CONSTANT HEAD PERMEABILITY

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	14.7
Maximum Dry Density (t/m <sup>3</sup> )	2.249
Optimum Moisture (%)	10
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	91.8
Laboratory Moisture Ratio (%)	102.8
Surcharge (kPa)	3.0
Hydraulic Gradient	0.7

**Coefficient of Permeability  $K_{20}$  (m/s): 3.43E-05**

Comments:

Approved Signatory:



Name: Matt van Herk  
Function: General Manager  
Date: 01-August-2019



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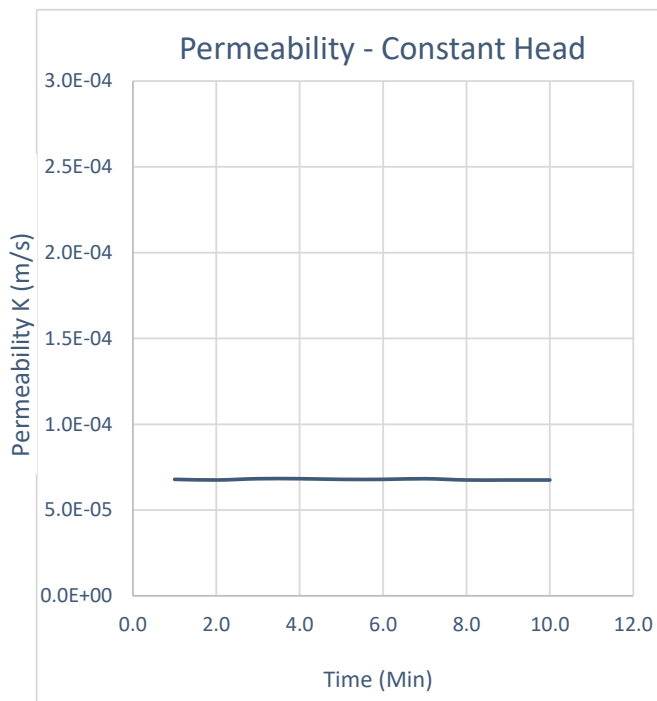
TEST REPORT - AS 1289.6.7.1

<b>Client:</b>	EMC	<b>Ticket No.</b>	S228
<b>Client Address:</b>	6/5 Beneficial Way, Wanagara WA 6065	<b>Report No.</b>	WG19/2149_1_CHPERM
<b>Project:</b>	Marillana Creek Infiltration GBI	<b>Sample No.</b>	WG19/2149
<b>Location:</b>	Pilbara WA	<b>Date Sampled:</b>	Not Specified
<b>Sample Identification:</b>	S9-PM01 (WG19-0565) 0.05-0.25m	<b>Date Tested:</b>	30-07-2019

TEST RESULTS - CONSTANT HEAD PERMEABILITY

Sampling Method:

Sampled by Client, Tested as Received



Compaction Details	
Compaction Method	AS 1289.5.1.1
Hammer Type	Standard
% Retained on 19.0mm	0
Maximum Dry Density (t/m <sup>3</sup> )	2.6
Optimum Moisture (%)	7.8
Target Dry Density Ratio	92
Target Moisture Ratio	100

Specimen Conditions at Compaction	
Laboratory Density Ratio (%)	91.8
Laboratory Moisture Ratio (%)	103.9
Surcharge (kPa)	3.0
Hydraulic Gradient	0.6


**Coefficient of Permeability  $K_{20}$  (m/s): 6.78E-05**

Comments:

Approved Signatory:



Name: Matt van Herk  
Function: General Manager  
Date: 31-July-2019



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**APPENDIX J**  
**PANDA DCP CROSS SECTION INTERPRETATIONS**