

## **Yandicoogina Hydrogeological Field Program Report**

**Bore Installation and Test pumping  
2008/09**

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## EXECUTIVE SUMMARY

A hydrogeological investigation into new mining deposits Oxbow and Junction South West (JSW), as well as accelerated mining of Junction South East (JSE) was carried out as part of the Order of Magnitude (OoM) Study for the proposed expansion of the RTIO Yandicoogina operations from 52Mta to 60Mta. The principal objective of the study was to gain a better understanding of the hydrogeological setting of the Yandicoogina region, to facilitate government approvals and provide information to support a Public Environmental Review (PER). The OoM hydrogeological investigation focused on obtaining aquifer parameters at the new mining deposits principally Oxbow and JSW, surface water groundwater interaction, and estimates of dewatering volumes for the proposed expansion sites.

Conceptually to date the Channel Iron Deposit (CID) orebody is believed to represent a relatively narrow throughflow aquifer incised into low permeability bedrock (Weeli Wolli Formation). Recharge occurs from intense rainfall events and more recently from Marillana Creek surface water flow which recharges the CID aquifer where the creek crosses over the palaeochannel. The hydraulic gradient for groundwater throughout the region is from west to east.

The result of drilling outside the CID in the flood plain of Marillana Creek revealed that the material surrounding the CID aquifer in the flood plain is unconsolidated alluvium or in situ weathered Weeli Wolli Formation bedrock. Regardless of the weathering history and origin of the deposits, the material is characterised by higher hydraulic properties than previously anticipated. The minimum thickness for the flood plain material estimated from drilling at JSE is 37 m that occurs adjacent to the deposit. The thickness of the flood plain material at JSW and Oxbow is variable (20 - >50 m) depending on elevation and proximity to Marillana Creek. The drilling also suggests that the flood plain/weathered bedrock material outside the CID boundary (denoted in this report as alluvium/insitu weathered bedrock aquifer) is characterised by relatively high storage and hydraulically connected to the CID aquifer.

The findings of the drilling program were corroborated during a test pumping program where nine production bores were pumped from two days to 15 days. The results of constant rate test pumping shows that the bores installed in the alluvium aquifer responded to the dewatering in the CID aquifer. This suggests good hydraulic connection between the alluvium and CID aquifers.

Well efficiencies and hydraulic parameters such as hydraulic conductivity (K) and specific yield (Sy) for the CID aquifer were calculated from different monitoring bores. The average K and Sy for the CID are 25 m/d and  $1 \times 10^{-2}$  respectively.

The extent of the CID and flood plain alluvial boundary with the Weeli Wolli Formation was evident on most of the drawdown versus time tests. Using the Theis distance drawdown method, predictions were made to the lateral extent of drawdown for each pumping bore. Conceptual transects for each production bore and the associated monitoring bores were illustrated to show the extent of drawdown during test pumping. The figures show the impact of drawdown in the CID on the water table in the adjacent alluvium aquifer as well as the relationship with Marillana Creek surface water.

Groundwater chemistry data was collected during drilling for environmental monitoring and at the end of test pumping for government requirements and analysis. EC, DO, pH, redox and temperature were all logged at 10 min intervals throughout the pumping tests to supplement the understanding of recharge from Marillana Creek surface water flow. The concentration of DO in groundwater and surface water in particular proved to be a very useful tool in

determining boundary conditions during test pumping. The results of dissolved oxygen monitoring during test pumping has helped to define the boundary between CID and Alluvium.

The results of the interference drawdown analysis suggests that designing borefields with clusters of bores are more efficient than locating multiple bores on a straight line perpendicular to the axes of the CID aquifer. The relatively high transmissivity of the alluvium may provide opportunity to partially dewater the pits from the alluvium (i.e. ex-pit). However, more work and drilling is required to confirm the depth of the alluvium and the hydraulic properties of the deeper sections of the alluvium aquifer. The test pumping of the shallow (37 m) bore in the alluvium at JSE area has shown that this option might be possible particularly on the boundary between CID and the alluvium.

The high yield from the constant rate pumping tests that ranged from 60L/s up to 100 L/s suggests that the dewatering of the pits are possible with fewer bores than previously anticipated. The high end of the pumping was only limited to ~ 100 L/s because of the capacity of the pumps, and the diameter and length of the discharge pipe. Discharge with higher rates up to 150 L/s is possible from several production bores if the discharge outlet were designed to handle higher volumes.

This programme has improved the conceptual understanding of the hydrogeological characteristics of the CID, flood plain alluvium/insitu weathered bedrock aquifers and relationship with Marillana Creek surface water. From this study a large scale regional CID groundwater model, including all deposits (BHPB and RTIO) will be constructed to predict dewatering requirements, manage current dewatering borefields and help in the understanding for mine closure.

## 1. INTRODUCTION

Hydrogeological investigations have been ongoing at RTIO Yandicoogina mine site since late 1970. The focus of early investigations was at Junction Central (JC) deposit, which commenced mining in 1998, followed by Junction South East (JSE) deposit, which commenced mining in 2005. In 2008 RTIO undertook an Order of Magnitude study (OoM) to obtain a better understanding of the hydrogeological setting of the Yandicoogina mine site and to gain government approvals to extend production beyond 52Mta.

The proposed expansion included accelerated mining of JSE deposit in a northerly direction and mining new deposits comprising Junction South West (JSW), and Oxbow. The aim of the OoM study was to obtain aquifer parameters at the proposed new pits, define surface water groundwater interaction, and estimates of dewatering volumes for the proposed expansion sites. The work was carried out internally by the Resource Development, Technical Projects, Hydrogeological team.

Based on the current mine schedule, mining of the Channel Iron Deposits of the JSW and Oxbow deposits is due to commence in 2013, and 2014 respectively.

A hydrogeological drilling and testing program comprised of drilling and testing of seven production bores and 22 monitoring bores in JSW, one production bore and seven monitoring bores in Oxbow and one production bore, one alluvial testing bore and five monitoring bores in JSE. The borefields were designed after a review of previous works including test pumping programs conducted at JC and JSE deposits. The results of the drilling, test pumping and hydrochemical sampling undertaken during the investigation will be used to update the Yandi groundwater and water balance models to predict impacts and optimise requirements for water management against the developing mine schedule.

The drilling program was undertaken from September 2008 through to April 2009 by Nudrill Pty Ltd and supervised by Hydrogeologists from RTIO. The exploration program was drilled by Nudrill's Rotamac Rig 3, with the production bores drilled with the Drilltech Rig 1 and 2. Test pumping was undertaken in June through to September 2009 by Test Pumping Australia Pty Ltd.

## 2. LOCATION AND ACCESS

The Yandicoogina mine site is located approximately 85 km northwest of Newman and 145 km east of Tom Price in the Central Pilbara. The 54 km Yandicoogina access road is 135 km northwest of Newman on the Great Northern Highway. Yandicoogina deposit, within the mining lease 274SA is divided into two active mining areas JC and JSE. The primary focus of the study is up gradient, adjacent to the west of the main mining areas in JSW and Oxbow, with a component of the investigations in JSE.

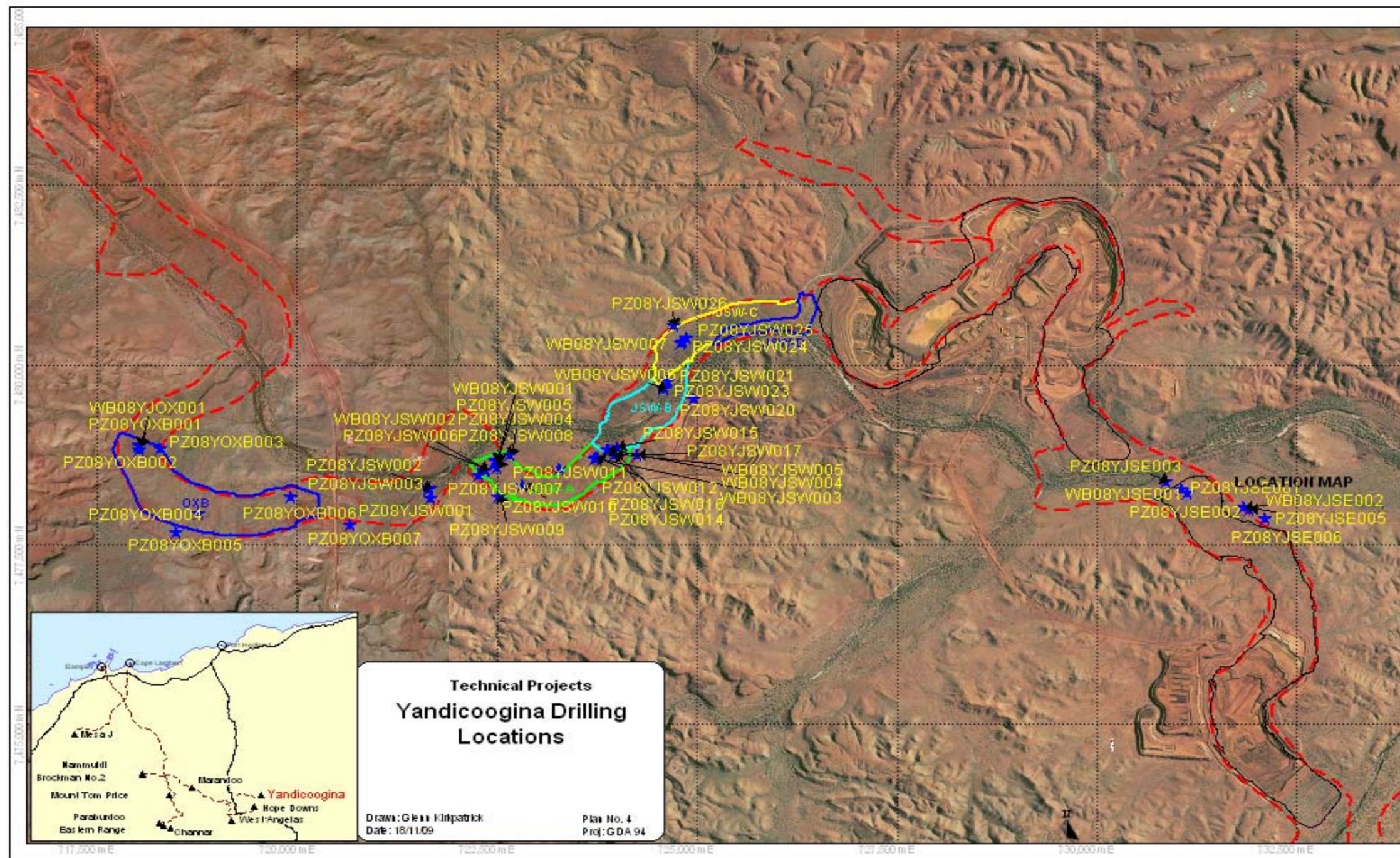


Figure 1 Location of Yandicoogina Pits with study drilling locations

### 3. TOPOGRAPHY AND DRAINAGE

The Yandicoogina CID deposit is located within the Marillana Creek regional catchment and occurs within a broad structural basin of the Yandicoogina Syncline, formed by synclinal folding of the Lower Proterozoic Brockman Iron Formation, which is conformably overlain by the Weeli Wolli Formation. The total length of the CID is over 80 km, but within the RTIO lease it is 26 km long, approximately 500 m wide and average thickness of 60 m. The Marillana Creek catchment originates 20 km to the east of the Great Northern Highway, and runs predominately east-west draining into the Weeli Wolli Creek system downstream of RTIO's Yandicoogina mine operations. Two tributaries, Phil's Creek and Yandicoogina Creek drain into Marillana Creek in the vicinity of RTIO mine site.

The CID is located in the central part of a broad, east-trending drainage basin with the Hamersley Range to the north and an unnamed range to the south (PPK, 1998). The CID where it outcrops resulting from recent dissection from modern day drainages, forms a series of characteristic 'mesas' which are generally orientated with the present day drainage, with surface gradients of 1.5 m/km along the axis of the basin (PPK, 1998).

The creeks in the region are dry for most of the year and are prone to flooding during the wet season, where stream flow is ephemeral. In recent times the two mining operations of BHP BILLITON and RTIO in the area the Marillana and Weeli Wolli Creek systems have been impacted by surplus water discharges, resulting in permanent surface water flow in parts of the drainage lines.

### 4. GEOLOGY

The Yandicoogina area consists of Proterozoic basement rocks of the Weeli Wolli Formation, overlain by relatively thin Tertiary and Quaternary sediments. The basement rocks comprise Banded Iron Formation (BIF), chert, shale and volcanic rocks. The thin alluvium and colluvium cover consists of valley fill and drainage deposits, which include the Channel Iron Deposits (CID) and are restricted to areas close to present drainages.

The Proterozoic basement rocks have been faulted and folded into a gently synclinal structure called the Yandicoogina Syncline which controls the geomorphology and ultimately the location of the CID and present drainage system. Fractured, siliceous rocks underlie and enclose the CID and are hydraulically connected to the CID aquifer.

The Tertiary Pisolite, commonly known as the Channel Iron Deposit or Robe Pisolite, is an iron rich sedimentary deposit which infilled the ancient river channels. The CID is a continuous, mineralised, pisolitic iron ore that occurs adjacent to the present day Marillana, Yandicoogina and Weeli Wolli Creek systems. The thickest sections of the CID occur close to the centre of the palaeochannel in long, straight sections but on meanders, the thickest section occurs close to the outer bend of the meander (PPK, 1998).

The CID consists of pisolitic clasts of goethite-hematite, clay and fossil wood fragments with an approximately average thickness of 60 m. The CID clasts (peloids) are typically irregular or subangular to subrounded in shape. These iron rich peloids are interpreted to represent the end product of detrital weathering via sheet wash processes deposited in low lying areas (Ramanaidou, et al, 2003).

A detailed 1:10,000 investigation into the geology of the Yandicoogina area was undertaken by Youngman, 1997, where the lithologies have been divided into four main groups:

- Basement rocks of Archaean and Proterozoic age consisting of Banded Iron Formation (BIF), shale, dolerite and dolomite (Wittenoom Formation found further upstream of the study area). Several dolerite dykes cut through these BIF sequences throughout the area.
- Altered basement rocks of the Proterozoic consisting of lateritic profiles and basement rocks of various weathered states. This Basal Clay Conglomerate (BCC) consists of clasts of BIF, chert and shale in a yellow-white clay matrix.
- Early Tertiary Channel Iron Deposits comprise chemically enriched weathered BIF and fluvial deposits. The CID consists of iron oxide spheroids that have formed insitu by chemical precipitation of hydrated iron oxides around hematite nuclei. The outer coating of the nuclei is composed of goethite and the matrix is a mixture of goethite, limonite and ferruginised wood fragments. The CID is divided into two parts; a clayey basal zone and an upper economic pisolite zone. The basal zone or Limonitic Goethite Channel (LGC) overlies the Basal Conglomerate and consists of limonite, clay and goethite often with the CID texture altered. The Pisolitic zone consists of two parts the bottom high grade ore (GVL), and the lower grade ore (GVU).
- Tertiary and Quaternary overburden material is a mixture of lateritic pisolite, poorly sorted angular to sub angular BIF, chert and dolomite gravels in a sandy matrix. Overlying the main ore deposit is a Weathered Channel Horizon (WCH) consisting of pisolitic material with low iron content and contains various amounts of clay.

## 5. HYDROGEOLOGY

There are three main aquifers recognised in this study in the Yandicoogina region. They comprise the fractured basement aquifer, the overlying fractured CID aquifer and the surficial unconfined alluvial aquifer. These aquifer systems are recharged by direct infiltration of rainfall and through creek beds. The CID is recharged primarily by the leakage through the alluvium in areas where the creeks overly the CID. Exceptionally high water levels in the creek from rainfall and discharge of surplus mine water flood areas beyond the alluvium resulting in direct infiltration to the CID. Groundwater can discharge from the aquifers to the Marillana Creek after a flood event. Recharge may also occur by groundwater inflow from the basement rocks where upward hydraulic gradients occur.

The CID aquifer is relative narrow meandering palaeochannel imbedded in the basement Weeli Wollie Formation. The heterogeneous aquifer exhibits a variable but overall increasing permeability with depth, related to secondary porosity developed in discrete zones formed by fractures, solution channels and cavity features (MWH, 2006). The development of solution features has largely superseded the primary porosity of the interstitial pore space. Permeability is generally lowest within the main ore zone, particularly the upper zone and highest in sections of the limonite-goethite clay zone where large voids are often encountered. However the LGC is highly variable and may exhibit very low permeability in clay zones.

The alluvial/insitu weathered bedrock flood plain deposits that overlie the CID and basement aquifers and the river gravel in the beds of Marillana and Weeli Wollie Creek systems consist of mixtures of poorly sorted gravels, sand, silt and clay. These alluvial deposits are predominantly saturated due to the surface water discharge from mining operations located up gradient of the Yandicoogina mine site. The alluvium deposit underlying the creek beds is primarily recharged by creek flow, usually following/during a rainfall event. Discharge of surplus water from mining operations to Marillana Creek has resulted in greater recharge and increased storage within the creek alluvium. Transmissivity is variable within the alluvial

aquifer. Groundwater moves freely within the coarser less clayey sediments but flow may slow within the underlying clayey sections of the alluvium. The coarse river gravels of the upper alluvium of the creek bed are generally highly permeable and if thick over a larger area has the ability to transmit large volumes of groundwater (Peck, 1998).

The basement aquifer is a fractured rock aquifer which consists of inter-bedded banded iron formation, banded chert, shale and dolerite. It underlies and surrounds the CID and alluvial aquifers. The aquifer permeability is variable and largely dependent on the development of structural features. Monitoring bores screened in basement lithologies demonstrate hydraulic connection with the CID following dewatering abstraction. Groundwater in the basement rocks occurs in secondary porosity of the weathered zone and within fractures in the bedrock, with lower hydraulic conductivities than the CID and flood plain alluvium. However, local and regional fracture systems within the basement rocks are capable of providing significant water yields.

The hydraulic connection between the alluvium and the CID is likely to be variable depending on the presence of clay or coarse sediment at the base of the alluvium and the presence of solution paths locally in the CID. The natural depth to groundwater varies from 3 to 20 m below surface with the groundwater flow direction to the east and southeast in the west and to the northeast in the east.

## 6. LICENSING AND APPROVALS

An Approvals Request (AR-08-03254) was submitted for the overall drilling program covering JSW, JSE and Oxbow. AR-08-03254 covered drilling locations within areas inside and outside Marillana Creek. Due to extra restrictions and the time constraints of drilling within the creek, the AR was split into two, AR-08-03254A (outside the creek) & AR-08-03254B (inside the creek).

AR-08-03254A was the permit issued for the drilling program and drill pads of 50 m x 50 m were constructed for each site. AR-08-03254B, due to company priorities, funding and logistics was not issued and the creek bores were not drilled. The request was issued on 1 August, 2008 for the construction of drill pads and sumps and expired in August 2009. A licence to construct or alter wells CAW166698 (1) was obtained as part of this process.

## 7. RESULTS

### 7.1 Drilling and Bore Completion

#### 7.1.1 Overview

Bore locations were designed based on the information collected from previous groundwater investigations, and Resource Evaluation geological models. Location of some of the drill sites was constrained by approvals due to heritage, bed and banks permits and access.

Production bores were designed to be drilled on the up gradient edge of each proposed pit to obtain aquifer hydraulic parameters and to be then incorporated into future dewatering borefield designs. Each production bore targeted the centre of the CID to maximise the full extent of the aquifer. Observation bores were designed to optimise the results obtained from test pumping such as determining the extent of the cone of depression and connection between CID and the alluvium and basement aquifers.

The drilling program, contracted to Nudrill Pty Ltd commenced in early September 2008 with the company using their Rotamac 1302, (Rig 3) initially to do all the exploration work. A

second drill rig (Drilltech Rig 1) was introduced in late September to capitalise on available resources. The program was about 75% completed by the 18<sup>th</sup> of December when drilling ceased for the Christmas and wet season break. The program recommenced in March 2009, with Nudrill using Drilltech (Rig2) to conclude the program in April.

During the program, drilling was conducted over three locations; Junction South West, Junction South East and Oxbow where a total of nine production bores, one testing bore and 34 monitoring bores were successfully installed. Drilling was undertaken by conventional air/percussion methods with some unconsolidated alluvial material being drilled by mud/rotary techniques. The project was supervised by RTIO hydrogeologists. Summary details of all monitoring and production bores completed during this programme are listed in Table 1 through to Table 6.

### **7.1.2 Monitoring bores**

The exploration drill program consisted of drilling 22 monitoring bores in Junction South West, seven in Oxbow and five in Junction South East (Figures 2-4). Monitoring bores were designed down gradient of the production bore, and placed approximately 10, 100, 500 m away in the CID. Additional 50 m deep bores were completed in the alluvial flood plain and river gravel adjacent to the CID channel. The monitoring bores were required for observing future dewatering impacts and groundwater responses to abstraction.

All CID bores were drilled by rotary air blast hammer drilling techniques. These holes were drilled into the BCC or Weeli Wolli Formation depending on the thickness of the BCC, to allow for sufficient drawdown below the base of the LGC. Alluvial bores drilled into the flood plain adjacent to the CID and Marillana Creek river gravels had nominal depths of 50 m to monitor hydraulic connections between the alluvial and the CID aquifer.

The bores were drilled conventionally to 5.7 m using a 317 mm bit. Steel surface casing (219 mm OD) was cemented allowing 700 mm above ground level. A 203 mm hole was drilled through the surface casing to the basement Weeli Wolli Formation or greater than 10 m below the LGC depending on thickness of the BCC lithology. The monitoring bores were then cased with slotted 50 mm PVC to the water table and blank casing then to surface. The annulus was gravel packed to approximately 6 m above the slotted PVC casing with 3.2 to 6.4 mm graded gravel. All monitoring bores were sealed with cement or A & B foam to prevent foreign objects, and create a water tight seal.

Four 50 m alluvial/insitu weathered bedrock holes were drilled using 152 mm mud/rotary techniques of which two (PZ09YJSW011 and PZ09YJSW020) were drilled in JSW (Figure 2) and two (PZ08YJSE005 and PZ08YJSE006) in JSE (Figure 3). Bores were drilled to 4 m using 317 mm tri cone then 203 mm (OD) steel surface casing was placed and cemented in the hole. The holes were then drilled and cased with slotted 50 mm C112 PVC to the water table and blank casing to surface.

Two monitoring bores, PZ08YJSW002 and PZ08YJSW003, were constructed in Marillana Creek river gravels at the western end of the JSW-A pit (Figure 2). The rationale behind was to establish head differences, chemistry and response to discharge from permanent surface water flow. Mud/rotary drilling techniques were used to drill up to 14 m in the unconsolidated coarse river gravels. PZ08YJSW002 was drilled to 28 m and cased with 203 mm (OD) diameter steel surface casing then 203 mm air/hammer through to 72 m. PZ08YJSW003 was drilled 311 mm mud /rotary to 8 m into the river gravels cased with slotted 50 mm C112 PVC to surface.

A prefix of 'PZ' was used to describe monitoring bores, followed by the year drilled; a four letter description of the site, and the last three digits reflected the order in which they were

designed. A summary of monitoring bores is presented in Table 1, 3 and 6. Bore completion diagrams can be found in Appendix 1.

### **7.1.3 Production bores**

Seven production bores were constructed in clusters at JSW. Bores were drilled up gradient of the proposed pit location taken from the 2008 long term mine plan design (Figure 2). Two bores were drilled on the western edge of pit JSW-A where mining is due to commence in 2015. A cluster of three bores was drilled on the up gradient edge of pit JSW-B where no mining is planned in the near future due to the ore body crossing Marillana Creek. Two bores were drilled up gradient of JSW-C which is due to commence mining in 2012.

One production bore was drilled on the northern edge of the Oxbow pit near the BHP Billiton mining lease boundary, where mining is due to commence in 2018. In JSE one production bore was drilled on the north western edge near the Marillana Creek and a test bore was drilled in the flood plain alluvials to the east on JSE pit (Figure 3). All production bores were located in the middle on the palaeochannel and were open the full extent of the CID aquifer and will be used for future dewatering.

Production bores were drilled 445 mm air/hammer down to 6 m, reamed out with a 609 mm air/roller and positioned 508 (OD) steel surface casing. The hole was then drilled 444 mm (OD) air/hammer into the BCC or Weeli Wolli Formation. The shallow, 37 m (WB08YJSE002) bore was drilled in the flood plain alluvials for testing aquifer characteristics. It was drilled 371 mm air/roller to 1.5 m and cased with 254 mm (OD) steel surface casing and cemented in place. The hole was drilled to the Weeli Wolli Formation with a 311 mm mud/rotary bit. The annulus was gravel packed, from the total drilled depth to the surface, using 3.2 to 6.4 mm graded gravel.

Bores drilled in the CID were constructed with 324 mm (OD) steel casing with the mineralised high permeability zones screened with stainless steel wire wound screens. Blank casing was set from surface to the water table followed by slotted casing to the 6 m sump at the bottom. Twelve meters of stainless steel screens were set over the lower most permeable zones. Bores were developed from 4 hours to 16 hours till continuous clean, silt and sand free water was obtained.

Bore WB08YJSE002 was drilled via mud/rotary methods due to the unconsolidated fine silt clay material. The bore was cased to 37 m with 200 mm (OD) PVC slotted casing with one 6 m blank length at the surface. All bore head works were capped with a lockable steel cap and a concrete plinth placed around each bore. A summary of all the production bores can be found in

Table 2, 4 and 6. Bore completion diagrams are presented in Appendix 1. The location of the bore is shown by Figure 2-3.

## 7.2 WELL DEVELOPMENT and GROUNDWATER SAMPLING

Well development was achieved by airlifting using the rig mounted compressor coupled to the truck mounted auxiliary compressors. Once constructed, the production bores were developed anywhere from four hours up to 15 hours, until a clean water sample was observed. At the end of airlifting, a water sample was taken and field tested for pH, Electrical Conductivity (EC) and temperature. Flow rates were measured using a 90° V-Notch weir. Holes drilled via mud/rotary techniques were developed using auxiliary compressor with poly pipe inserted in the 50 mm casing. Field water chemistry was taken at the end of each rod to monitor pH, EC and Temperature in different lithological units. A final measurement of the field parameters was taken and recorded at the end of airlifting (Tables 1-6).



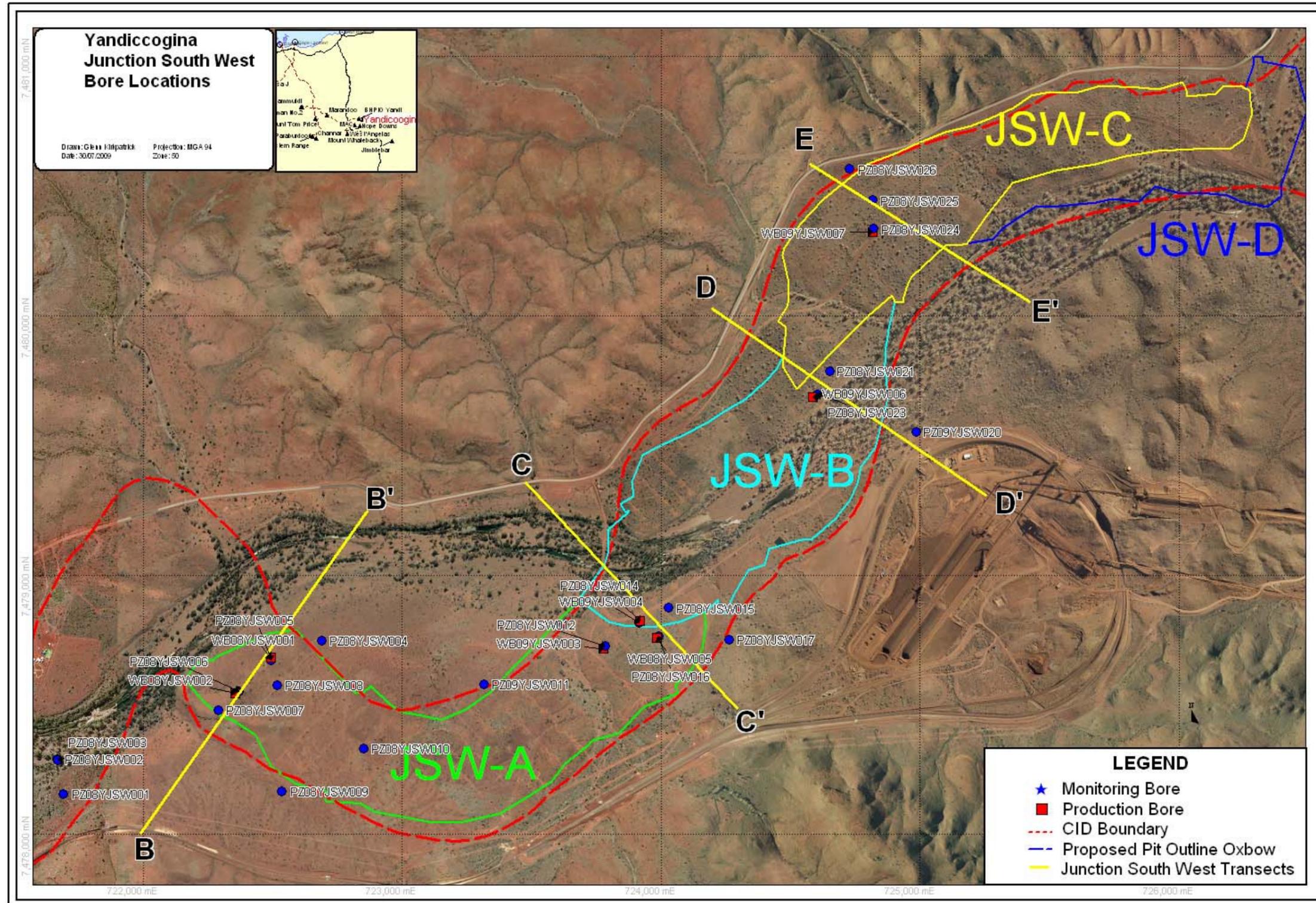


Figure 3 Location of bores drilled at Junction South West with superimposed transect line

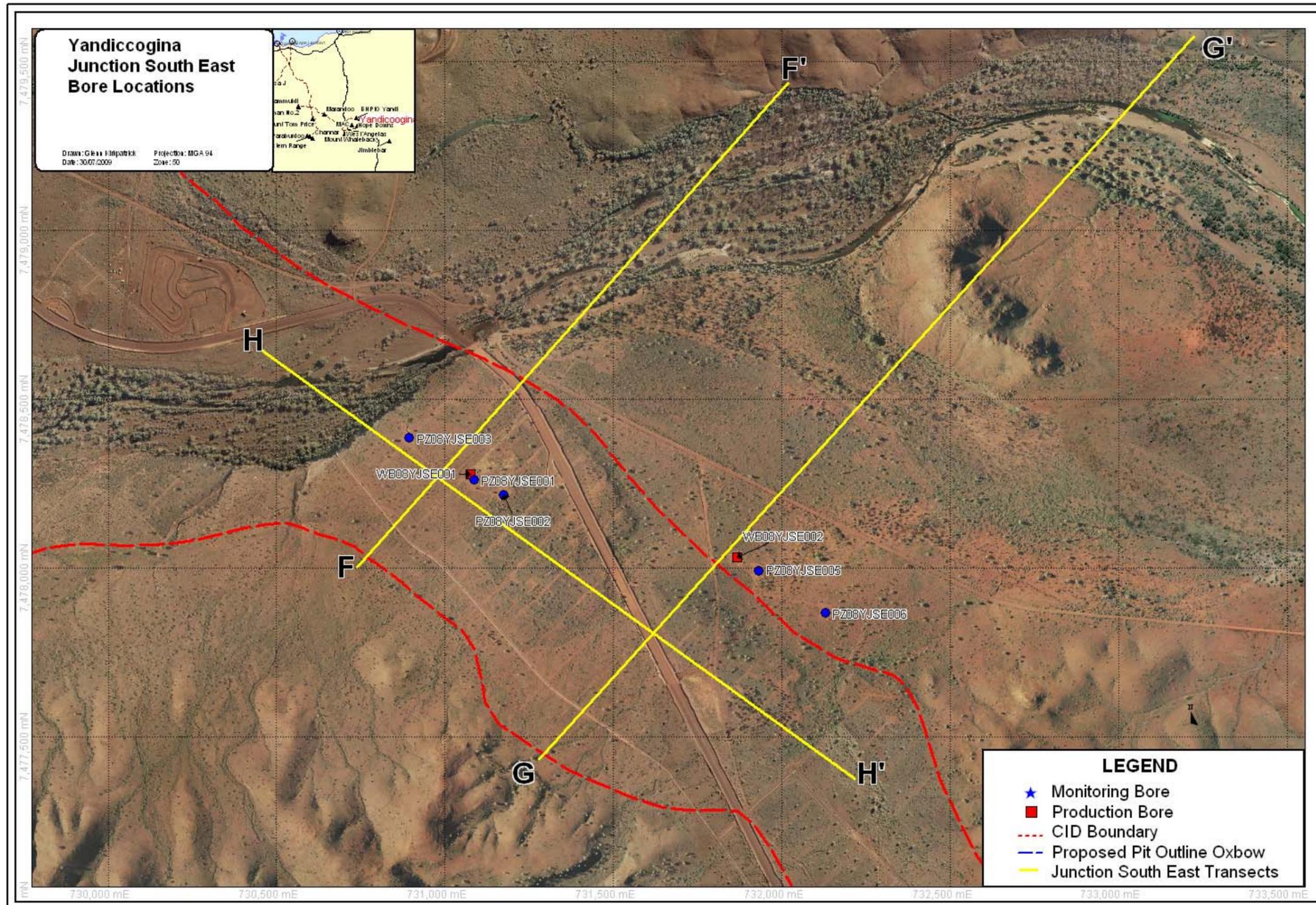


Figure 4 Location of bores drilled at Junction South East with superimposed transect line

Table 1 Junction South West Monitoring Bore Completion Data

Bore number	Mine grid		MGA94		Top of Casing (RL)	Drill depth (m)	PVC Casing type (C112) (mm)	Slotted interval (m bgl)	Airlift Yield (L/sec)	pH	EC (µS/cm)	Screened Formation	Water level (RL)
	Easting	Northing	Easting	Northing									
PZ08YJSW001	55554.08	8000.97	721690.89	7478156.29	531.82	77	50	11 - 77	2	8.24	875	CID	522.11
PZ08YJSW002	55531	8134.60	721667.81	7478289.94	525.97	72	50	24 - 72	N/A	N/A	N/A	CID	522.01
PZ08YJSW003	55533.12	8132.06	721669.93	7478287.40	526.02	8	50	0 - 8	N/A	N/A	N/A	Alluvium	523.94
PZ08YJSW004	56550.98	8590.94	722687.95	7478746.36	528.29	50	50	30 - 50	<0.1	N/A	N/A	Alluvium	519.64
PZ08YJSW005	56357.92	8515.59	722494.86	7478671	530.46	92	50	16 - 88	21.5	7.71	967	CID	519.80
PZ08YJSW006	56231.17	8394.98	722368.1	7478550.36	538.77	95	50	19 - 91	29.3	7.31	858	CID	519.15
PZ08YJSW007	56153.06	8324.59	722289.98	7478479.96	540.19	90	50	23 - 89	1	8.09	966	CID	519.66
PZ08YJSW008	56380.51	8420.86	722517.46	7478576.25	530.09	92	50	9 - 87	30	7.87	921	CID	519.34
PZ08YJSW009	56396.59	8011.22	722533.54	7478166.54	532.86	50	50	22 - 46	0.5	7.76	1223	Alluvium	520.56
PZ08YJSW010	56715.94	8174.55	722852.95	7478329.91	532.14	104	50	18.5 - 102.5	29.3	7.89	916	CID	519.03
PZ08YJSW012	57647.25	8569.74	723784.41	7478725.17	531.49	94	50	0 - 73	31	7.73	925	CID	517.33
PZ08YJSW014	57781.27	8668.19	723918.46	7478823.64	528.51	100	32	35 - 95	36.5	7.94	924	CID	517.05
PZ08YJSW015	57892.04	8718.47	724029.24	7478873.93	532.20	98	50	26 - 98	42.7	7.59	896	CID	517.01
PZ08YJSW016	57852.841	8610.43	723990.04	7478765.87	533.34	96	50	17.5 - 89.5	38.5	8.02	955	CID	517.1
PZ08YJSW017	58125.26	8594.25	724262.50	7478749.68	535.38	50	50	20 - 50	2	7.83	987	Alluvium	516.14
PZ08YJSW021	58512.74	9631.27	724650.04	7479786.9	523.98	92	50	14 - 86	18.8	7.20	1050	CID	513

Bore number	Mine grid		MGA94		Top of Casing (RL)	Drill depth (m)	PVC Casing type (C112) (mm)	Slotted interval (m bgl)	Airlift Yield (L/sec)	pH	EC (µS/cm)	Screened Formation	Water level (RL)
	Easting	Northing	Easting	Northing									
PZ08YJSW023	58466.37	9542	724603.66	7479697.6	524.06	94	50	20 - 92	32.8	7.77	912	CID	513.2
PZ08YJSW024	58681.29	10182.55	724818.62	7480338.26	524.18	100	50	26 - 98	15	7.85	973	CID	508.05
PZ08YJSW025	58678.31	10290.95	724815.64	7480446.68	525.74	86	50	19 - 85	18.8	7.75	960	CID	508.72
PZ08YJSW026	58591.09	10411.72	724728.41	7480567.46	533.08	50	50	21 - 48	1	8.03	1106	Weeli Wolli	507.5
PZ09YJSW011	57179.97	8423.47	723317.05	7478578.87	526.46	50	50	14 - 50	5	N/A	N/A	Alluvium	519.58
PZ09YJSW020	58847.65	9397.23	724985.01	7479552.81	519.95	50	50	14 - 50	2	7.93	843	Alluvium	513.35
<b>Total metres drilled</b>						<b>1690</b>							

Table 2 Junction South West Production Bore Completion Data

Bore number	Mine grid		MGA94		Top of Casing (RL)	Drill depth (m)	Steel Casing (324OD mm)	Slotted interval (m bgl)	Airlift Yield (L/sec)	pH	EC (µS/cm)	Screened Formation	Water level (RL)
	Easting	Northing	Easting	Northing									
WB08YJSW001	56358.09	8526.54	722495.03	7478681.95	530.6	88	324	15 - 75	31	8.02	995	CID	519.93
WB08YJSW002	56217.9	8387.27	722354.82	7478542.66	539.14	90	324	17.5 – 83.5	37	7.86	948	CID	519.65
WB08YJSW005	57848.18	8601.48	723985.38	7478756.92	533.41	80	324	20 - 74	31	7.82	767	CID	516.99
WB09YJSW003	57641	8558.97	723778.16	7478714.4	531.26	94	324	18 - 90	34.6	7.67	762	CID	517.05
WB09YJSW004	57778.02	8661.28	723915.21	7478816.72	528.94	89	324	11 - 83	34	7.75	1007	CID	517.13
WB09YJSW006	58448.7	9529.58	724586	7479685.17	524.16	90	324	18 - 84	35	7.74	670	CID	512.67
WB09YJSW007	58679.21	10166.07	724816.54	7480321.78	524.06	83	324	17 - 77	39	8.44	841	CID	508.34
<b>Total metres drilled</b>						<b>614</b>							

Table 3 Oxbow Monitoring Bore Completion Data

Bore number	Mine grid		MGA94		Top of Casing (RL)	Drill depth (m)	PVC Casing type (C112) (mm)	Slotted interval (m bgl)	Airlift Yield (L/sec)	pH	EC (µS/cm)	Screened Formation	Water level (RL)
	Easting	Northing	Easting	Northing									
PZ08YOXB001	51916.81	8727.81	718053	7478883.22	551.52	102	50	35 - 101	20	7.95	1005	CID	517.12
PZ08YOXB002	51918.56	8665	718054.75	7478820.4	550.55	102	50	34 - 100	18.8	7.92	940	CID	517.47
PZ08YOXB003	52171.85	8691.19	718308.08	7478846.6	544.45	56	50	31 - 55	N/A	N/A	N/A	Alluvium	518.13
PZ08YOXB004	52651.23	7781.62	718787.55	7477936.88	545.58	104	50	29 - 101	10.9	7.84	671	CID	520.52
PZ08YOXB005	52377.59	7514.63	718513.86	7477669.84	547.85	50	50	20 - 50	0.4	8.49	992	Alluvium	531.49
PZ08YOXB006	53799.93	8002.74	719936.45	7478158.04	534.15	90	50	18 - 90	11.9	7.88	871	CID	521.65
PZ08YOXB007	54544.78	7621.38	720681.42	7477776.62	538.33	56	50	26 - 56	0.76	8.04	1106	CID	522.48
<b>Total metres drilled</b>						<b>560</b>							

Table 4 Oxbow Production Bore Completion Data

Bore number	Mine grid		MGA94		Top of Casing (RL)	Drill depth (m)	Steel Casing (324OD mm)	Slotted interval (m bgl)	Airlift Yield (L/sec)	pH	EC (µS/cm)	Screened Formation	Water level (RL)
	Easting	Northing	Easting	Northing									
WB09YOXB001	51924.45	8742.11	718060.63	7478897.52	551.54	105	324	30 - 96	20	7.96	884	CID	516.32
<b>Total metres drilled</b>						<b>105</b>							

Table 5 Junction South East Monitoring Bore Completion Data

Bore number	Mine grid		MGA94		Top of Casing (RL)	Drill depth (m)	PVC Casing type (C12) (mm)	Slotted interval (m bgl)	Airlift Yield (L/sec)	pH	EC (µS/cm)	Screened Formation	Water level (RL)
	Easting	Northing	Easting	Northing									
PZ08YJSE001	64947.14	8106.06	731085.55	7478261.46	508.99	98	50	20 - 98	11	7.77	1043	CID	484.89
PZ08YJSE002	65034.63	8061.47	731173.06	7478216.86	508.65	92	50	20 - 92	12	7.85	1016	CID	486.62
PZ08YJSE003	64755.82	8230.52	730894.2	7478385.95	507.72	100	50	22 - 100	21.5	7.73	1085	CID	486.04
PZ08YJSE005	65793.4	7837.46	731931.96	7477992.82	499.69	39.1	50	9.1 – 39.1	2	8.5	1096	Alluvium	489.06
PZ08YJSE006	65989.89	7711.05	732128.48	7477866.39	500.18	48	50	24 - 48	2	8.48	1030	Alluvium	489.4
<b>Total metres drilled</b>						<b>377.1</b>							

Table 6 Junction South East Production Bore Completion Data

Bore number	Mine grid		MGA94		Top of Casing (RL)	Drill depth (m)	Steel Casing (324OD mm)	Slotted interval (m bgl)	Airlift Yield (L/sec)	pH	EC (µS/cm)	Screened Formation	Water level (RL)
	Easting	Northing	Easting	Northing									
WB08YJSE001	64936.18	8122.94	731074.59	7478278.34	508.84	98	324	25 - 91	18	7.67	996	CID	484.22
WB08YJSE002	65728.83	7875.77	731867.37	7478031.14	499.47	37	200 mm CL12 PVC	0 - 37	5	8.49	1007	Alluvium	488.52
<b>Total metres drilled</b>						<b>135</b>							

## 7.3 TEST PUMPING

### 7.3.1 Test pumping contract

Test pumping was conducted on nine bores between the 14<sup>th</sup> June and 1<sup>st</sup> October. The work was contracted to Test Pumping Australia Pty Ltd and supervised by RTIO hydrogeologists. Test pumping was carried using a vertical turbine Everflow 250FHM submersible pump rated in excess of 100 L/sec for higher yielding CID bores, and a vertical turbine Pomona 6HC rated to 25 L/sec for the lower yielding alluvial bore.

### 7.3.2 Test Procedures

Step rate tests were conducted on all the production bores to establish the pumping capacity of the bore and the capacity of the submersible pump. Each test comprised of three to five steps depending on the available drawdown. The step tests were conducted for 60 mins. Following recovery from the step test, production bores were subject to a constant rate test, with discharge rates selected based on the step test data.

Controlled constant rate discharge tests were conducted from two days up to 15 days. The tests were conducted to estimate aquifer hydraulics, and possibly determine boundary conditions. Aquifer characteristics obtained from test pumping included hydraulic conductivity (K), transmissivity (T), specific yield (Sy) and storage coefficient (S). Conventional manual curve matching methods were used to determine parameters followed by estimation of the same parameters by computer program (Aquifer Test Pro V 2.5).

The methods selected for analysis of the hydraulic properties of the unconfined aquifer depend primarily on the hydrogeological conditions at the test site, and includes several assumptions;

- The aquifer is homogenous, isotropic, uniform thickness and of infinite extent.
- The production bore fully penetrates the aquifer.
- The discharge from the pumping bore is constant throughout the pumping test.
- The flow to the aquifer is laminar.
- Transmissivity is constant with time.
- The aquifer is unconfined
- $Sy/S > 10$
- An observation bore screened over its entire length penetrates the full thickness of the aquifer
- The diameters of the pumped and observations bores are small, i.e. storage in them can be neglected

Calculations of hydraulic properties from the production bores were carried out using various monitoring bores by the Neuman method. The Neuman (1972) method of analysing the CID and alluvial monitoring bores was selected due to the unconfined delayed yield response and geology of the aquifer. A comparison to the unconfined aquifer analysis to the standard confined aquifer analysis of Cooper-Jacob (Time Drawdown) method and Theis method was conducted for the CID, giving representative results within the same order of magnitude. The results are tabulated in Appendix 5.

The main assumption for the calculation of specific yield by Newman equation is that the ratio of specific yield (Sy) to that of storage (S) should be greater than 10 ( $Sy/S > 10$ ). Where this assumption is satisfied the calculated Sy values for CID aquifer ranges from 0.1 to 0.01. The

few analysis where the assumption of  $S_y/S > 10$  were not satisfied, the  $S_y$  values were relatively low and therefore the result were not considered.

The Theis non-equilibrium distance drawdown flow equation was used to determine the extent of cone of depression or the permissible drawdown. The Theis equation assumes that flow towards the pumping bore would almost certainly be from the throughflow in the ore body and therefore be in a non-equilibrium or transient state.

$$h - h_0 = \frac{Q}{4\pi T} W(u) \quad (1)$$

$$S = \frac{4Tut}{r^2} \quad (2)$$

Where  $h-h_0$  is drawdown,  $Q$  is flow,  $T$  is transmissivity, and  $W(u)$  is the well function,  $t$  is time since pumping,  $S$  is the dimensionless storativity of the aquifer,  $r$  the radial distance from the discharge point and  $u$  is a dimensionless constant.

In addition to water levels and pumping rates, groundwater temperature, pH, dissolved oxygen (DO), and oxidation potential were monitored using a through flow cell attached to the pump in the production bore. Data was collected at 10 min intervals during step rate and constant rate tests using an In-situ Multi Parameter Troll 9500 water quality meter. The DO concentration was measured during test pumping by electrode and meter. The electrode measures the partial pressure of oxygen in the water, which is converted to oxygen mass weight concentration. The objective of using chemical and physical parameters is to identify geological and recharge boundary conditions. In particular, boundaries to the CID and recharge from the Marillana Creek alluvium aquifer during the long term pumping test. These parameters can provide information regarding boundary condition and inter-aquifer connections. The same parameters were recorded for Oxbow and Junction South East production bores.

### **7.3.3 The use of dissolved oxygen as tracer during pumping testing**

Field parameters such as electrical conductivity and pH measured for the CID, and alluvium/insitu weathered bedrock groundwaters during test pumping and Marillana Creek surface water were similar. Therefore identifying the source of recharge into the production bore using these parameters was not possible. The DO concentration is relatively higher in the Marillana Creek and the upper parts of the alluvium/insitu weathered bedrock and CID aquifers (~ 20 below water table) compared to groundwater in the deeper part of the aquifers (Figure 5).

The higher level of DO in the top 20 m of alluvium/insitu weathered bedrock and CID aquifer is due to the recharge of oxygenated water and equilibrium with atmospheric oxygen. The down hole profile of DO throughout the study area shows a decreasing trend from ~ 8 mg/L at the water table to close to zero in deeper (> 30 m below watertable) parts of CID and alluvium/insitu weathered bedrock aquifers. It is therefore anticipated that the DO concentration will change in the later part of the test pumping due to groundwater mixing between the deep and shallow groundwater.

Furthermore, the measured DO concentration in the Marillana Creek is in equilibrium with atmospheric  $O_2$ . The DO concentration of the flowing Marillana Creek water is ~ 8 mg/L indicating equilibrium with atmospheric  $O_2$ . The increased leakage and recharge from Marillana Creek during test pumping may result in increased levels of DO in groundwater due to mixing of stagnant water from deeper parts of the CID with oxygenated water from Marillana Creek.

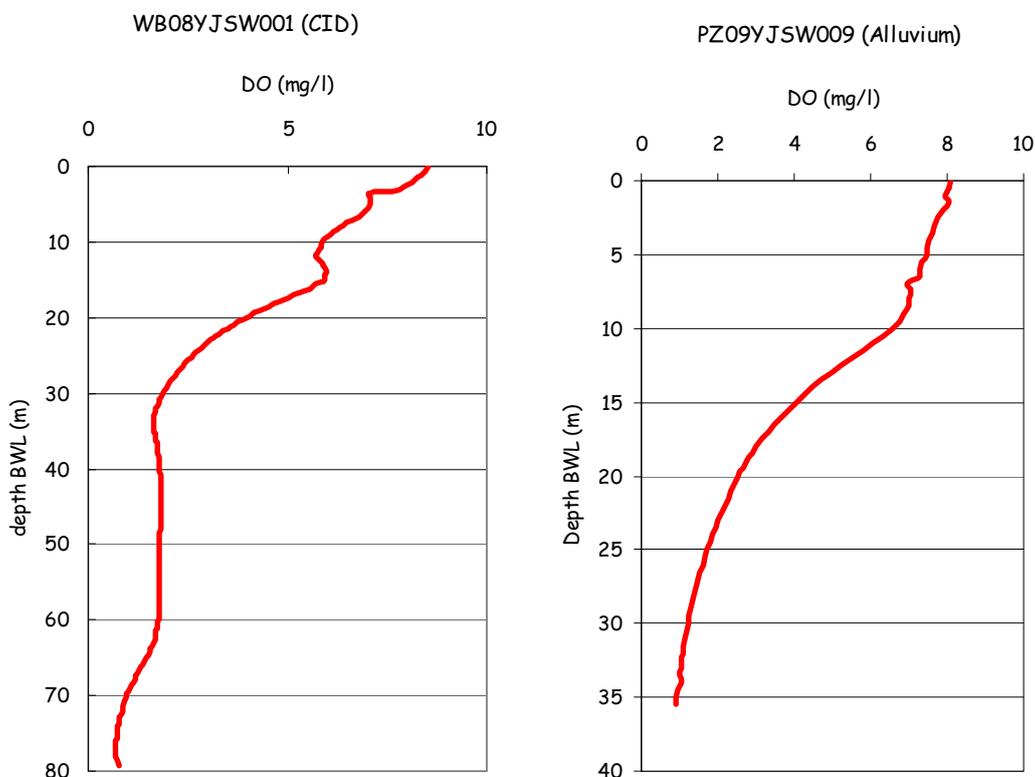


Figure 5. The DO concentration below the water table in a CID and an alluvium monitoring bore.

### 7.3.4 WB09YOXB001

One hour duration step tests were conducted on bore WB09YOXB001 for four different discharge rates; 24, 35, 45 and 60 L/sec respectively, to obtain the apparent well efficiency in the production bore performance and a rate for the constant discharge test. Drawdown for the four steps stabilised well within the hour. A constant discharge rate of 60 L/sec was selected based on the results of the step test analysis.

The apparent efficiency in the production bore performance ranges from 60-80% which indicates a reasonably efficient bore, with only a modest component of well loss in the pumping well. The negligible inefficiencies are likely the result of well loss as an effect of turbulent flow related to the relatively high pumping rates. The step test was analysed using Rorabaugh's equation and presented in Appendix 4.

Production bore WB09YOXB001, with a standing water level of 37.79 mbtc (meters below the top of the casing) was pumped at 60 L/sec for five days with the water level declining 19.66 m to 57.45 m. The time drawdown curve on the log-linear graph (Appendix 5) resembles the theoretical unconfined aquifer curve with a delayed yield or recharge boundary.

Early time data up to six minutes indicates water is released from storage within the bore. Then there appears to be a large low gradient response in the drawdown curve where the hydraulic head partially stabilises, indicating withdrawal of groundwater from available storage in the CID. After 17 hours pumping the drawdown trend steepened. The steeper slope may indicate the

boundary between the CID and the adjacent flood plain alluvial/insitu weathered bedrock aquifer. This may also explain the slight increase in the dissolved oxygen concentration in the groundwater. As the percentage of groundwater mixing from alluvial/insitu weathered bedrock aquifer to that of CID aquifer increases, the dissolved oxygen level increases with time.

The rise in water levels at approximately three and a half days was a result of faulty equipment which was rectified and pumping continued. The drawdown in the CID monitoring bores, ranged from 0.5 to 4.5 m depending on their distance from the production bore. The drawdown in the monitoring bore PZ08YOXB001 10 m from the production bore was 4.5 m indicating moderately transmissive material and no influence of recharge from Marillana Creek surface flow.

Water levels in PZ08YOXB003 and PZ08YOXB005 located 258 m and 1308 m from the pumping bore (Figure 2) drew down 0.7 m and 0.14 m respectively, indicating high transmissive material and good connection to the CID. Monitoring bore PZ08YOXB004, approximately 1200 m from the pumping bore, had 0.5 m drawdown showing the high transmissive nature of the CID.

A schematic cross section of the production and monitoring bores was drawn to show the water table elevation across the valley floor (Figure 7). The test pumping results and the response of the alluvium bore to the production bore suggest a well connected flood plain alluvium/insitu weathered bedrock to the CID aquifer. The Theis non-equilibrium distance drawdown flow equation was used to determine the extent of the cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from the constant rate test pumping analysis ( $T=2000 \text{ m}^2/\text{d}$  and  $S=0.01$ ). The results shows that at a distance of 260 m from the production bore drawdown should be approximately 0.7 m, which is what is observed in PZ08YOXB003. Due to the distance of Marillana Creek from the production bore no water table response can be expected in the Marillana creek. The non-equilibrium distance drawdown analysis also suggests that the boundary between the alluvium and Weeli Wolli Formation cannot be reached with the discharge rate used in five day test pumping.

The DO concentration during the test pumping is plotted against drawdown (Appendix 5). The DO content of groundwater in the CID aquifer is approximately 0.5 - 1 mg/L. The down hole DO profiles (Figure 6) for the two alluvium monitoring bores PX08YOXB003 and PZ08YOXB005 show higher concentration compared to that measured in the CID. The DO concentration after approximately one day of test pumping, where the CID/alluvial boundary was reached increased to ~ 1.5 mg/L indicating the increased percentage of mixing from groundwater derived from the alluvium. The redox potential mimics the dissolved oxygen concentration and increases from low potential of -86 mv to 164 mv at the end of the test pumping were groundwater is sourced from the dewatering of the upper sections of CID aquifer with a small percentage of alluvial/insitu weathered bedrock groundwater (Appendix 3). There is no apparent change in EC (averaging 900  $\mu\text{S}/\text{cm}$ ) during the test pumping because both aquifers are characterised by similar EC. The pH averaged 6.65 throughout the test.

The results for the drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method. The values of T, K and  $S_y$ , using the described methods are given in Appendix 5. The values for T for the CID range from 600 to 3000  $\text{m}^2/\text{d}$  adopting 2000  $\text{m}^2/\text{d}$ . The values for  $S_y$  range from 0.1 to 0.001, adopting 0.01. The adopted values were based on averages for the CID and comparisons with previous work for the area. The hydraulic conductivity, calculated by dividing the transmissivity by the 60 m aquifer thickness of the bore is 50 m/d. The flood plain alluvial/insitu weathered bedrock has a higher transmissivity of 3500  $\text{m}^2/\text{d}$  due to the unconsolidated nature of the material. The calculated specific yield from monitoring bores observations installed in the alluvial/insitu weathered bedrock aquifer provide higher values compared to the monitoring bores installed in the CID aquifer. The connection

between the aquifers was further illustrated by the measured dissolved oxygen (DO) concentration during pumping test.

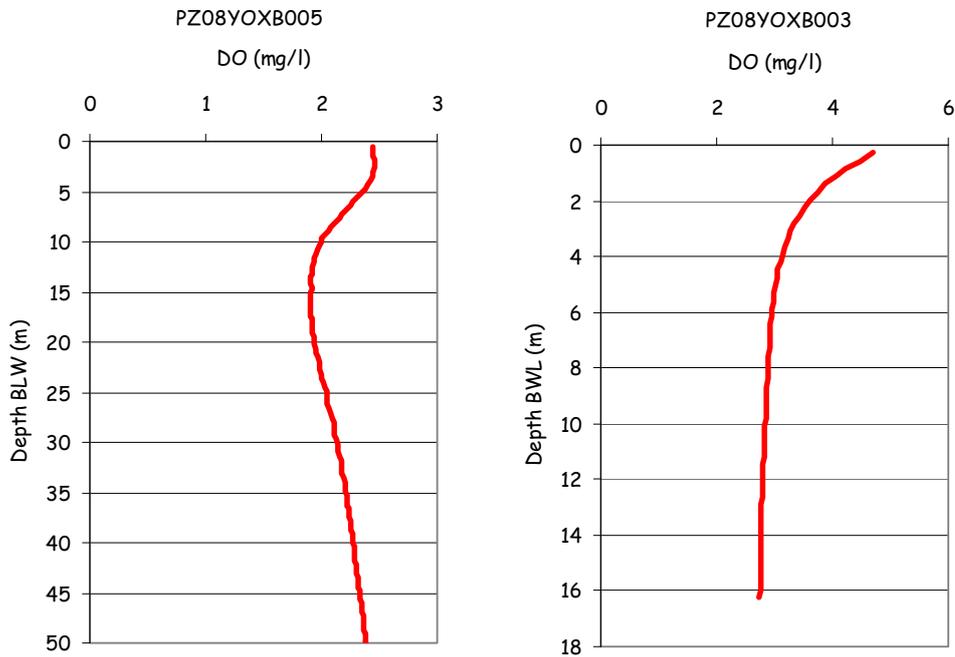


Figure 6 The down hole DO concentration in two alluvium bores located at the flanks of CID

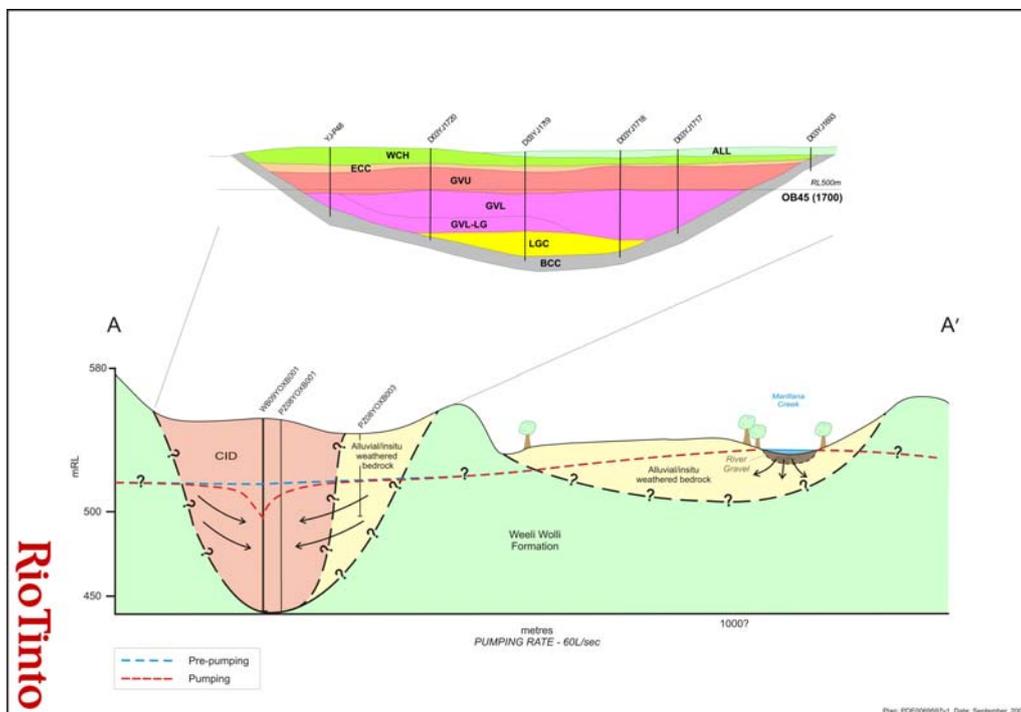


Figure 7 Schematic cross section of Oxbow pumping test.

### 7.3.5 WB08YJSW001

Four one hour duration step tests were conducted on bore WB08YJSW001 at discharge rates 32, 45, 60 and 80 L/sec respectively. Drawdown for the first three steps stabilised within the hour, but increasing the discharge to 80 L/sec did not stabilise and continued to decline. A constant discharge rate of 65 L/sec was selected based on the results of the step test analysis.

The apparent efficiency in the production bore performance ranges from 40-60% which indicates a significant component of well loss in the pumping well. The low efficiencies are likely the result of turbulent flow related to higher pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

Production bore WB08YJSW001, was pumped at 65 L/sec for five days. The groundwater level declined 47.85 m from the pre-pumping level of 11.15 mbtc to 59 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles the theoretical unconfined aquifer curve with a delayed yield.

Early time data up to 10 mins indicates the water is released from well storage. Then the curve partially stabilises, indicating withdrawal of groundwater from available storage in the CID. After approximately one and quarter days of pumping, the late time observations steepened indicating a decrease of flow through the CID aquifer. This response and an increase in dissolved oxygen suggests a boundary condition of the CID and flood plain alluvial/insitu weathered bedrock was reached. A further 10 m decline in drawdown after approximately 4.2 days was measured. A similar response was measured in all CID monitoring bores and may represent the extent of the palaeochannel and indicate the low permeability boundary of the Weeli Wolli Formation. The water level draw down in the CID monitoring bores, ranged from 5 to 10 m depending on distance from the production bore. 10 m of drawdown in the monitoring bore PZ08YJSW005, located 10 m from the production bore indicates relatively transmissive material and the influence of recharge from Marillana Creek surface flow.

Water level in PZ08YJSW004 and PZ08YJSW009 located 207 m and 516 m from the pumping bore (Figure 3) drew down 1 m and 1.8 m respectively, indicating high recharge influence from Marillana Creek and connection to the CID. Alluvial/insitu weathered bedrock monitoring bore PZ08YJSW004 located between Marillana Creek and the CID, had less drawdown compared to the alluvial/insitu weathered bedrock monitoring bore PZ08YJSW009 on the opposite flank of the CID. This may be due to the continuous recharge from the proximal Marillana Creek (despite the former being closer to the actual pumping bore).

The Theis non-equilibrium distance drawdown equation was used to determine the extent of the cone of depression or the permissible drawdown. The hydraulic parameters ( $T=450 \text{ m}^2/\text{d}$  and  $S=0.01$ ) used in the equation were obtained from the constant rate test pumping analysis. The equation shows that at a distance of 550 m from the production bore drawdown should be approximately 2 m, which would result in the Marillana Creek surface water flow delinking with the underlying alluvial/insitu weathered bedrock aquifer. Groundwater levels in PZ08YJSW009, 400 m from the production bore, would theoretically be drawn down 2.2 m which is close to what was observed. The theoretical drawdown of 4 m in PZ08YJSW004, drilled into the flood plain alluvials close to Marillana Creek was greater than observed due to the recharge from Marillana Creek.

Figure 8 illustrates the old conceptual model whereby the CID was interpreted to be flanked by impermeable basement Weeli Wolli Formation with a thin veneer of alluvial cover. A revised schematic cross section across the flood plain based on the results of drilling and test pumping was prepared. It illustrates the drawdown across the CID, alluvial/insitu weathered bedrock and Marillana Creek (Figure 9). The test results showed the flood plain alluvium/insitu weathered

bedrock and Marillana Creek gravels are well connected to the CID aquifer. It also demonstrates the delinking of the surface water flow from Marillana Creek from the underlying aquifer towards the end of pumping.

Dissolved oxygen, pH, electrical conductivity, temperature and redox potential were logged at 10 min intervals. The concentration of dissolved oxygen is plotted against drawdown (Appendix 5). The oxygen content of groundwater in the CID aquifer is approximately 2-3 mg/L at the start of the pumping test, and after approximately 1.5 days, increased to 7 mg/L when the CID/alluvial boundary was reached. This may be due to the mixing of shallow groundwater from the alluvium and potentially surface water from Marillana Creek. A percentage of Marillana Creek surface flow may recharge the CID aquifer where it crosses the CID palaeochannel 140 m up gradient. The measured redox potential mimics the dissolved oxygen concentration and increases steadily from 5 mv to 100 mv (end of the test pumping) where most of the groundwater is from the alluvium underlying Marillana Creek. There is no change in EC during the test pumping as the surface water from Marillana creek and groundwater of the CID and alluvium has similar EC<sup>1</sup> values.

The results for the production bore of drawdown data were analysed using the Theis recovery, Logan's formula and Neuman method. Transmissivity values ranged from 222 to 558 m<sup>2</sup>/d. The adopted value was 250 m<sup>2</sup>/d (from Neuman). The values of T, K and Sy, using the described methods are given in Appendix 5. These values for the CID range from 319 to 631 m<sup>2</sup>/d adopting 500 m<sup>2</sup>/d. The values for Sy range from 0.1 to 0.0001, adopting 0.01. The hydraulic conductivity, calculated by dividing the transmissivity by the 70 m aquifer thickness of the bore is 6.4 m/d. The flood plain alluvial/insitu weathered bedrock aquifer has a higher transmissivity of 1500 m<sup>2</sup>/d due to the unconsolidated nature of the material. Calculated specific yield in the alluvium gives similar values as the CID.

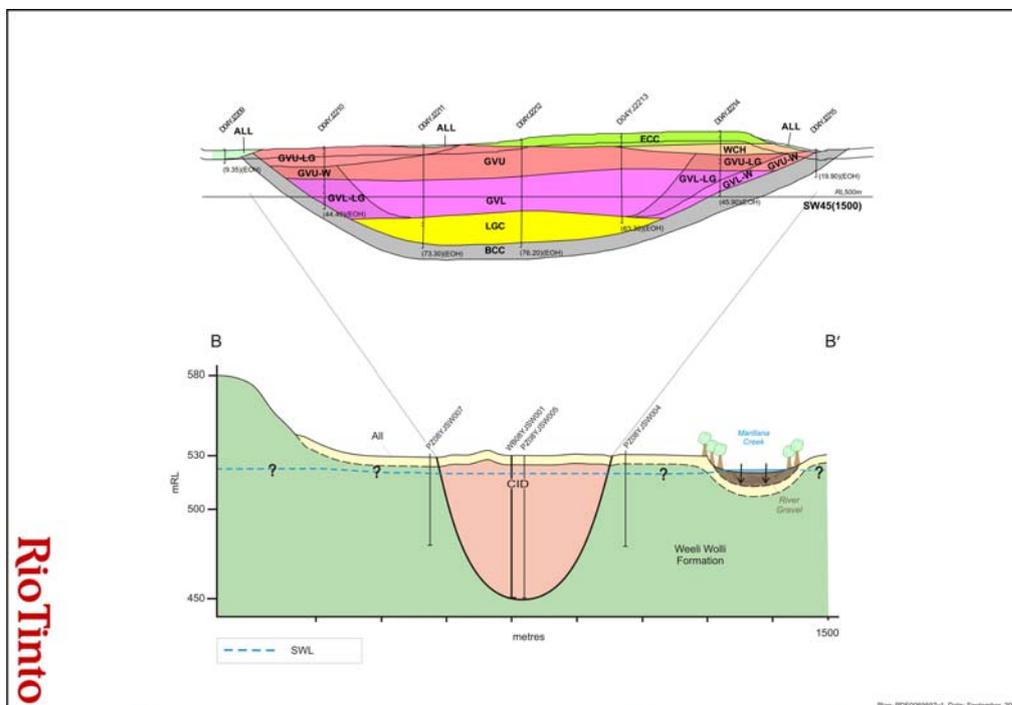


Figure 8 Conceptual model of cross section across the flood plain encompassing WB08YJSW001 bore prior to this study

<sup>1</sup> This to be expected as most surface water in Marillana Creek is actually derived from groundwater discharged as a result of BHPB dewatering operations upstream of RTIO Yandicoogina

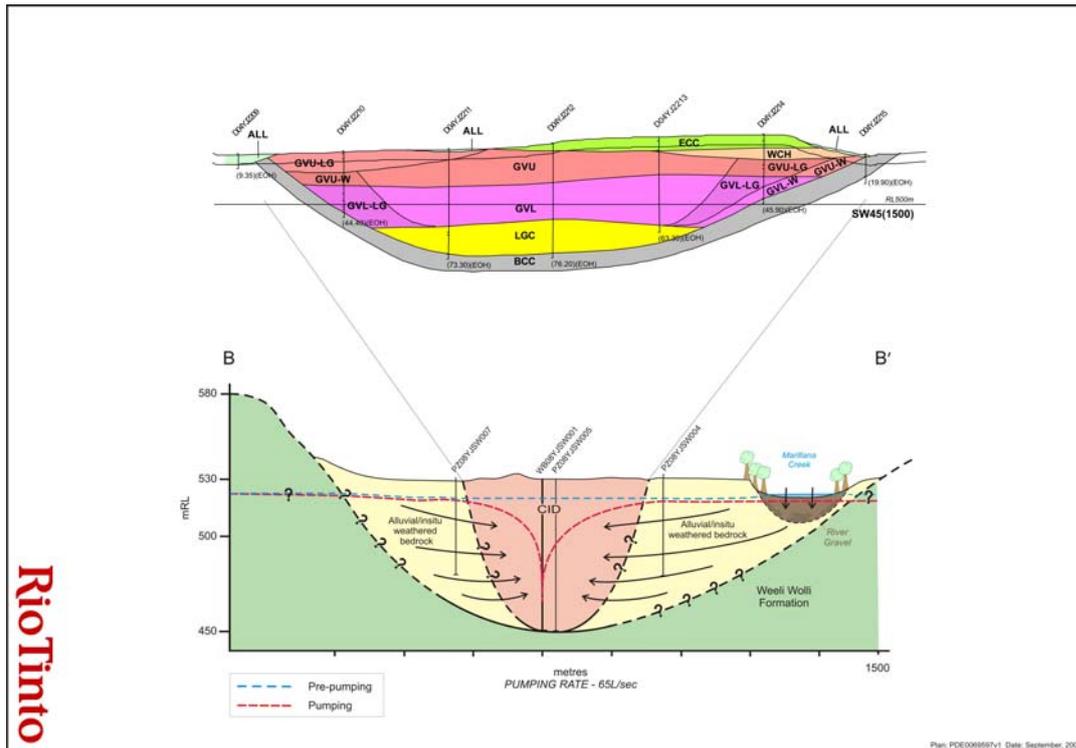


Figure 9 Conceptual model of cross section across the flood plain encompassing WB08YJSW001 drawn based on the results of this study

### 7.3.6 WB08YJSW002

Three one hour duration step tests were conducted on bore WB08YJSW002 at discharge rates 40, 50 and 80 L/sec respectively. Drawdown for the first two steps went close to stabilising within the hour, but increasing the discharge to 80 L/sec steepened the drawdown curve which only started to slightly stabilise towards the end of the test hour. The water level after the three steps at high pumping rates was lowered in the bore column to 43 m below standing water level. Based on the results of the step test analysis and comparison with adjacent production bore WB08YJSE001 a constant discharge rate of 60 L/sec was chosen.

The apparent efficiency in the production bore performance ranges from 23-37% which indicates there to be a significant component of well loss in the pumping well. The low efficiencies are likely the result of turbulent flow related to higher pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

Production bore WB08YJSW002, was pumped at 60 L/sec for two days. The groundwater level declined 38.19 m from the pre-pumping level of 20.78 mbtc to 58.97 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles the theoretical unconfined aquifer curve with a delayed yield. At early time data up to six minutes, water is released from well storage. Then there appears to be a slight decline in drawdown where water is released from storage within the CID to the point where the boundary of the flood plain alluvial/insitu weathered bedrock and the CID is reached. After 13 hours of pumping, the steeper decline in the drawdown curve, up (~8 m) to two days, indicates where the groundwater is obtained from the mixed storage of the CID and alluvial aquifers.

WB08YJSW002 was tested for a short duration for comparison with the adjacent production bore (WB08YJSW001). The two day constant rate test did not reach the boundary condition of

the alluvial and Weeli Wolli Formation. The steepened slope of the curve suggests a decrease in the flow to the production bore for the latter part of the test. The approximate 1 m drawdown in the flood plain alluvial bores after two days suggests that the water level was not lowered below the Marillana Creek bed.

The Theis non-equilibrium distance drawdown equation was used to determine the extent of the cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from the constant rate test pumping analysis ( $T=500 \text{ m}^2/\text{d}$  and  $S=0.01$ ). The equation shows that at a distance of 250 m (the distance of Marillana creek to the production bore) the water level will be lowered by approximately 3 m. This suggests that after two days of pumping, the Marillana Creek surface flow will be delinked from the underlying aquifer along the elongated palaeochannel.

Water level in PZ08YJSW004 and PZYJSW009 located 390 m and 512 m from production had approximately 1 m of drawdown after two days of pumping suggesting a good hydraulic connection between the alluvial/insitu weathered bedrock and the CID. The proximal monitoring bore to Marillana Creek had less drawdown due to the recharge from Marillana creek to the alluvium aquifer/insitu weathered bedrock.

The DO content of groundwater in the CID aquifer is approximately 2 mg/L for the first 300 minutes then sharply increases to approximately 8 mg/L, indicating that the water is sourced from the well connected alluvium/insitu weathered bedrock aquifer that is recharged by surface water from Marillana creek. The DO content in groundwater at the end of pumping resembles that of surface water in equilibrium with atmosphere, suggesting recharge from shallow alluvium and potentially surface water from Marillana Creek. The measured redox potential increases rapidly from -40 mv to 60 mv by the end of the test pumping were most of the discharge water is sourced from the adjacent alluvium/insitu weathered bedrock. There is a slight increase in EC during test pumping, but averaging 800  $\mu\text{S}/\text{cm}$  because the surface water from Marillana creek and groundwater of the CID and alluvium/insitu weathered bedrock has similar EC. The high concentration of DO corroborates the test pumping results that both production bores WB08YJSW001 and WB08YJSW002 are recharged mainly by shallow alluvium/insitu weathered bedrock aquifer by the end of test pumping.

The results for production bore WB08YJSW002 of drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method giving an adopted transmissivity of  $150 \text{ m}^2/\text{d}$ . The values of T, K and  $S_y$ , using these methods are given in Appendix 4. The values for T using the Neuman method for the CID range from 300 to  $600 \text{ m}^2/\text{d}$  adopting  $500 \text{ m}^2/\text{d}$ . The values for  $S_y$  range from 0.0001 to 0.1, adopting 0.01. The hydraulic conductivity, calculated by dividing the transmissivity by the 60 m aquifer thickness of the bore is 8.3 m/d.

### **7.3.7 WB09YJSW004**

Four one hour duration step tests were conducted on bore WB09YJSW004 at discharge rates 50, 70, 90 and 110 L/sec respectively. Drawdown for the first two steps went close to stabilising within the hour, but increasing the discharge to 90 L/sec and 110 L/sec continued to decline which resulted in a steepened drawdown curve which started to stabilise towards the end of each test hour. The fourth step was close to the pumps maximum pumping rate and lowered the water level in the bore column to 14.46 m below standing water level. Based on the results of the step testing a constant discharge rate of 95 L/sec was chosen.

The apparent efficiency in production bore performance ranges from 25-42% which indicates there to be a significant component of well loss in the pumping well. The efficiencies are likely the result of turbulent flow related to higher pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

Production bore WB09YJSW004, was pumped at 95 L/sec for 15 days. The groundwater level declined 16.53 m from the pre-pumping level of 13 mbtc to 29.61 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles close to the theoretical unconfined aquifer curve with a recharge boundary. The open cone of depression interpreted from the pumping data also suggests that the CID aquifer is unconfined.

At early time data up to two minutes on the drawdown curve, water is released from storage within the bore. Then a continued decrease in the slope of drawdown curve where water is released from storage within the CID to the point where the flood plain alluvial/insitu weathered bedrock and the CID boundary are reached after one hour of pumping. From this point a steeper constant decline in drawdown (~9 m) to 15 days, where the water is drawn from the CID, alluvial/insitu weathered bedrock and from the Marillana Creek surface water flow.

The drawdown versus time slope in the monitoring bores begins to stabilise suggesting a continuous recharge from the alluvium/insitu weathered bedrock aquifer. The proximity of the saturated Marillana Creek crossing the palaeochannel with the potential to recharge the CID 200 m down gradient provides the explanation for the minimal drawdown observed in the production bore. Mixing of the CID groundwater with that of the surface water from Marillana Creek and the underlying alluvium/insitu weathered bedrock aquifer is apparent in the graph of DO for the test duration. The constant dissolved oxygen data (~4 mg/L) measured throughout the whole test representing an aquifer readily recharged by oxygenated surface water.

The water level drawdown in the CID monitoring bores located in a 150 m radius of the pumping bore, ranged from 8.5 to 9.5 m depending on distance from the production bore. Drawdown of 9 m in the monitoring bores PZ08YJSW014 (7.5 m) and PZ08YJSW016 (92 m) from production bore was similar representing relatively homogenous transmissive material. After 15 days no apparent boundary with the Weeli Wolli Formation was reached, owing to the recharge from the alluvium/insitu weathered bedrock and nearby saturated river gravels.

The Theis non-equilibrium distance drawdown equation was used to determine the extent of the cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from the constant rate test pumping analysis ( $T=600 \text{ m}^2/\text{d}$  and  $S=0.1$ ). The equation shows that at a distance of 300 (distance Marillana creek to the production bore) the water level will be lowered by approximately 3.5 m. This suggests that after 15 days of pumping duration the Marillana Creek surface flow will be delinked from the underlying aquifer adjacent to the palaeochannel.

Water levels in PZ08YJSW017, PZ09YJSW011 and PZ09YJSW020 located 352 m, 650 m and 1291 m respectively, from the pumping bore parallel to Marillana Creek (Figure 2), had drawdowns of approximately 0.5 m, demonstrating again the connection between the flood plain alluvial/insitu weathered bedrock and the CID. PZ09YJSW020, over 1 km distance from the pumping bore had 0.4 m of drawdown demonstrating the highly transmissive material of the flood plain alluvial/insitu weathered bedrock.

Figure 10 illustrates the old conceptual model whereby the CID was interpreted to be flanked by impermeable basement Weeli Wolli Formation with a thin veneer of alluvial cover. Schematic section across the flood plain drawn from the production and monitoring bore logs illustrates the aquifer setting, the water level and drawdown during the test pumping (Figure 11). The test pumping results and the response of the alluvium/insitu weathered bedrock bores to the production bore suggest a well connected flood plain alluvium/insitu weathered bedrock and Marillana river bed gravel to the CID aquifer. The diagram also demonstrates the delinking of the surface water flow from Marillana Creek from the underlying aquifer towards the end of pumping.

The results for the production bore of drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method giving an adopted T of 500 m<sup>2</sup>/d. The values of T, K and Sy, using these methods are given in Appendix 4. The values for the Neuman method for T for the CID range from 500 to 600 m<sup>2</sup>/d adopting 600 m<sup>2</sup>/d. The values for Sy range from 0.001 to 0.1, adopting 0.1. The hydraulic conductivity, calculated by dividing the transmissivity by the 70 m aquifer thickness of the bore is 7.8 m/d.

The hydraulic properties calculated using the three 50 m deep monitoring alluvial bores (PZ09YJSW011, PZ09YJSW017 & PZ09YJSW020), which were close to the margins of Marillana Creek, provide higher transmissivities of average 3000 m<sup>2</sup>/d than CID monitoring bores. This is due to the larger distance of the alluvium/insitu weathered bedrock monitoring bores from the production bore utilised in test pumping equations.

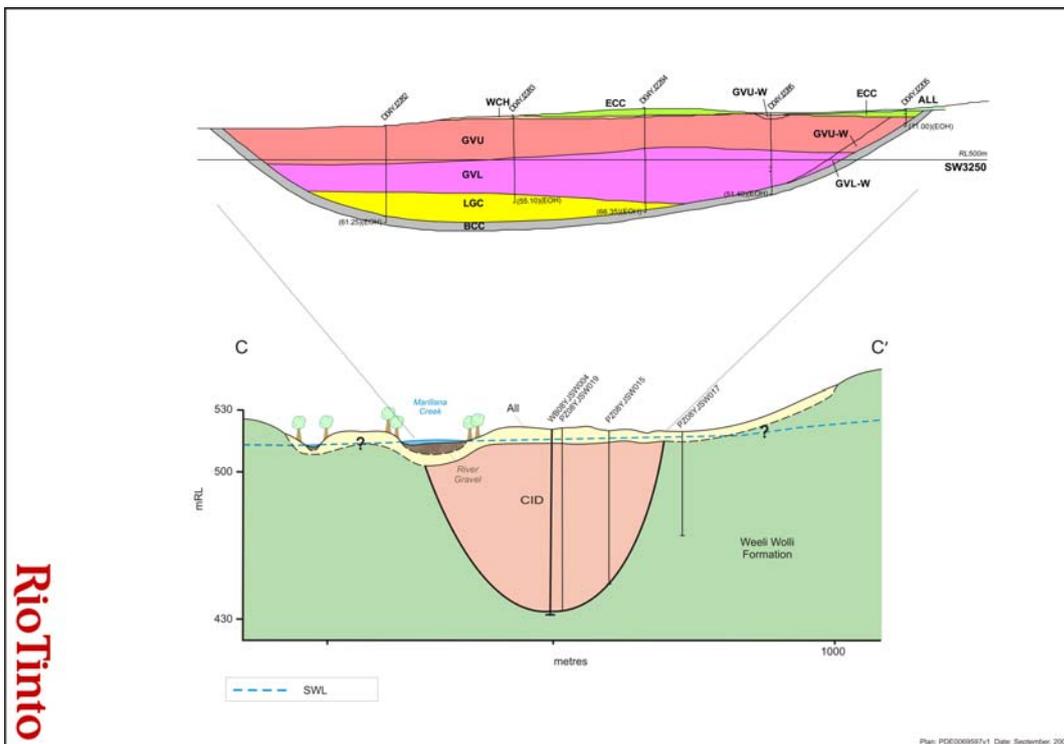


Figure 10 Conceptual model of cross section across the flood plain encompassing WB08YJSW001 bore prior to this study

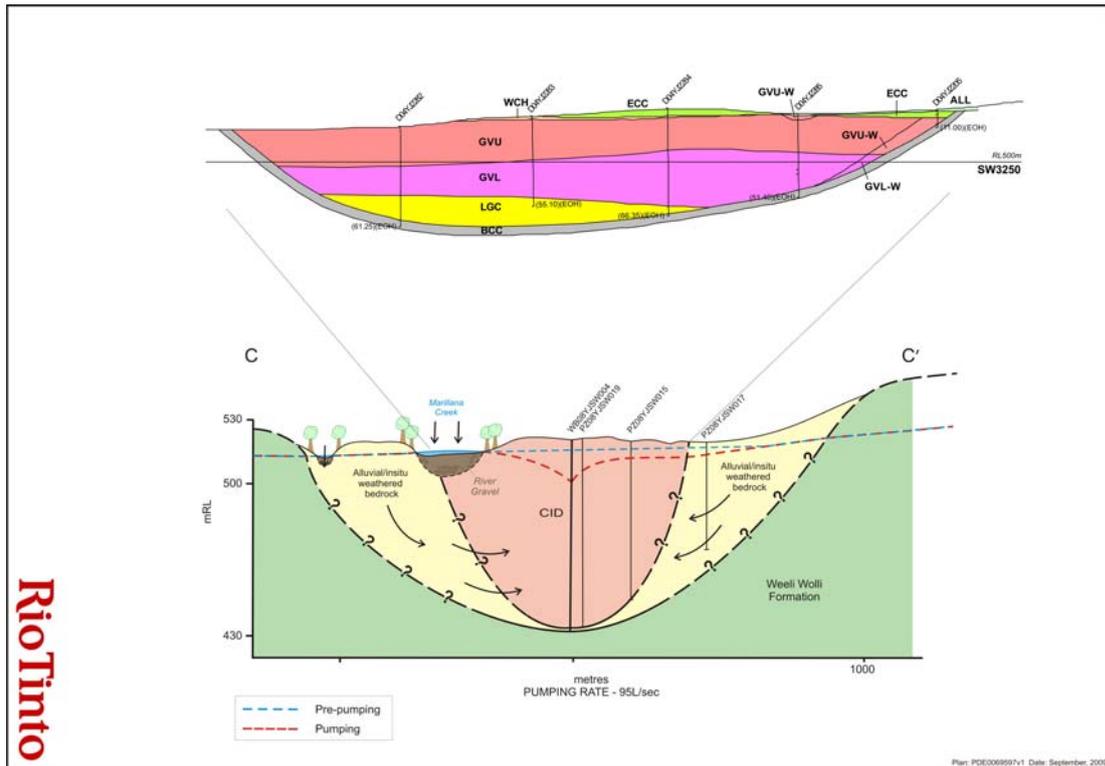


Figure 11 Schematic section across flood plain encompassing WB09YJSW004 production bore and associated monitoring bores. The geological model of CID is shown to illustrate width of CID compared to flood plain alluvium/insitu weathered bedrock.

### 7.3.8 WB09YJSW005

Four one hour duration step tests were conducted on bore WB09YJSW005 at discharge rates 50, 70, 90 and 110 L/sec respectively. Drawdown for the first three steps practically stabilised within the hour, but increasing the discharge to the pumps maximum pumping rate of 110 L/sec continued to decline the drawdown curve which only started to slightly stabilise towards the end of the test hour. Based on the results of the step test analysis a constant discharge rate of 100 L/sec was chosen.

The apparent efficiency in production bore performance ranges from 29-48% which indicates a component of well loss in the pumping well. The low efficiencies are the result of turbulent flow related to higher pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

Production bore WB09YJSW005, was pumped at 100 L/sec for three days. The groundwater level declined 37.55 m from the pre-pumping level of 17.82 mbtc to 55.37 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles close to the theoretical unconfined aquifer curve with a recharge boundary.

The early time data show that the water is release from storage within the well. Then there appears to be a decrease in drawdown versus time where water is released from storage within the CID to the point where the boundary of the flood plain alluvial/insitu weathered bedrock and the CID is likely to be reached. After ten hours of pumping a steeper decline in drawdown (~7

m) to three days, where the groundwater is obtained from the mixed storage of the CID and alluvial/insitu weathered bedrock aquifers.

The steep slope of the curve suggests decrease in flow to the production bore for the latter part of the test. The drawdown in all CID monitoring bores is approximately 8 m indicating a reasonably homogenous aquifer in the vicinity of the production bore. The alluvial bore PZ08YJSW017, located 277 m from the production bore had little to no drawdown due to the short duration of the test. However, the results of the three days of test pumping mimics that measured for the adjacent production bore WB09YJSW004.

Unlike the DO concentration in the adjacent WB09YJSW004 production bore, the DO content of groundwater in WB09YJSW005 increases from ~ 2 mg/l to 8 mg/L by the end of the three day constant rate test whereby more recharge from Marillana Creek is evident. The concentration represents a well mixed shallow groundwater from alluvium/insitu weathered bedrock and potentially surface water from Marillana Creek with the CID groundwater. The measured redox potential increases rapidly from -30 mv to 40 mv by the end of test pumping were most of the discharge water is sourced from the alluvium/insitu weathered bedrock underlying Marillana creek (Appendix 3).

Comparison of DO to the adjacent production bore (WB09YJSW004) suggests that the recharge boundary with the alluvium/insitu weathered bedrock was reached in a relatively shorter time. This is due to the dewatering of the aquifer by the WB09YJSW004 bore pumping testing. The 15 day test showed that the discharge water was derived from a well mixed CID and alluvial/insitu weathered bedrock aquifer where the DO was consistent throughout the test at approximately 4 mg/L. The results of DO concentration in the subsequent three day test of WB09YJSW005 shows water initially was derived from the deep CID aquifer to a point approximately 10 hours into the test, when the CID and alluvial/insitu weathered bedrock boundary was reached and the DO increased sharply to 8 mg/L.

The Theis non-equilibrium distance drawdown equation was used to determine the extent of the cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from the constant test pumping analysis ( $T=600 \text{ m}^2/\text{d}$  and  $S=0.1$ ). The equation shows that at a distance of 400 (the distance from Marillana Creek to the production bore) the water level will be lowered by 2 m. This suggests that after three days of pumping the Marillana Creek surface flow will be delinked from the underlying aquifer.

The results of drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method giving an average transmissivity of  $400 \text{ m}^2/\text{d}$ . The values of T, K and Sy, using these methods are given in Appendix 5. The values for T for the CID range from 300 to  $650 \text{ m}^2/\text{d}$  adopting  $600 \text{ m}^2/\text{d}$ . The values for Sy range from 0.1 to 0.01, adopting 0.1. The hydraulic conductivity, calculated by dividing the transmissivity by the 65 m aquifer thickness of the bore is  $9.2 \text{ m/d}$ .

### **7.3.9 WB09YJSW006**

Five one hour step tests of discharge rates of 40, 50, 60, 75 and 105 L/ were conducted on bore WB09YJSW006 respectively. Drawdown for the all steps began to stabilise within the hour, with overall drawdown after the five hours only 15 m. To be able to sustain a long term constant rate within the pump capacity 80 L/sec was chosen for a five day constant rate test.

The apparent efficiency in production bore performance ranges from 56-77% which indicates there to be a slight component of well loss in the pumping well. The inefficiencies are likely the result of turbulent flow related to high pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

Production bore WB09YJSW006, was pumped at 80 L/sec for five days. The groundwater level declined 14.15 m from the pre-pumping level of 12.2 mbtc to 26.35 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles close to the theoretical unconfined aquifer curve with a recharge boundary. The open cone of depression interpreted from the pumping data also suggests that the CID aquifer is unconfined.

At early time data up to five minutes, water is release from storage. This is supported by relatively low concentration of dissolved oxygen values of 2 -2.5 mg/L in the CID aquifer. Then there appears a change of the slope of the drawdown curve, indicating withdrawal of groundwater from aquifer storage.

At approximately 16 hours after pumping commenced a slight steepening in drawdown gradients is observed in the production bore and all monitoring bores. This coincides with a slight decrease in the dissolved oxygen indicating the decline in available volume of storage in the CID and pumping is sourced from a relatively deeper part of the aquifer. The late relatively steep decline in drawdown in the pumping bore at approximately four days after the commencement of the test is similar to that observed in WB08YJSW001 production bore. This may suggest the boundary of the palaeochannel and the Weeli Wolli Formation.

The water level draw down in the CID monitoring bores, ranged from 2 - 8 m depending on distance from the production bore. Drawdown of 7.9 m in the monitoring bore PZ08YJSW021, located 21m from production bore indicates relatively transmissive material. The Theis non-equilibrium distance drawdown equation was used to determine the extent of the cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from the constant rate test pumping analysis ( $T=800 \text{ m}^2/\text{d}$  and  $S= 0.01$ ). The equation shows that at a distance of 120 m (the distance from Marillana Creek to the production bore) the water level will be lowered by 2.8 m. This suggests that after five days of pumping the Marillana Creek surface flow will be delinked from the underlying aquifer.

The distance between Marillana Creek and WB09YJSW006 is only 120 m. The pre testing Marillana Creek water level is ~ 1m higher than the water level in the production bore, suggesting a losing stream and potential for leakage from alluvium/insitu weathered bedrock into the CID aquifer. The response in the alluvial/insitu weathered bedrock monitoring bore PZ09YJSW020 which was located on the eastern side of the Marillana Creek is minimal due the continuous recharge from Marillana Creek. The recharge from Marillana Creek towards the production bore creates a hydraulic divide that potentially drives the groundwater from the eastern side of the Marillana Creek towards the creek and not the production bore. Therefore the response of the groundwater on the opposite side of Marillana Creek to the production bore is minimal during the pumping test.

A Schematic cross section showing the production and monitoring bores illustrates the drawdown across flood plain including Marillana Creek (Figure 12), with minimal drawdown in the alluvial/insitu weathered bedrock bore on the opposite side of Marillana Creek. The test pumping results, and that the concentration of dissolved oxygen, the redox potential, pH, electrical conductivity and temperature of pumped groundwater remains constant throughout five days of pumping, suggests a well mixed groundwater derived from the CID and Marillana Creek surface water flow. The diagram also demonstrates the delinking of the surface water flow from Marillana Creek from the underlying aquifer towards the end of pumping.

The results for the production bore of drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method giving an adopted  $T$  of  $450 \text{ m}^2/\text{d}$ . The values of  $T$ ,  $K$  and  $S_y$ , using these methods are given in Appendix 5. The values using the Neuman method for  $T$  for the CID range from 550 to  $1600 \text{ m}^2/\text{d}$  adopting  $800 \text{ m}^2/\text{d}$  for prediction analysis.

The values for  $S_y$  range from 0.001 to 0.01, adopting 0.01. The hydraulic conductivity, calculated by dividing the Transmissivity by the 70 m aquifer thickness of the bore is 14.3 m/d. Due to the recharge from Marillana Creek surface flow, there was very minimal drawdown in the adjoining flood plain in the 50 m deep bore and therefore aquifer parameters could not be obtained using the alluvium/insitu weathered bedrock monitoring bore.

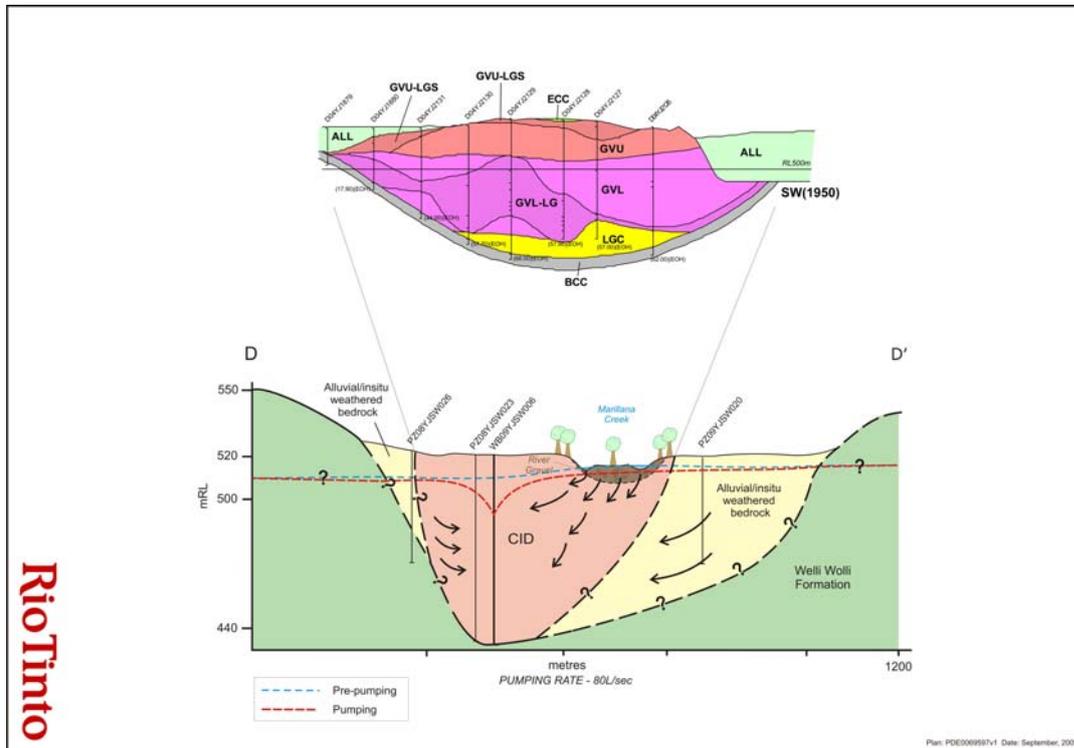


Figure 12 Schematic section across flood plain encompassing WB09YJSW006 production bore and associated monitoring bores. The geological model of CID is shown to illustrate width of CID compared to flood plain alluvium/insitu weathered bedrock.

### 7.3.10 WB09YJSW007

Four one hour duration step tests were conducted on bore WB09YJSW007 at discharge rates 50, 70, 90 and 110 L/sec respectively. Drawdown for the first two steps went close to stabilising within the hour, but increasing the discharge to 90 L/sec and 110 L/sec, close to the pumps maximum pumping capacity, continued to decline which only started to partially stabilise towards the end of each test hour. Based on the results of the step test analysis a constant discharge rate of 100 L/sec was chosen. The apparent efficiency in the production bore performance ranges from 43-62% which indicates there to be a reasonable component of well loss in the pumping well. The low efficiencies are likely the result of turbulent flow related to higher pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

Production bore WB09YJSW007, was pumped at 100 L/sec for five days. The groundwater level declined 21.35 m from the pre-pumping level of 25.4 mbtc to 37.18 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles the theoretical unconfined aquifer curve with a recharge boundary. The open cone of depression interpreted from the pumping data also suggests that the CID aquifer is unconfined.

The early time data up to four mins of the drawdown curve show a steep decline of water level suggesting that water is released from storage within the well. Then the slope of the drawdown time curve decreases indicating that the water is released from storage within the CID aquifer to the point where the boundary of the flood plain alluvial/insitu weathered bedrock and the CID is likely to be reached. After six hours of pumping the steeper decline in drawdown (~7.5 m) to 3.5 days results from, the groundwater being obtained from the mixed storage of the CID and alluvial/insitu weathered bedrock aquifers.

The sharp decrease in the slope of drawdown at approximately three and a half days represents the boundary between the well connected alluvial/CID aquifer and the Weeli Wolli Formation. The steep slope of the curve suggests decrease in flow to the production bore for the latter part of the test. The 2 m drawdown at 262 m in the basement monitoring bore PZ08YJSW026 suggests that the water level was lowered below the Marillana Creek bed.

The Theis non-equilibrium distance drawdown equation was used to determine the extent of cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from constant test pumping analysis ( $T=500 \text{ m}^2/\text{d}$  and  $S=0.001$ ). The equation shows that at a distance of 340 (distance Marillana creek to the production bore) the water level will be lowered by 5 m. This suggests that after 5 days of pumping the Marillana Creek surface flow will be delinked from the underlying aquifer.

Water level in PZ08YJSW026 located 262 m from the pumping bore on the northern border of the CID ore body, (Figure 3), had drawdown approximately 2 m indicating a high rate of flow from the basement to the CID aquifer.

The oxygen content of groundwater in the CID aquifer is approximately 3 mg/L throughout the five day test with a slight increase at two and a half days whereby more recharge from Marillana Creek is evident. The concentration represents well mixed shallow groundwater from the alluvium/insitu weathered bedrock and potentially surface water from Marillana Creek with the CID aquifer. The measured redox potential increases rapidly from 74 mv to 329 mv by the end of test pumping were most of the discharge water is sourced from alluvium/insitu weathered bedrock aquifer underlying Marillana creek. There is no apparent change in EC during the test pumping because the two end members (i.e. surface water from Marillana creek and groundwater of the CID and alluvium/insitu weathered bedrock) have similar EC.

A Schematic cross section showing the production and monitoring bores illustrates the relative water level and drawdown in the CID, flood plain and the Weeli Wolli Formation (Figure 13). The test pumping results and the response of the bores to the production bore suggest a reasonably well connected alluvium/insitu weathered bedrock and basement to the CID aquifer. The diagram also demonstrates the connection and influence of recharge from Marillana Creek to the CID aquifer. The results of drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method giving an average  $T$  of  $500 \text{ m}^2/\text{d}$ . The values of  $T$ ,  $K$  and  $S_y$ , using these methods are given in Appendix 4. The values for  $T$  for the CID range from 550 to  $500 \text{ m}^2/\text{d}$  adopting  $500 \text{ m}^2/\text{d}$ . The values for  $S_y$  range from 0.001 to 0.1, adopting 0.01. The hydraulic conductivity, calculated by dividing the transmissivity by the 60 m aquifer thickness of the bore is  $13 \text{ m}/\text{d}$ .

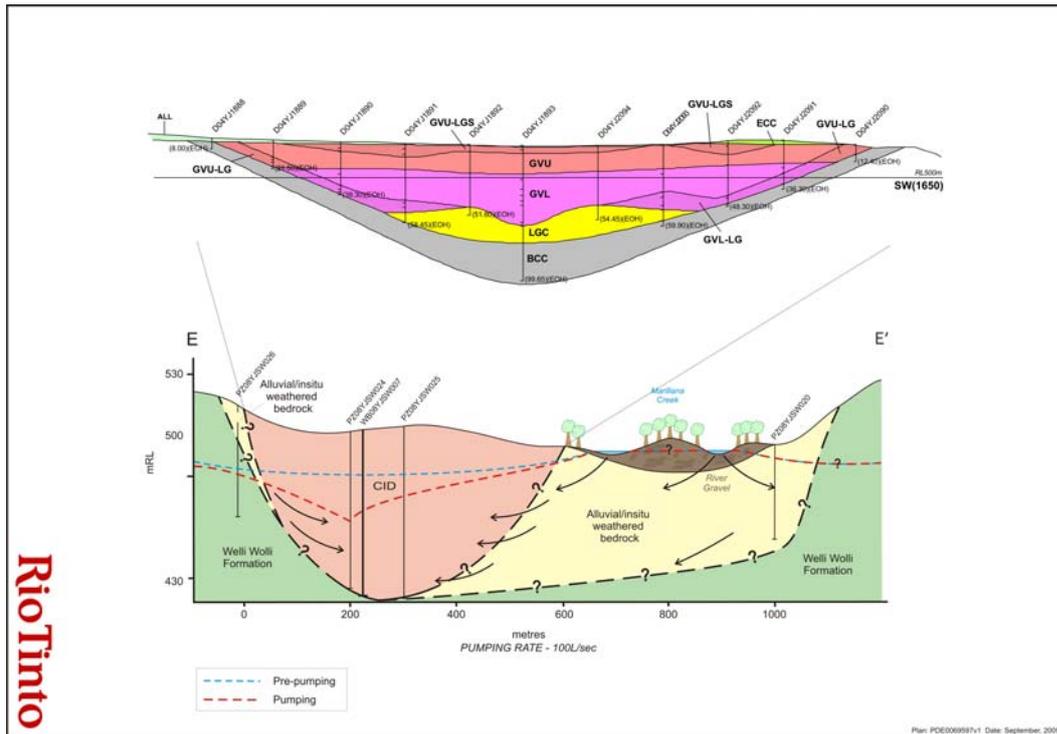


Figure 13. Schematic section across flood plain encompassing WB09YJSW007 production bore and associated monitoring bores. The geological model of CID is shown to illustrate width of CID compared to flood plain alluvium/insitu weathered bedrock.

### 7.3.11 WB08YJSE001

Four one hour duration step tests were conducted on bore WB08YJSE001 at discharge rates 43, 53, 65 and 80 L/sec respectively. Drawdown for the first three steps stabilised within the hour, but increasing the discharge to 80 L/sec did not stabilise and lowered the water level in the bore column to 32 m below standing water table. A constant discharge rate of 77 L/sec was selected based on the results of the step test analysis.

The apparent efficiency in production bore performance ranges from 13-21% which indicates there to be a significant component of well loss in the pumping well. The low efficiencies are likely to be the result of turbulent flow related to higher pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

Production bore WB08YJSE001, was pumped at 77 L/sec for five days. The groundwater level declined 32.62 m from the pre-pumping level of 25.4 mbtc to 58 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles the theoretical unconfined aquifer curve with a recharge boundary. The open cone of depression interpreted from the pumping data also suggests that the CID aquifer is unconfined.

At early time data up to five minutes, the water is released from storage in the water column (Appendix 5). Then the slope of drawdown versus time decreases suggesting that the water is released from storage in the CID aquifer where the drawdown is lowered by 10 m. The drawdown stabilises after five hours, indicating constant recharge into the CID by throughflow. The Production bore is located 300m down gradient from Marillana Creek crossing the CID aquifer. Therefore the constant recharge of the CID aquifer is due to the seepage from Marillana Creek to the alluvium/insitu weathered bedrock and mixing with groundwater in the

CID. This is corroborated by the increasing concentrations of dissolved oxygen in groundwater in the middle part of the curve. The higher than expected level of DO concentration during this part of the pump testing is likely to be due to the mixing between CID and shallow alluvium/insitu weathered bedrock groundwater. In the latter part of the curve where oxygen concentration stabilised at 7 mg/L resembling that of open water in equilibrium with atmosphere indicate the constant input from Marillana creek surface water into underlying CID aquifer.

The water level drawdown in the CID monitoring bores, ranged from 3.3 to 5.5 m depending on the distance from the production bore. Drawdown of 4.4 m in the monitoring bore PZ08YJSE001, located 20 m from production bore indicates relatively transmissive material and the influence of recharge from Marillana Creek surface flow. The response in the up gradient two monitoring bores PZ08YJSE003 and D03YJ1653, located between Marillana creek and the production bores are 3.3 m and 5.5 m. Unlike the down gradient monitoring bores the response of relatively more distant monitoring bore D03YJ1653 is higher (5.5 m) than the closer monitoring bore PZ08YJSE003 (3.3 m) by one fold.

The Theis non-equilibrium distance drawdown equation was used to determine the extent of cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from the constant rate test pumping analysis ( $T=1100 \text{ m}^2/\text{d}$  and  $S=0.001$ ). The equation shows that at a distance of 430 m (the distance from Marillana Creek to the production bore) the water level will be lowered by 1.3 m. This suggests that after five days of pumping the Marillana Creek surface flow will be delinked from the underlying aquifer.

The relatively smaller drawdown in PZ08YJSE003 bore is due to the higher rate of surface flow in the adjacent Marillana creek. The monitoring bore PZ08YJSE003 is located up gradient from D05 (Yandi operation discharge site) which discharges  $3000 \text{ m}^3/\text{d}$  into the creek. The higher rate of recharge from the creek results in the higher flow into the underlying aquifer and lower response in the monitoring bore. The discharge of surplus water into Marillana Creek at D05 will create a localised higher head down gradient with potentially higher flow into the underlying aquifer, therefore maintaining higher water levels compared to up gradient section of the CID aquifer.

A Schematic cross section showing the production and monitoring bores illustrates the water level and drawdown across the CID, alluvial/insitu weathered bedrock flood plain and Marillana Creek (Figure 14). The test pumping results and the response of the alluvium/insitu weathered bedrock bores to the production bore suggest a well connected flood plain alluvium/insitu weathered bedrock and Marillana river bed gravel to the CID aquifer. The diagram also demonstrates the connection and influence of recharge from Marillana Creek. A second schematic section was drawn along the strike of CID palaeochannel (Figure 15) illustrates the drawdown along the CID and potential recharge from the proximal Marillana Creek.

The results of drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method giving an adopted  $T$  of  $300 \text{ m}^2/\text{d}$ . The values of  $T$ ,  $K$  and  $S_y$ , using these methods are given in Appendix 4. The values obtained from the Neuman method for  $T$  for the CID range from 700 to  $200 \text{ m}^2/\text{d}$  adopting  $1100 \text{ m}^2/\text{d}$ . The values for  $S_y$  range from 0.5 to 0.01, adopting 0.1. The hydraulic conductivity, calculated by dividing the transmissivity by the 65 m aquifer thickness of the bore is  $23 \text{ m}/\text{d}$ .



### 7.3.12 WB08YJSE002

Four one hour duration step tests were conducted on the flood plain alluvial/insitu weathered bedrock testing bore WB08YJSE002 at discharge rates 5, 7, 10 and 13 down to 11 L/sec respectively. Drawdown for the first three steps stabilised within the hour, but increasing the discharge to 13 L/sec did not stabilise within the hour and had to be regulated back to 11 L/sec. With only 26 m of available drawdown, increasing the rate from 10 L/sec to 13 L/sec brought the water in the bore column down an extra 12 m to 32.7 m, close to the pump intake at 33 m. The fourth step test was therefore carried out by reducing the discharge rate to 11 L/sec. Based on the step test data, a constant discharge rate of 10 L/sec was selected.

The apparent efficiency in production bore performance ranges from 36-60% which indicates there to be a significant component of well loss in the pumping well. The low efficiencies are likely the result of turbulent flow related to higher pumping rates. The step test was analysed by using Rorabaugh's equation and presented in Appendix 4.

The flood plain alluvial testing bore WB08YJSE002, was pumped at 10 L/sec for 36 hours. The groundwater level declined 9 m from the pre-pumping level of 10.8 mbtc to 19.8 mbtc. The time drawdown curve on the log-linear graph (Appendix 5) resembles the theoretical unconfined aquifer curve with a recharge boundary. The open cone of depression interpreted from the pumping data also suggests that the CID aquifer is unconfined.

At early time data up to six minutes, water is released from storage within the bore. The drawdown curve then stabilises, indicating withdrawal of groundwater from available storage in the flood plain alluvium. At approximately 36 hours after pumping commenced the discharge rate was increased lowering the drawdown by 6.5 m with an instant response. The rapid response of the water level to higher discharge coincides with the significant increase in the concentration of dissolved oxygen in pumped groundwater (Appendix 5).

The water level drawdown in the flood plain alluvial/insitu weathered bedrock monitoring bores, ranged from 0.25 at a distance of 310 m and to 0.52 m at a distance of 75 m. The drawdown in the CID 507 m away showed response of 4 mm over the two day test. The rapid response of the alluvium bores to the discharge from production bore despite the relatively shallow bores (37m) indicates reasonable transmissive alluvial/insitu weathered bedrock material. The results of the test pumping also suggests that the boundary between alluvium/insitu weathered bedrock aquifer and Marillana Creek has not been reached probably due to low pumping rates and the distance of the Creek from the production bore. The layout of the bores and the drawdown across the alluvial/insitu weathered bedrock flood plain and the adjacent CID is shown in Figure 3.

The Theis non-equilibrium distance drawdown equation was used to determine the extent of cone of depression or the permissible drawdown. The hydraulic parameters used in the equation were obtained from constant test pumping analysis ( $T=700 \text{ m}^2/\text{d}$  and  $S=0.01$ ). The equation shows that at a distance of 180 (distance from alluvial bore to monitored CID bore) the water level will be lowered by 0.3 m. No apparent drawdown was observed after two days of continuous pumping of the flood plain alluvial/insitu weathered bedrock aquifer, suggesting a high hydraulic connection to the Marillana Creek 900 m away.

The concentration of dissolved oxygen is plotted against drawdown (Appendix 5). The oxygen content of groundwater in the alluvial/insitu weathered bedrock aquifer is approximately 2.8 mg/L and is constant throughout the test. Following 36 hours after pumping commenced the discharge rate was increased with an immediate response in the dissolved oxygen content to 7 mg/L. This reflects a greater influence from the shallower alluvium/insitu weathered bedrock aquifer water. This is also corroborated by an increase in the redox potential from 60 mV to 110

mV mimicking the dissolved oxygen. There is no apparent change in EC (Averaging 1100  $\mu\text{S}/\text{cm}$ ) during the test pumping which is reasonably higher than in the CID and the pH at 6.5 starts to increase slightly with the increase in discharge representing the influence of shallower water.

The results of drawdown versus time was analysed using the Theis recovery, Logan's formula and Neuman method giving an adopted T of 150  $\text{m}^2/\text{d}$ . The values of T, K and  $S_y$ , using these methods are given in Appendix 4. The values obtained from the Neuman method for T for the CID range from 600 to 800  $\text{m}^2/\text{d}$  adopting 700  $\text{m}^2/\text{d}$ . The values for  $S_y$  range from 0.01 to 0.001, adopting 0.01. The hydraulic conductivity, calculated by dividing the transmissivity by the 30 m aquifer thickness of the bore is 23  $\text{m}/\text{d}$ .

## 7.4 GROUNDWATER CHEMISTRY

Water samples were collected for each bore at the end of test pumping (Appendix 2). Total Dissolved Solids (TDS) in groundwater ranges from 548  $\text{mg}/\text{L}$  to 630  $\text{mg}/\text{L}$  throughout the CID aquifer. Chloride concentrations for the surface water in Marillana Creek are higher than that in the CID groundwater due to evaporation. The concentration of Cl in the Marillana Creek ranges from 90  $\text{mg}/\text{l}$  at the BHP discharge point to 140  $\text{mg}/\text{l}$  approximately 16 km down gradient from discharge point. The Cl concentration of the groundwater analysed at the end of pumping test is slightly elevated due to the mixing with the Marillana Creek surface water. This corroborates the pumping test analysis and the dissolved oxygen concentrations showing that the groundwater is a mixture of the two end members namely surface water of Marillana Creek with average Cl concentrations of 130  $\text{m}/\text{L}$  and CID groundwater with average Cl concentration of 85  $\text{mg}/\text{L}$ .

## 7.5 BOREFIELD DESIGN

Optimum dewatering rates in the vicinity of the production bore were calculated using a source reliable output spread sheet. The number of production bores, and distances between production bores were determined. The input parameters to the model used in the spread sheet were obtained from test pumping analysis. To achieve dry mining conditions in the vicinity around each bore six months was assumed. This analysis aims to predict the impact of interference effects of adjacent production bores and subsequent drawdown. Borefields configurations were designed either in a straight line up gradient perpendicular to the channel or as clusters around the centre of the CID. The optimal design to yield maximum drawdown is a cluster bore field in the centre of palaeochannel. Here the deepest part of the channel is located in the centre of CID. The model was run for six sites to calculate the rate of pumping to achieve maximum drawdown. The modelling results indicate a cluster of five production bores will be needed to achieve required drawdown (200 m radius) from the borefield.

The calculations assume transient radial flow, laminar flow and apparent well loss and utilises the parameters obtained from the step test analysis. Assuming constant distances between the bores and using a constant discharge rate for the bore field, the interference effects are calculated using T and S derived from the test pumping analysis. Table 7 shows the recommended pumping rates for each borefield around the initial production bore. For a five cluster borefield around the initial oxbow production bore (WB09YOXB001), with the pump inlet set at 93 m below ground level, a rate of 7500  $\text{m}^3/\text{d}$  per bore will be required to achieve maximum drawdown. The estimates obtained from this method should be considered indicative only as the equation used does not account for the increased recharge from surface water flow where interference effects may be greater than estimated. The recommended discharge rates are presented in Table 7.

Borefield	SWL (mbgl)	Pump Setting (mbgl)	Pumping Water Level (mbgl)	Recommended Rate (ML/d)
WB09YOXB001	35.2	93	90	37.5
WB08YJSW001	10.6	72	65	25
WB09YJSW004	11.8	77	75	50
WB09YJSW006	11.5	78	75	62.5
WB09YJSW007	15.7	71	69	60
WB08YJSE001	24.6	91	90	43.5

Table 7 Production Borefield optimum dewatering rates after 6 months

The optimum borefield design to dewater pits JSW-B and JSW-C on the northern side of Marillana Creek included three cluster borefields each comprising of three production bores. The three cluster borefields along the axis of palaeochannel each 600 m apart. To achieve dry mining conditions a pumping rate of 45,000 m<sup>3</sup>/d (16.4GL/a) was calculated. The capacity of the production bores drilled in this area range from 70 – 100 L/sec. Taking a conservative pumping rate of 60L/sec, and using variable speed driven pumps the drawdown to the base of ore body can be achieved within 12 months. The abstraction volumes of 16.4 GL/a obtained from this method is comparable to the results derived from empirical modelling used for Yandicoogina Order of Magnitude study.

## 7.6 PROJECT COSTS

Project costs were recorded from the daily plod sheets and confirmed during project invoicing. A breakdown of major costs within the project is provided in Table 8 below. Some of the ancillary costs associated with the program such as flights, accommodation and supervision were not recorded for this report as only drilling costs were measured for an average cost per metre.

Description	Actual Costs (based on invoices)
Drilling	\$ 640,149.00
Work time	\$ 643,440.00
Standby	\$ 151,072.00
Mobilisation	\$ 23,625.00
Supply and deliver materials	\$ 601,033.25
Muds and chemicals	\$ 55,462.76
Other	\$ 45,500.00
Test pumping	\$ 236,000.00
Total	\$ 2,396,282.01

\*Note: Other includes the use of auxiliary equipment.

Table 8 Break down of the project costs.

The calculated cost per metre for the program was \$621/m (3,481.1 m at \$2,160,282.01). The cost per metre is high, largely due to the cost average which does not take into account the hole diameter, materials for production bores and the amount of production bores drilled. The average cost of the step and constant rate discharge tests per bore was approximately \$26,000.

## 8. SUMMARY

This investigation comprised exploration drilling, bore construction, test pumping and hydrochemical sampling. The hydrogeological drilling programme of 34 monitoring and 10 production bores was completed throughout the expanding Yandicoogina deposits. Drilling was carried out using conventional air/percussion methods with a number of bores in unconsolidated alluvials drilled using mud/rotary techniques. Monitoring bores were constructed with 50 mm PVC, and production bores cased with 324 mm steel casing.

The test pumping results of the nine production bores along the Marillana Creek indicate a good hydraulic connection between CID and alluvium/insitu weathered bedrock aquifers. The average distance of the production bores and alluvium/insitu weathered bedrock monitoring bores installed outside the CID boundary is 250 m. Despite this considerable distance, water levels in the monitoring bores responded within two days of continuous pumping. Furthermore, the data on the drawdown versus time plot for most of the monitoring bores mimics that of the production bore suggesting similar hydraulic characteristics in the CID and alluvium/insitu weathered bedrock.

Based on the information obtained from test pumping the hydraulic properties of the CID and alluvium aquifer, and the conceptual model for the groundwater flow system underlying the flood plains of Marillana Creek was updated. The new conceptual model comprises a CID that is surrounded by relatively transmissive alluvium/insitu weathered bedrock that is in direct connection with the CID. The river gravel underlying Marillana Creek and overlying the alluvium/insitu weathered bedrock contributes a significant volume of water to the CID aquifer. The new conceptual outline of the flood plain of Marillana creek is shown in figures (5 to 14). The combined width of the CID and alluvium/insitu weathered bedrock aquifers range from ~1000 to ~2000 m. The maximum depth is 110 m in the middle of the CID channel diminishing towards the flanks of the flood plain.

The updated model has important implications for the dewatering of proposed and existing pits at JSW, Oxbow and JSE. The direct contribution of Marillana Creek surface water to the CID and the relatively large storage capacity of the alluvium/insitu weathered bedrock surrounding CID indicate that drawdown in the CID aquifer during dewatering results in considerable leakage from the saturated alluvium/insitu weathered bedrock and creek bed. The prediction of the dewatering volume to achieve dry mining conditions will be accomplished by using the updated conceptual model of the flood plain to carry out groundwater modelling for the whole RTIO Yandicoogina mining area.

The results of the source reliable output suggests that borefields in clusters are more efficient than locating multiple bores perpendicular to the axes of CID aquifer. The relatively high transmissivity of the alluvium may provide opportunity to partially dewater the pits from the alluvium (i.e. ex-pit). However more drilling is required to confirm the depth of the alluvium and the hydraulic properties of the deeper sections of the alluvium aquifer. The pump testing of the shallow (37 m) bore in the alluvium/insitu weathered bedrock at JSE area has shown that this option might be possible particularly on the boundary between CID and the alluvium/insitu weathered bedrock.

The DO concentration and redox potential measured during test pumping provide valuable information on the degree of hydraulic connection and mixing of CID and alluvium/insitu weathered bedrock aquifers. The increased trend of DO concentration during most of the test pumping suggests direct contribution from Marillana Creek surface water to the underlying alluvium and CID aquifers. This was particularly evident in the two production bores

immediately down gradient from Marillana Creek at WB08YJSW001 and WB08YJSE001. The distance of these bores from Marillana Creek is ~ 250 m and recharge from Marillana Creek was observed after 18 hours of pumping.

The high yield from the constant rate test pumping ranged from 60L/s up to 100 L/s suggests that the dewatering of the pits is possible with fewer bores than previously anticipated. The high end of the pumping was limited to ~ 100 L/s because of the capacity of the pump, the diameter and length of the discharge pipe. Discharge with higher rates up to 150 L/s is possible from several production bores if the discharge outlet were designed to handle higher volumes.

## 9. REFERENCES

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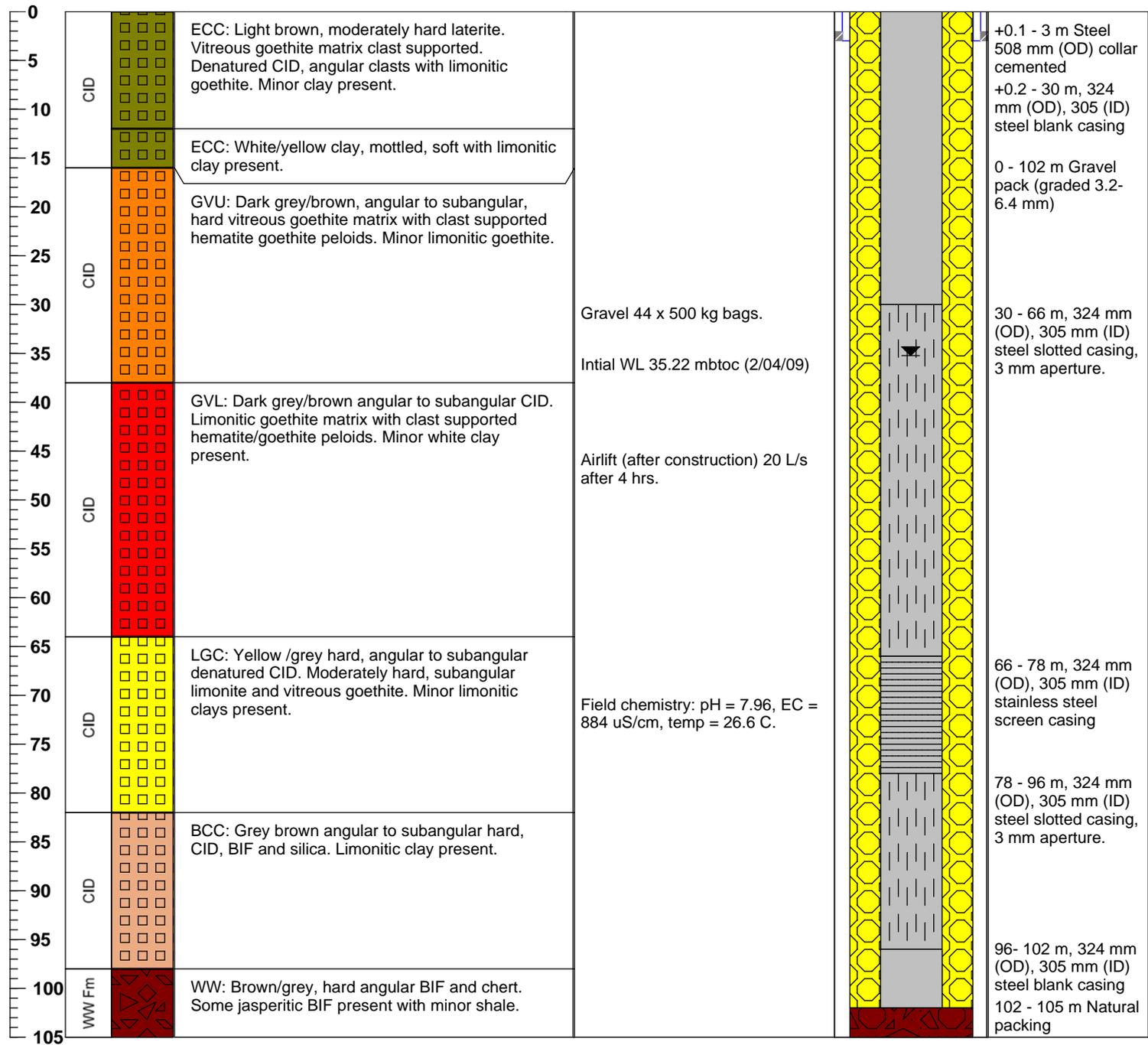
## **Appendix 1: Bore logs**

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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Oxbow	<b>DRILLER:</b> Michael Larkman	<b>EASTING:</b> 718060.63
<b>DATE COMMENCED:</b> 25-Mar-09	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478897.52
<b>DATE COMPLETED:</b> 28-Mar-09	<b>HYDROGEOLOGIST(s):</b> Niall Inverarity	<b>ELEVATION:</b> 551.54 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-3 m, 17.5 inch air/hammer 3-105 m.

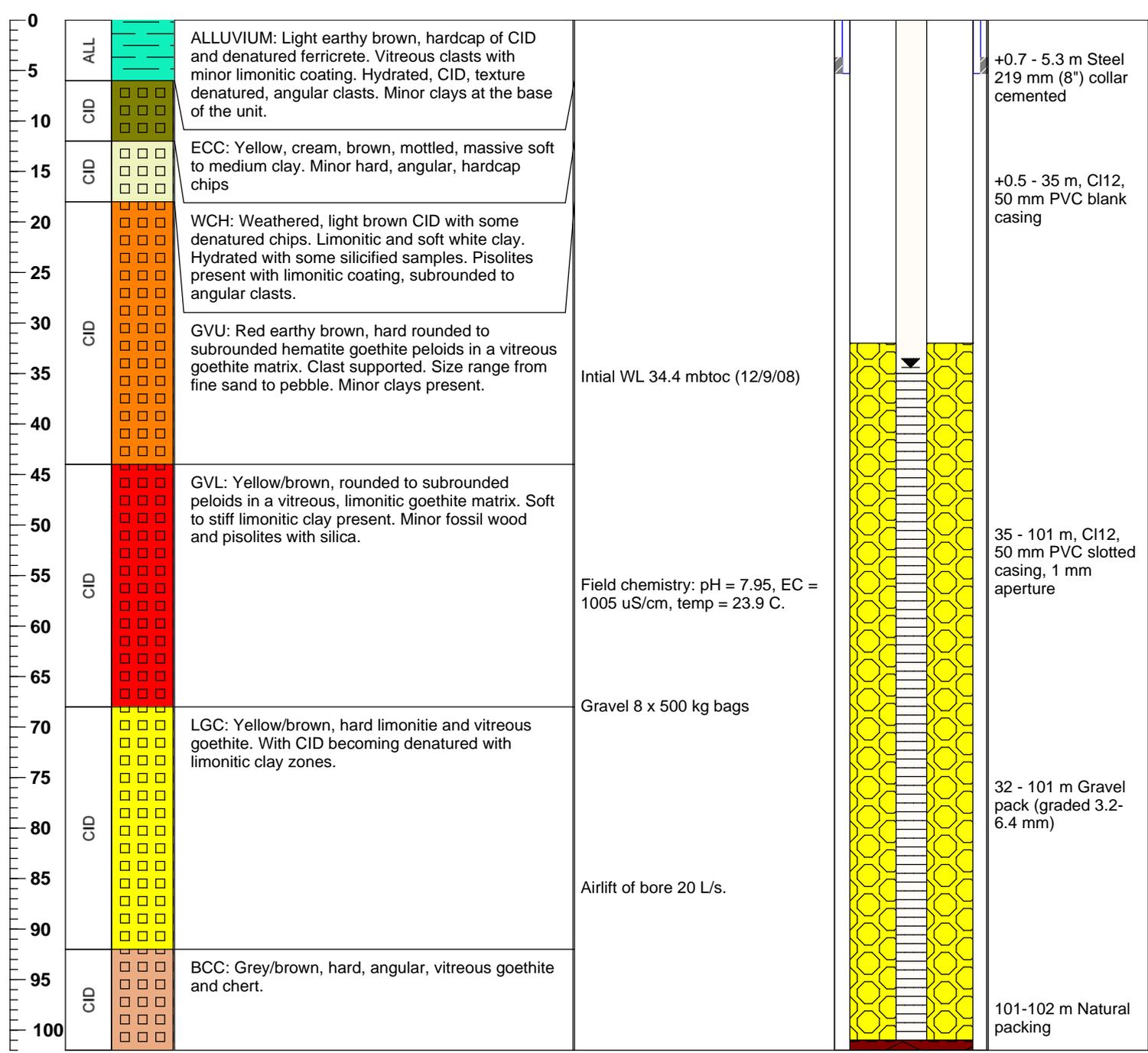
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Oxbow	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 718053.0
<b>DATE COMMENCED:</b> 8-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478883.22
<b>DATE COMPLETED:</b> 9-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 551.52 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-102m.

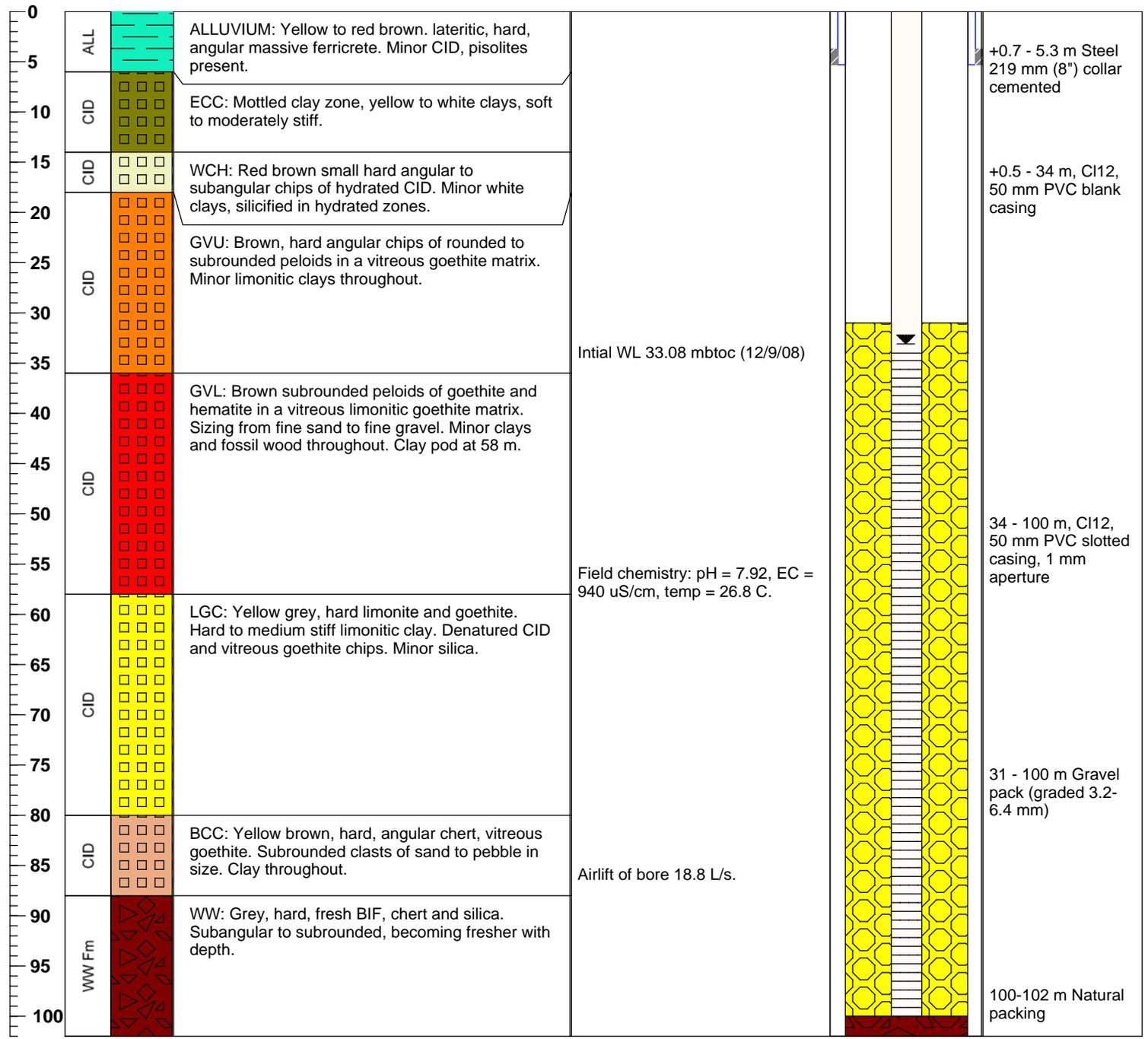
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Oxbow	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 718054.74
<b>DATE COMMENCED:</b> 9-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478820.39
<b>DATE COMPLETED:</b> 11-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 550.55 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-102m.

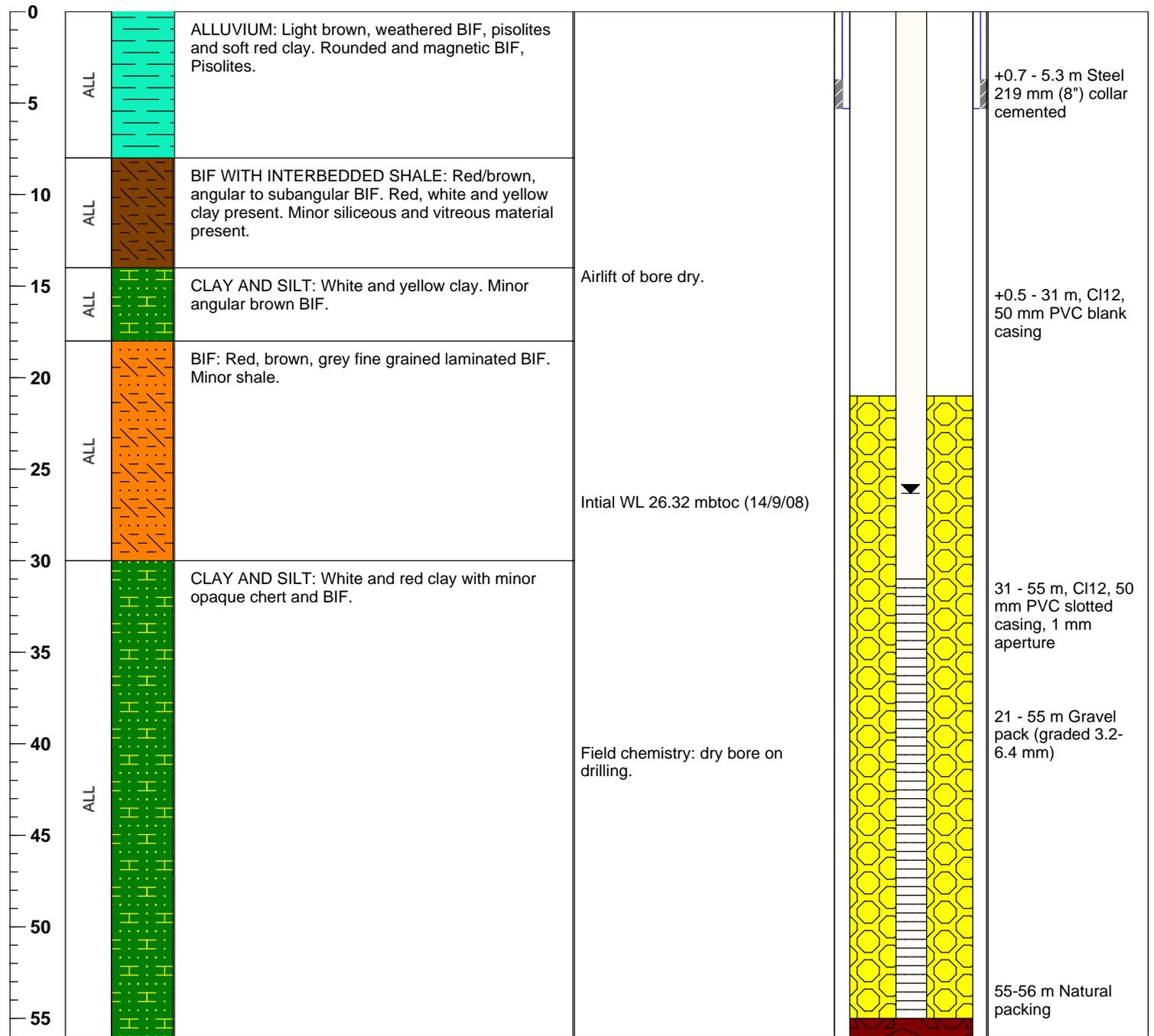
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Oxbow	<b>DRILLER:</b> Ross Pendelbury	<b>EASTING:</b> 718308.08
<b>DATE COMMENCED:</b> 11-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478846.6
<b>DATE COMPLETED:</b> 12-Sept-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 544.45 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-56m.

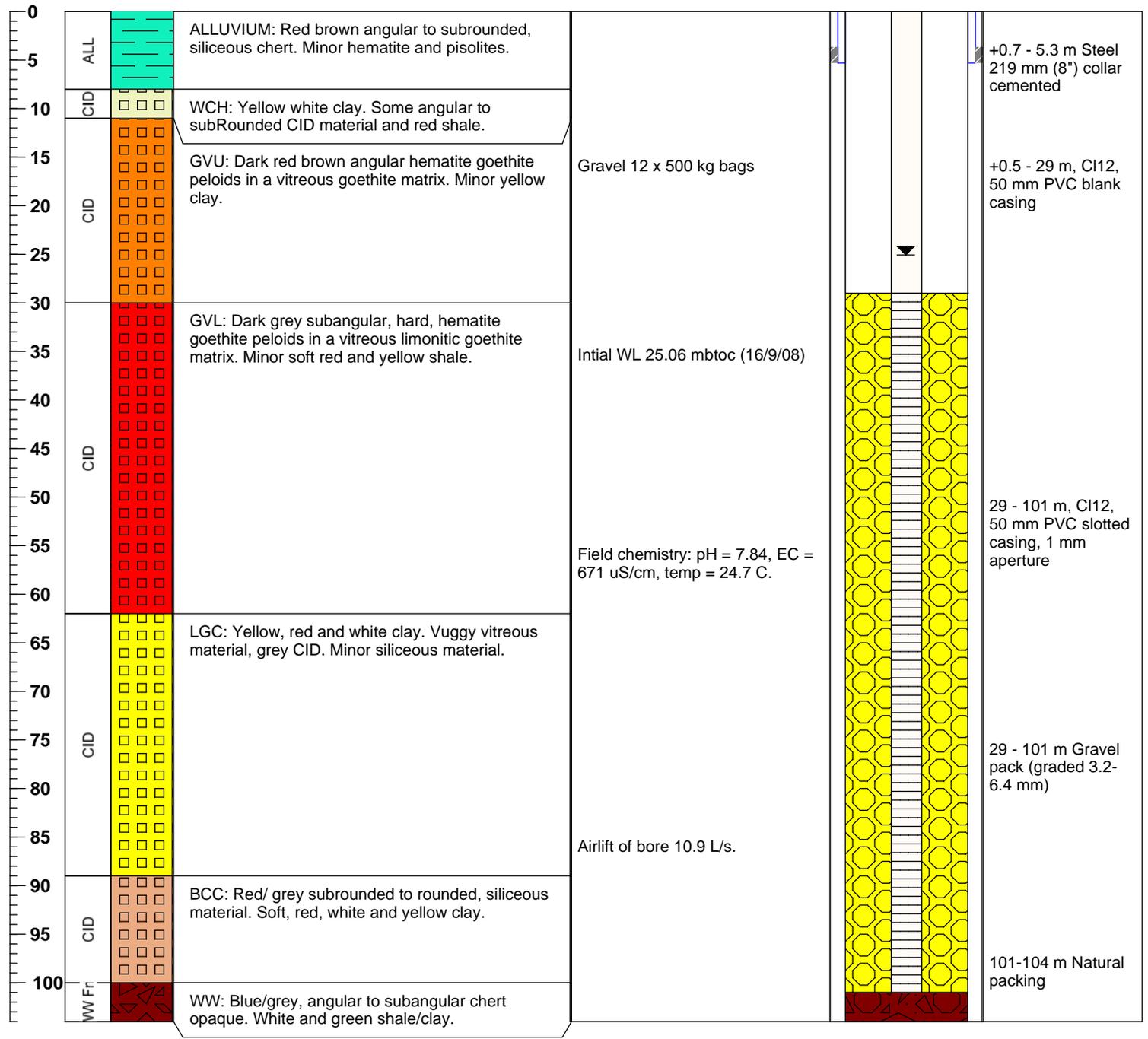
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Oxbow	<b>DRILLER:</b> Ross Pendelbury	<b>EASTING:</b> 718787.55
<b>DATE COMMENCED:</b> 12-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7477936.88
<b>DATE COMPLETED:</b> 14-Sept-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 545.58 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-104m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Oxbow  
**DATE COMMENCED:** 14-Sept-08  
**DATE COMPLETED:** 17-Sept-08

### DRILLING DETAILS

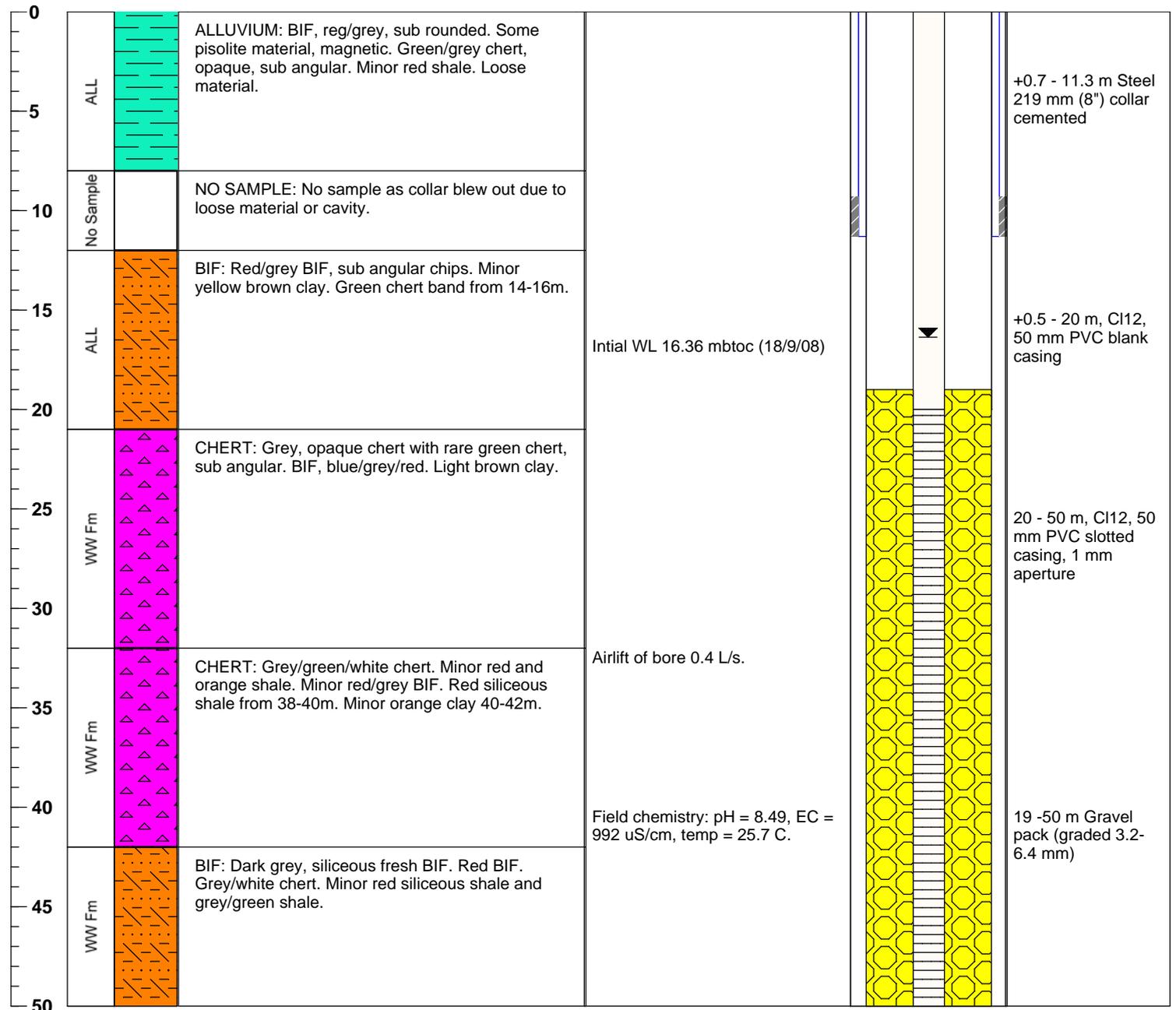
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Ross Pendelbury  
**DRILLING METHOD:** Air/Hammer  
**HYDROGEOLOGIST(s):** Emma McGiven

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 718513.86  
**NORTHING:** 7477669.84  
**ELEVATION:** 547.85 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-11.3m, 7&7/8 inch hammer 11.3-50m.

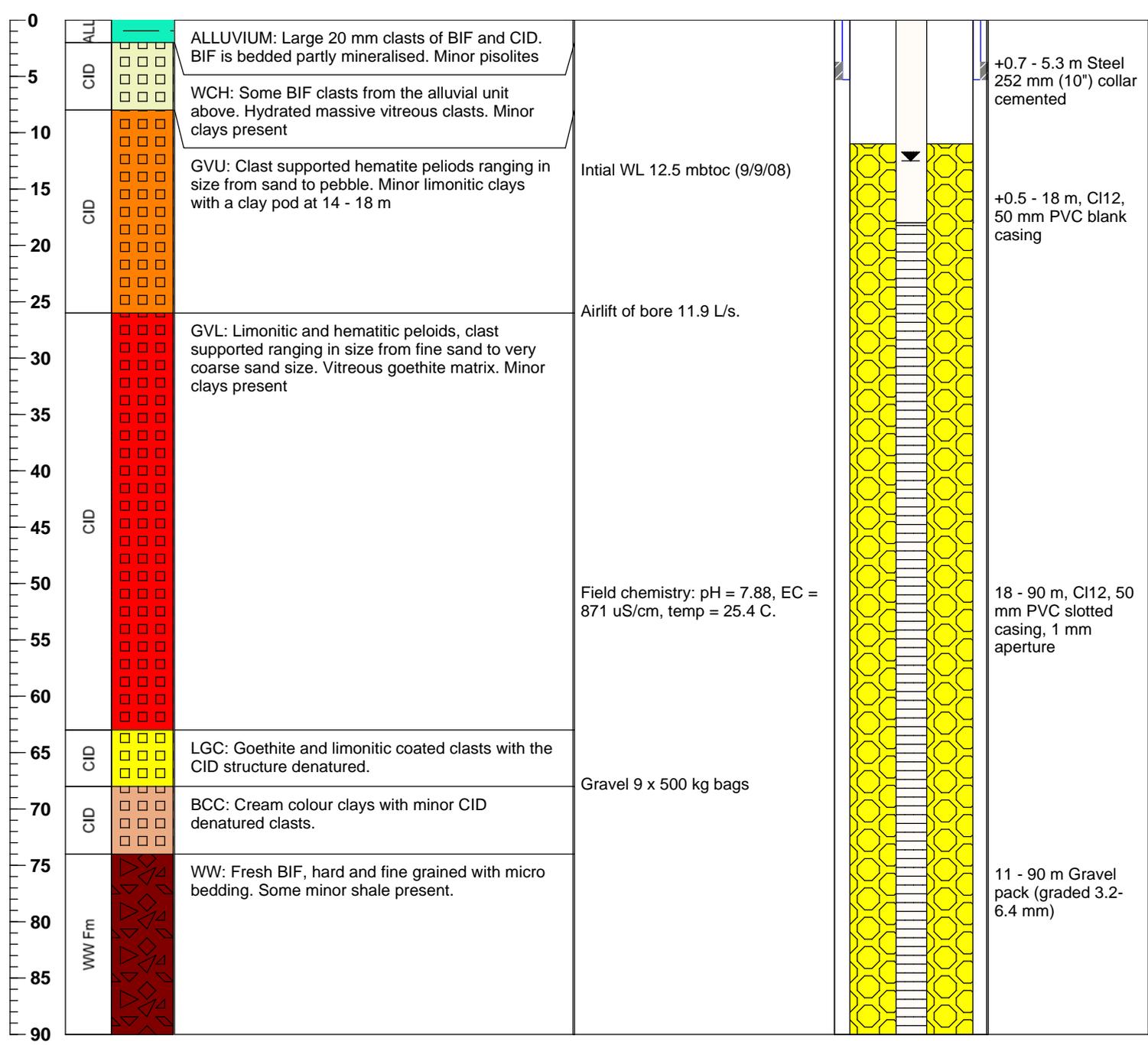
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Oxbow	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 719936.45
<b>DATE COMMENCED:</b> 5-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478158.04
<b>DATE COMPLETED:</b> 7-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 534.15 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-90m.

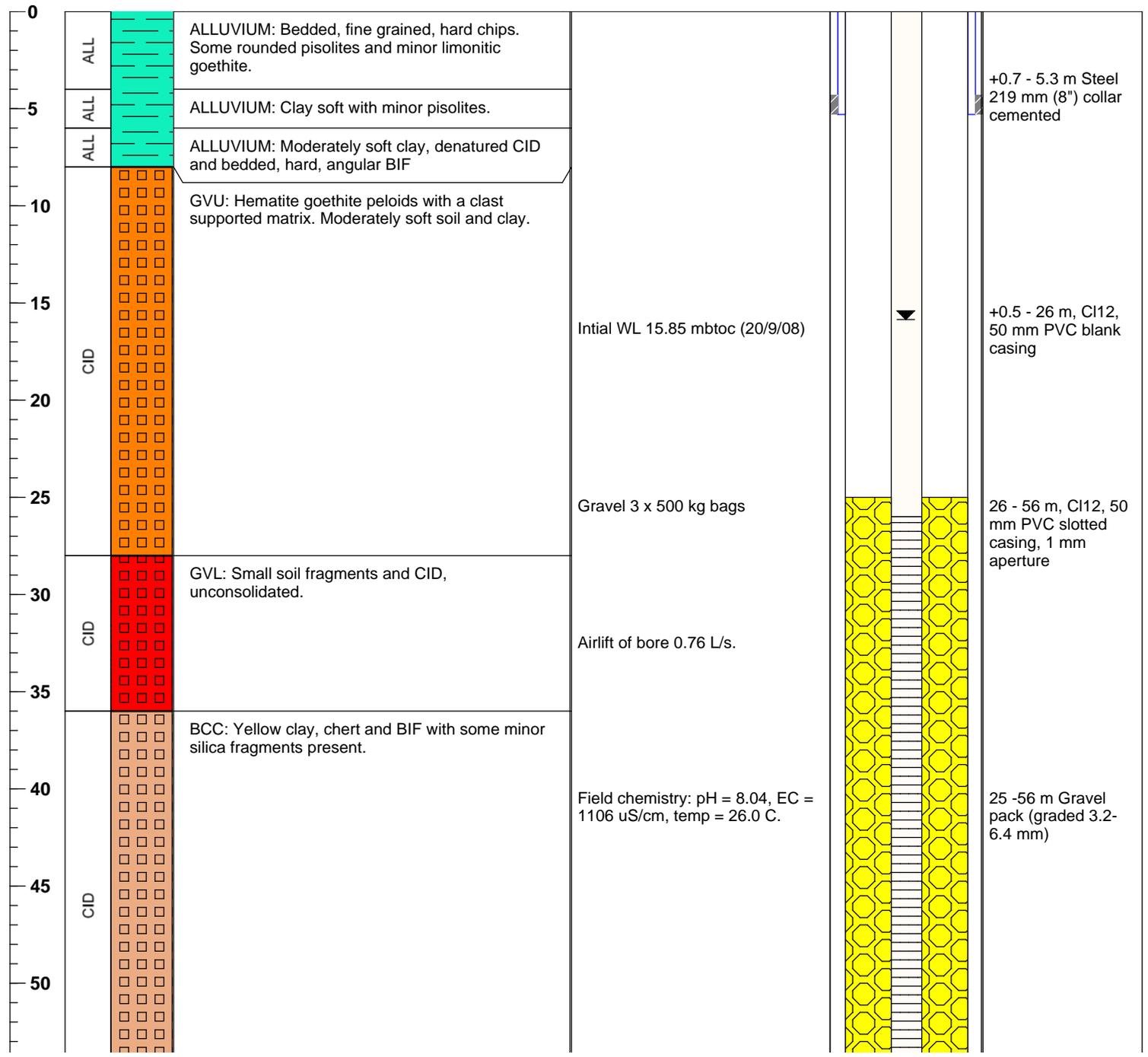
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Oxbow	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 720681.42
<b>DATE COMMENCED:</b> 17-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7477776.62
<b>DATE COMPLETED:</b> 19-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 538.33 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-56m.

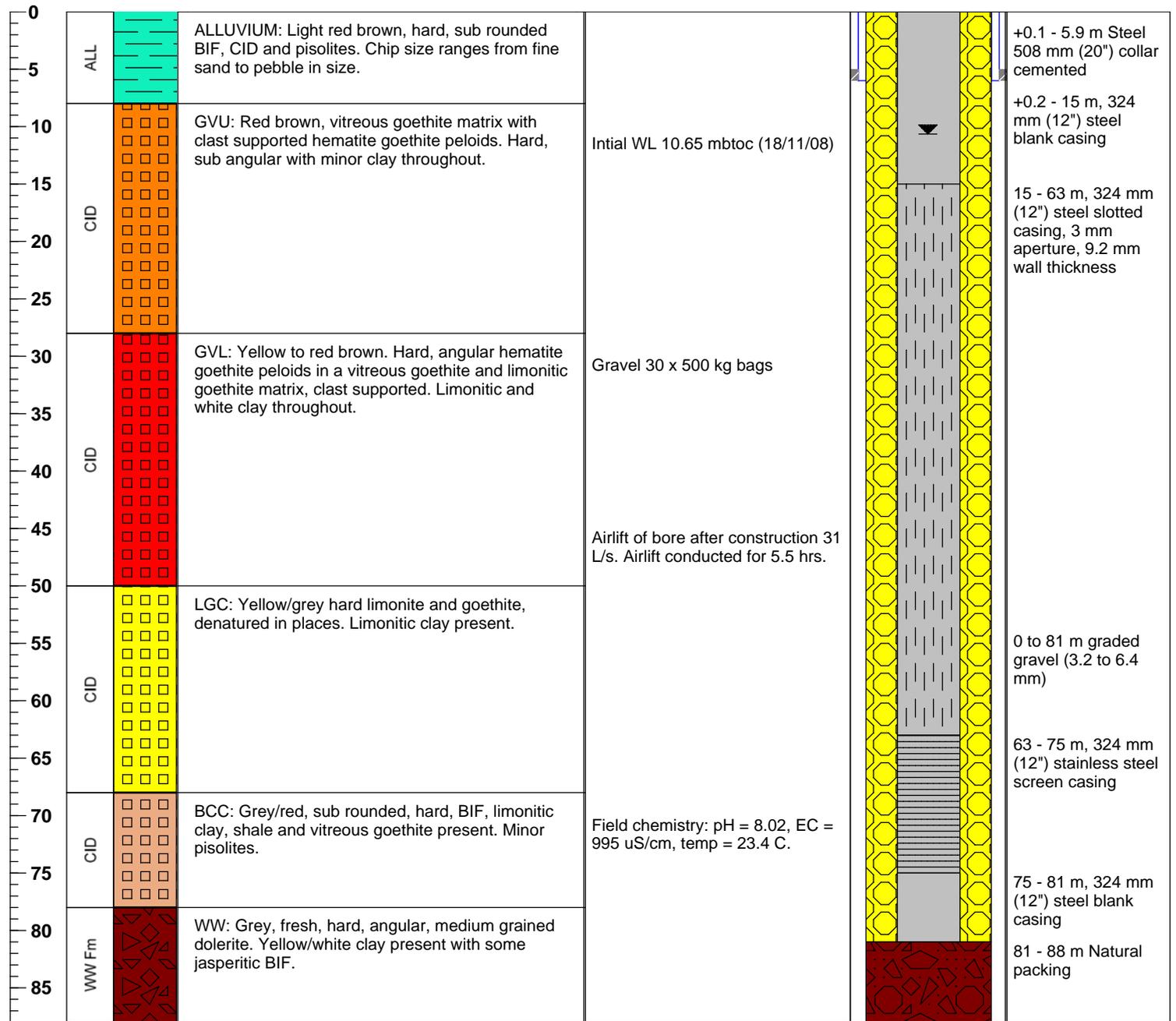
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Lee Martin	<b>EASTING:</b> 731867.37
<b>DATE COMMENCED:</b> 23-Nov-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478031.14
<b>DATE COMPLETED:</b> 30-Nov-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 499.46 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-6 m, 17.5 inch air/hammer 6-88 m.

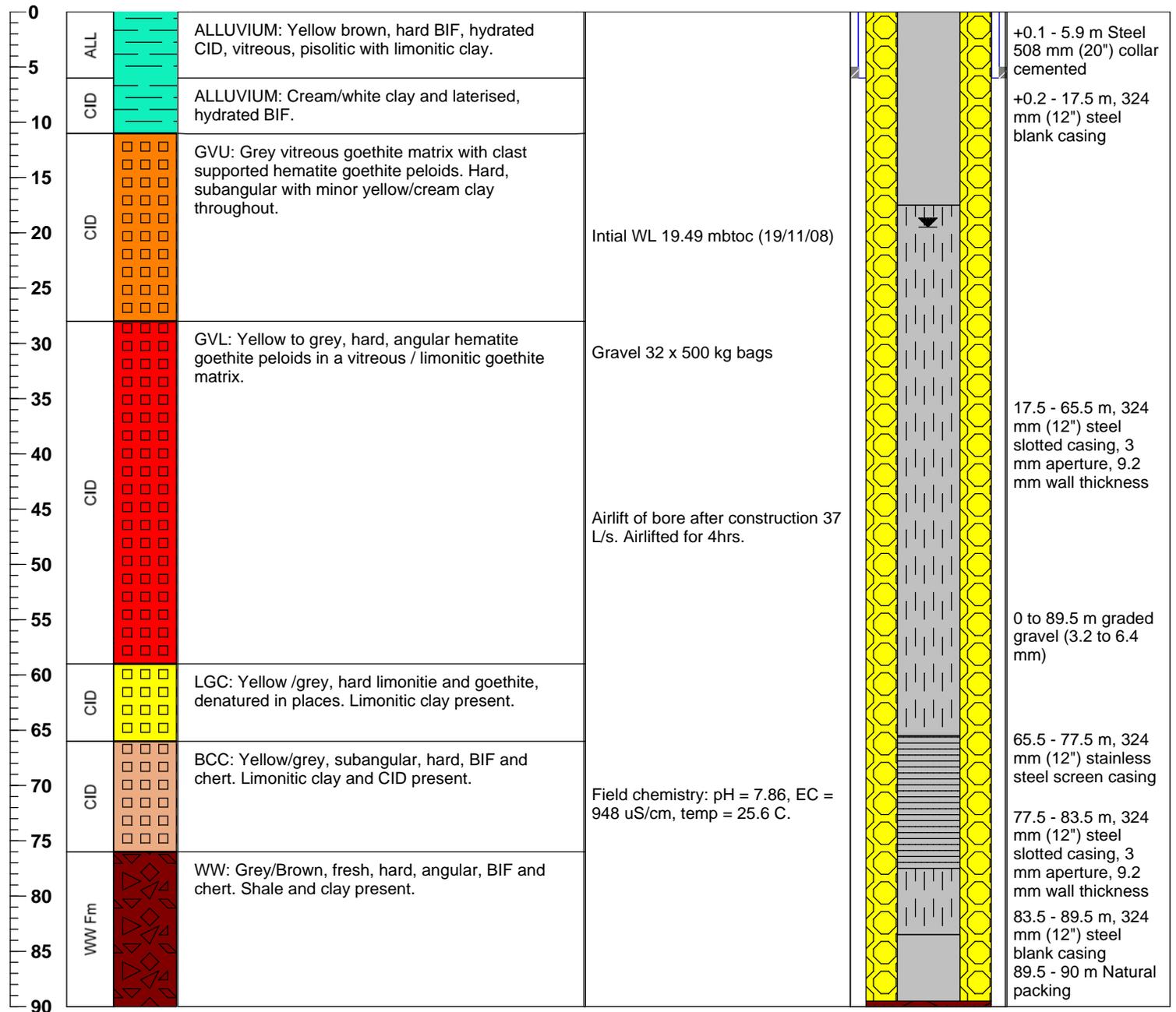
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Lee Martin/Shawn McGinley	<b>EASTING:</b> 722354.82
<b>DATE COMMENCED:</b> 5-Nov-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478542.66
<b>DATE COMPLETED:</b> 14-Nov-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 539.14 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-6 m, 17.5 inch air/hammer 6-90 m.

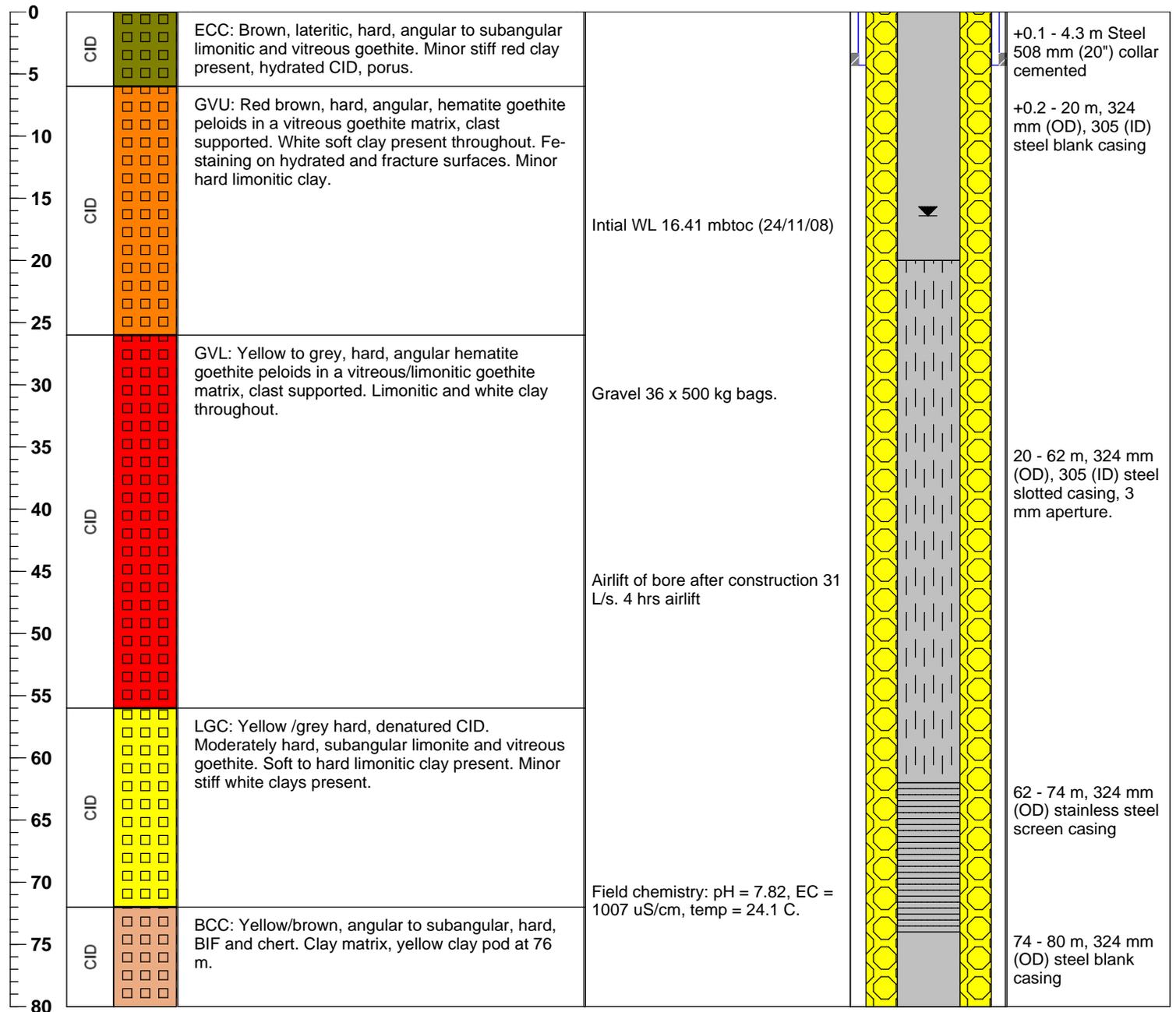
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Lee Martin/Shawn McGinley	<b>EASTING:</b> 723985.37
<b>DATE COMMENCED:</b> 14-Nov-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478756.92
<b>DATE COMPLETED:</b> 23-Nov-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 533.4 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-4.3 m, 17.5 inch air/hammer 4.3-80 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South West  
**DATE COMMENCED:** 19-Mar-09  
**DATE COMPLETED:** 23-Mar-09

### DRILLING DETAILS

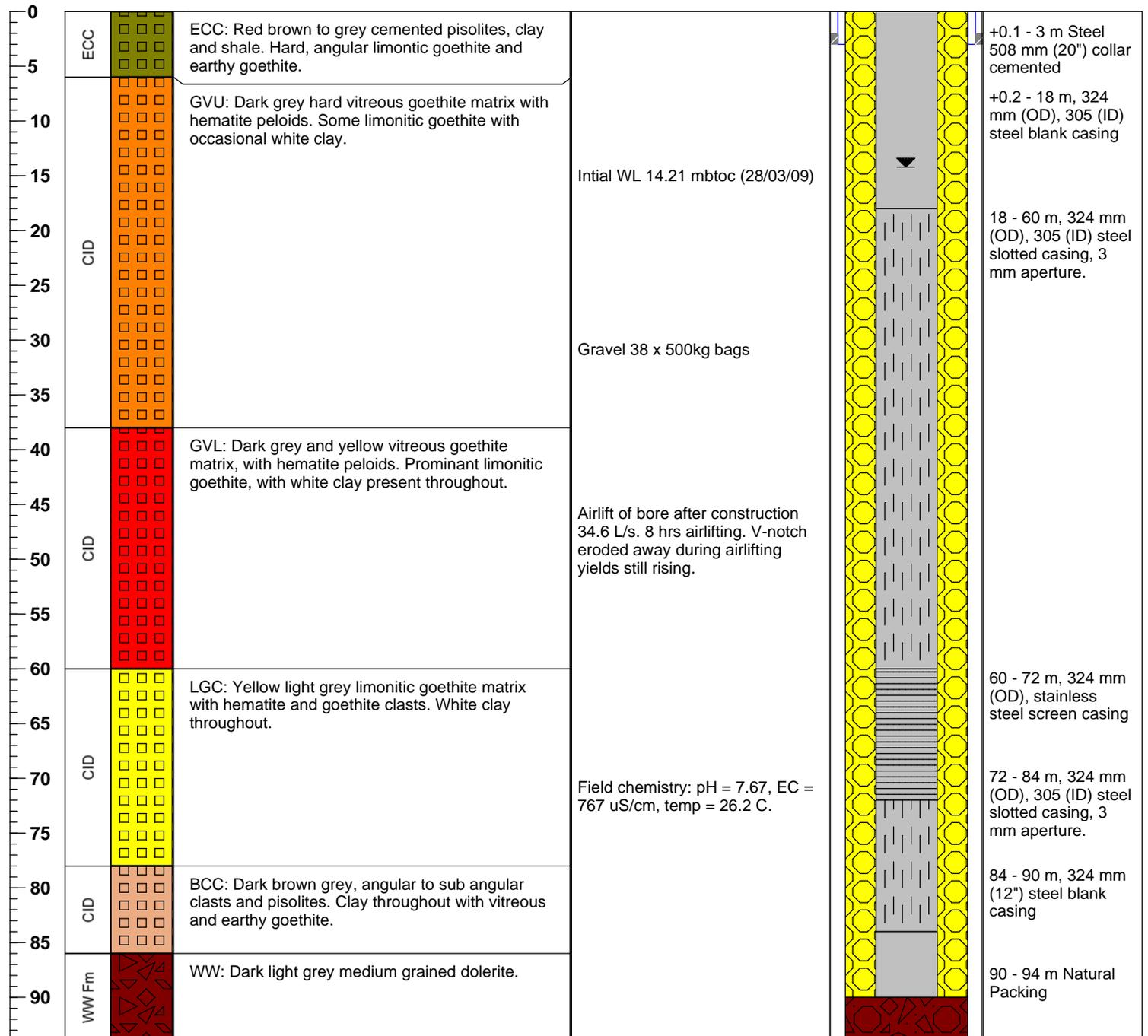
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Michael Larkman  
**DRILLING METHOD:** Air/Hammer  
**HYDROGEOLOGIST(s):** Niall Inverarity

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 723778.16  
**NORTHING:** 7478714.39  
**ELEVATION:** 531.26 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-3 m, 17.5 inch air/hammer 3-94m.

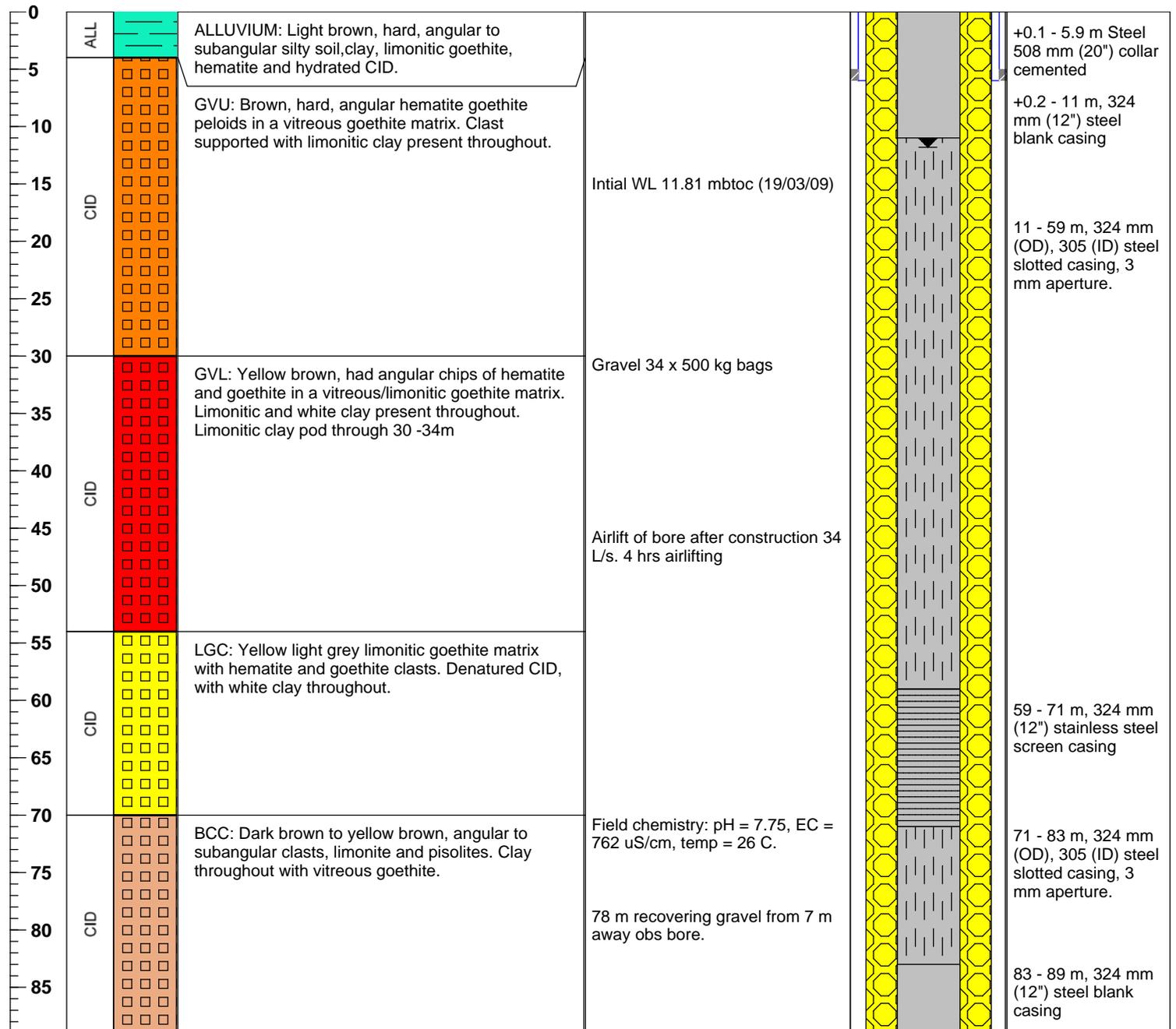
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Richard Attard	<b>EASTING:</b> 723915.21
<b>DATE COMMENCED:</b> 8-Mar-09	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478816.72
<b>DATE COMPLETED:</b> 16-Mar-09	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 528.94 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-6 m, 17.5 inch air/hammer 6-89m.

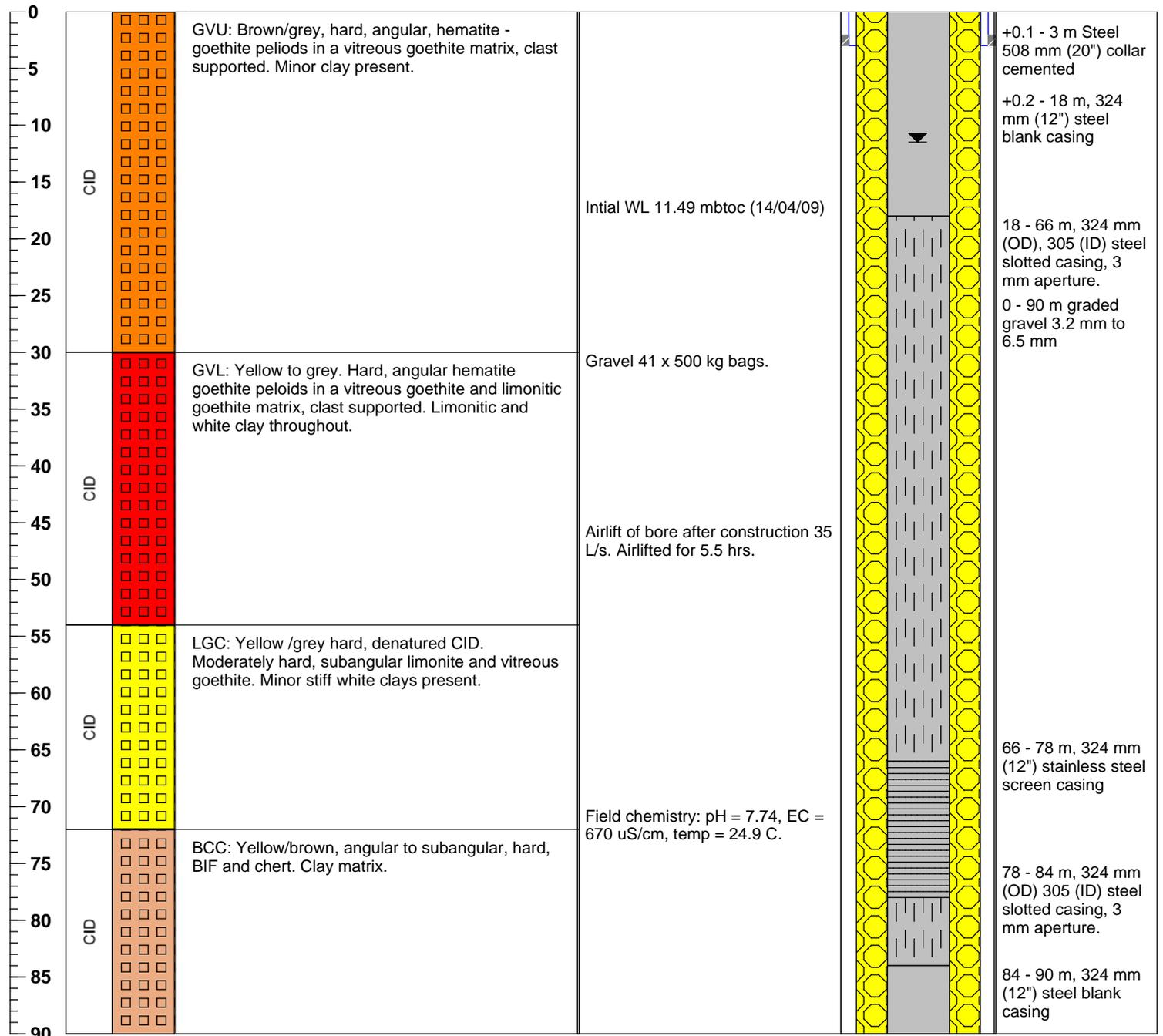
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Richad Attard	<b>EASTING:</b> 724585.99
<b>DATE COMMENCED:</b> 5-April-09	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7479685.17
<b>DATE COMPLETED:</b> 13-April-09	<b>HYDROGEOLOGIST(s):</b> Niall Inverarity	<b>ELEVATION:</b> 524.16 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-3 m, 17.5 inch air/hammer 6-90 m.

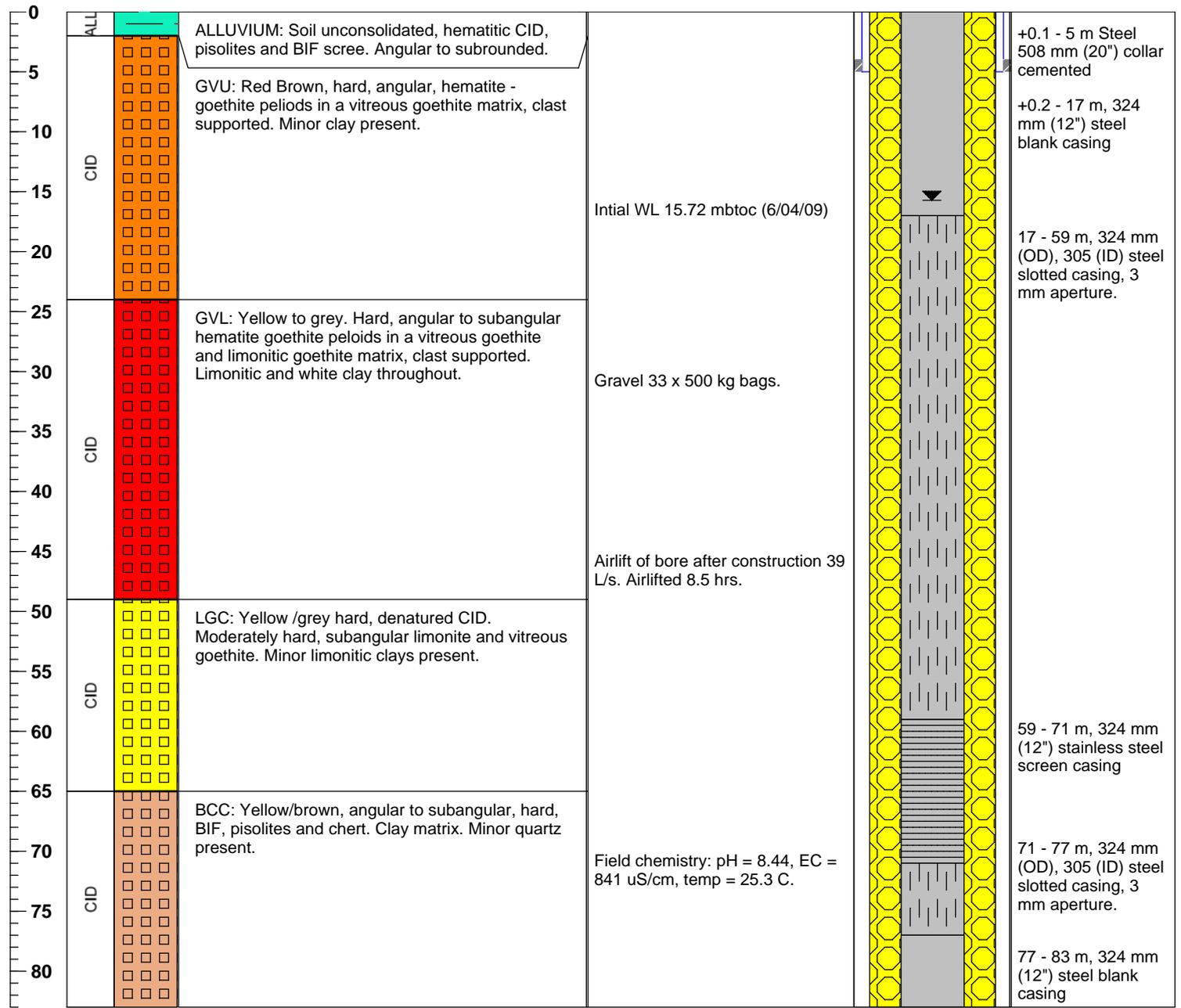
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Michael Larkman	<b>EASTING:</b> 724816.54
<b>DATE COMMENCED:</b> 29-Mar-09	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7480321.78
<b>DATE COMPLETED:</b> 2-April-09	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 524.06 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-5 m, 17.5 inch air/hammer 5-83 m.

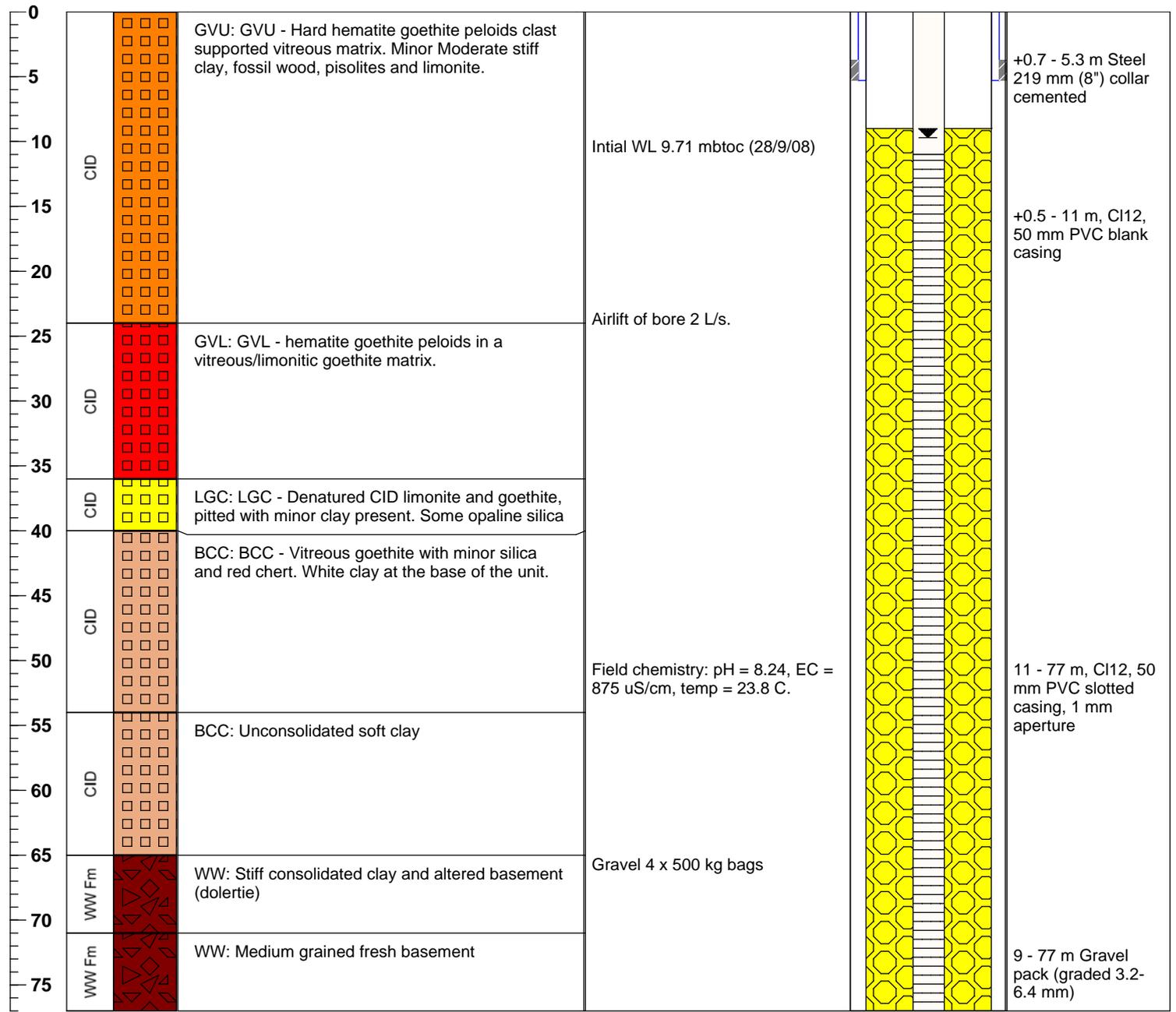
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Shaun McGinley	<b>EASTING:</b> 721690.89
<b>DATE COMMENCED:</b> 21-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478156.28
<b>DATE COMPLETED:</b> 22-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 531.82 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-77 m.

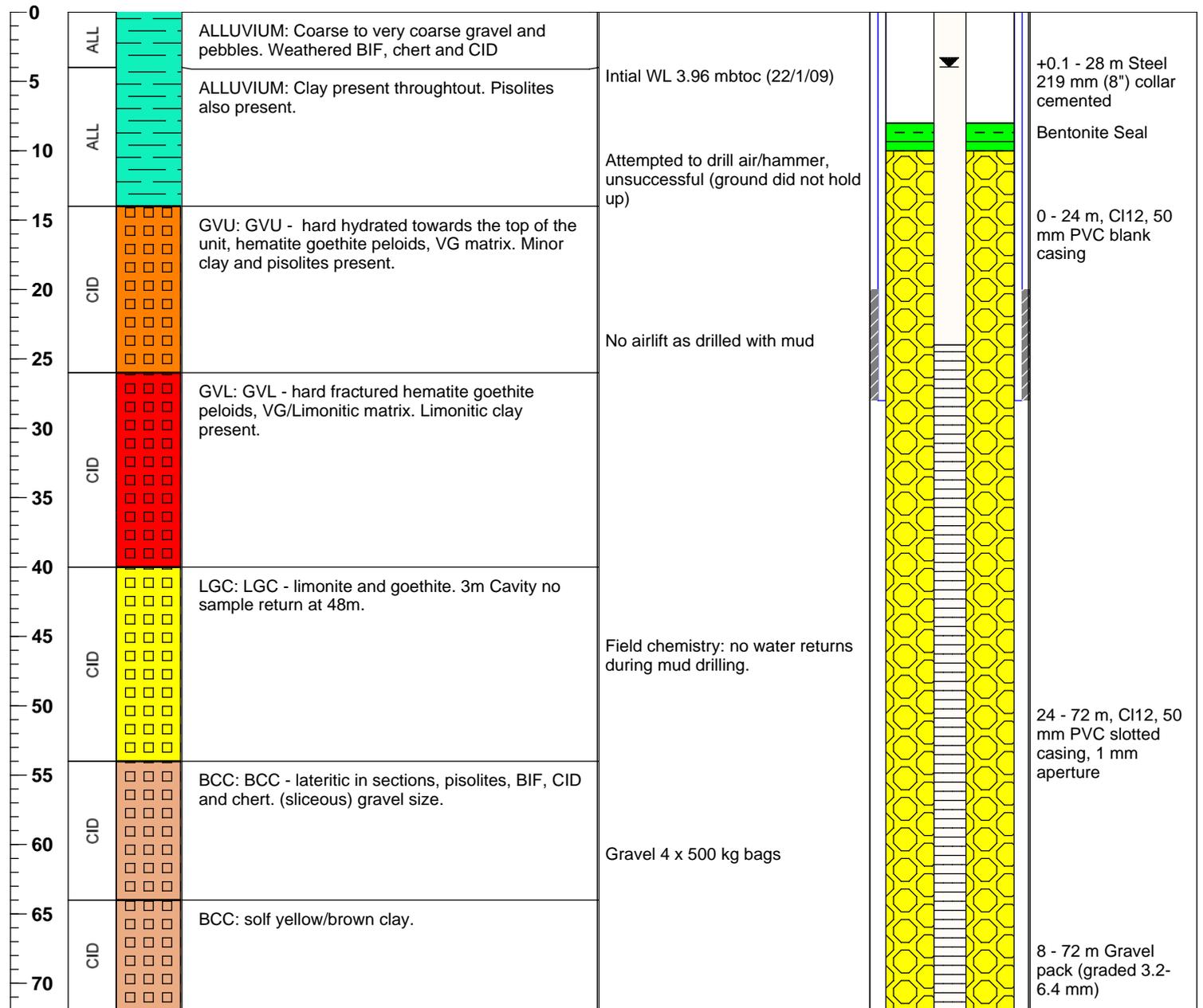
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Shaun McGinley	<b>EASTING:</b> 721667.81
<b>DATE COMMENCED:</b> 8-Dec-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478289.94
<b>DATE COMPLETED:</b> 17-Dec-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 525.96 mRL (TOC)

Well Notes: Hole diameter 12 inch mud/rotary 0-28m, 6 inch mud/rotary 28-72 m.

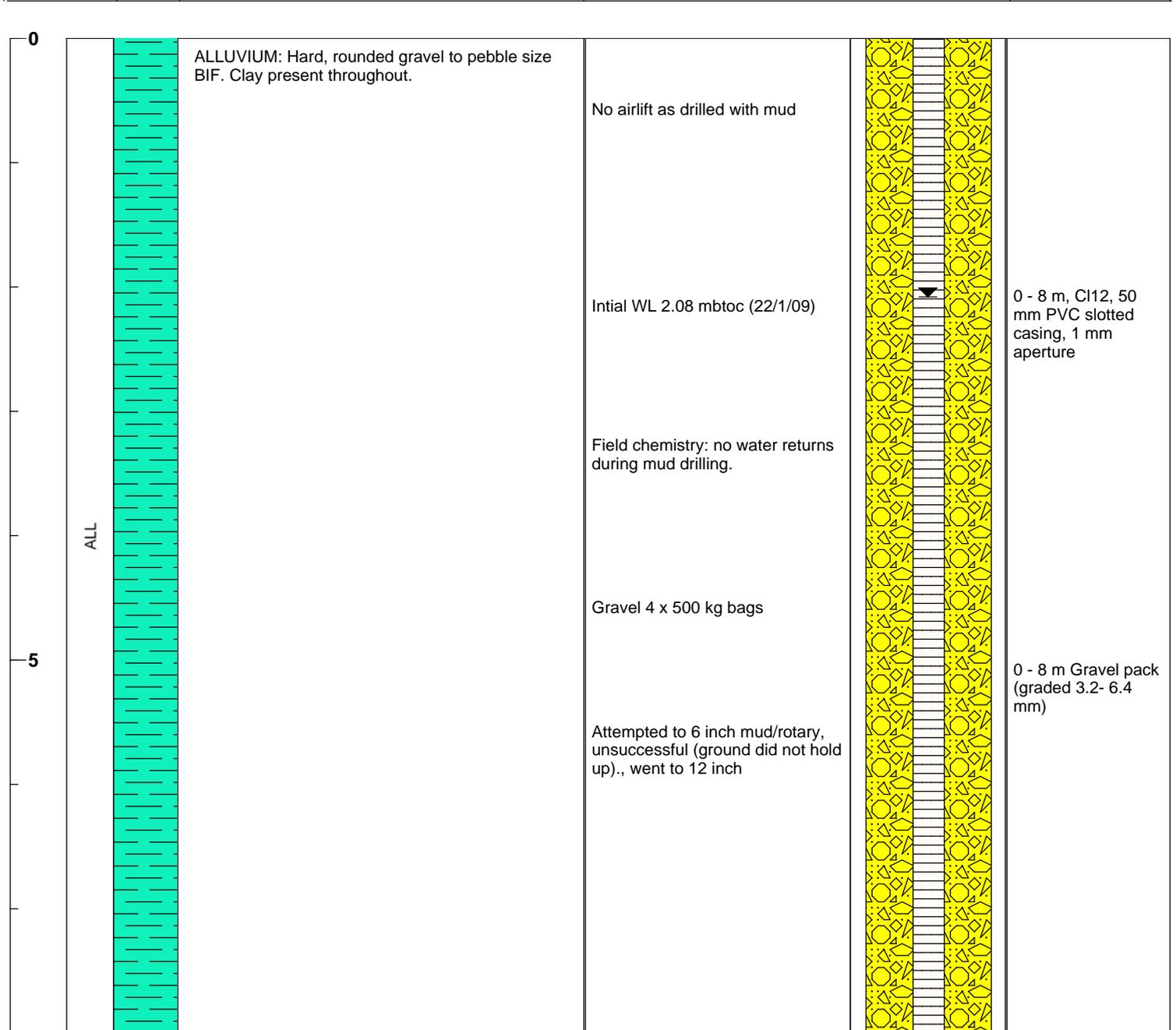
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Shaun McGinley	<b>EASTING:</b> 721669.93
<b>DATE COMMENCED:</b> 17-Dec-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478287.4
<b>DATE COMPLETED:</b> 17-Dec-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 526.02 mRL (TOC)

Well Notes: Hole diameter 12 inch mud/rotary 0-8m.

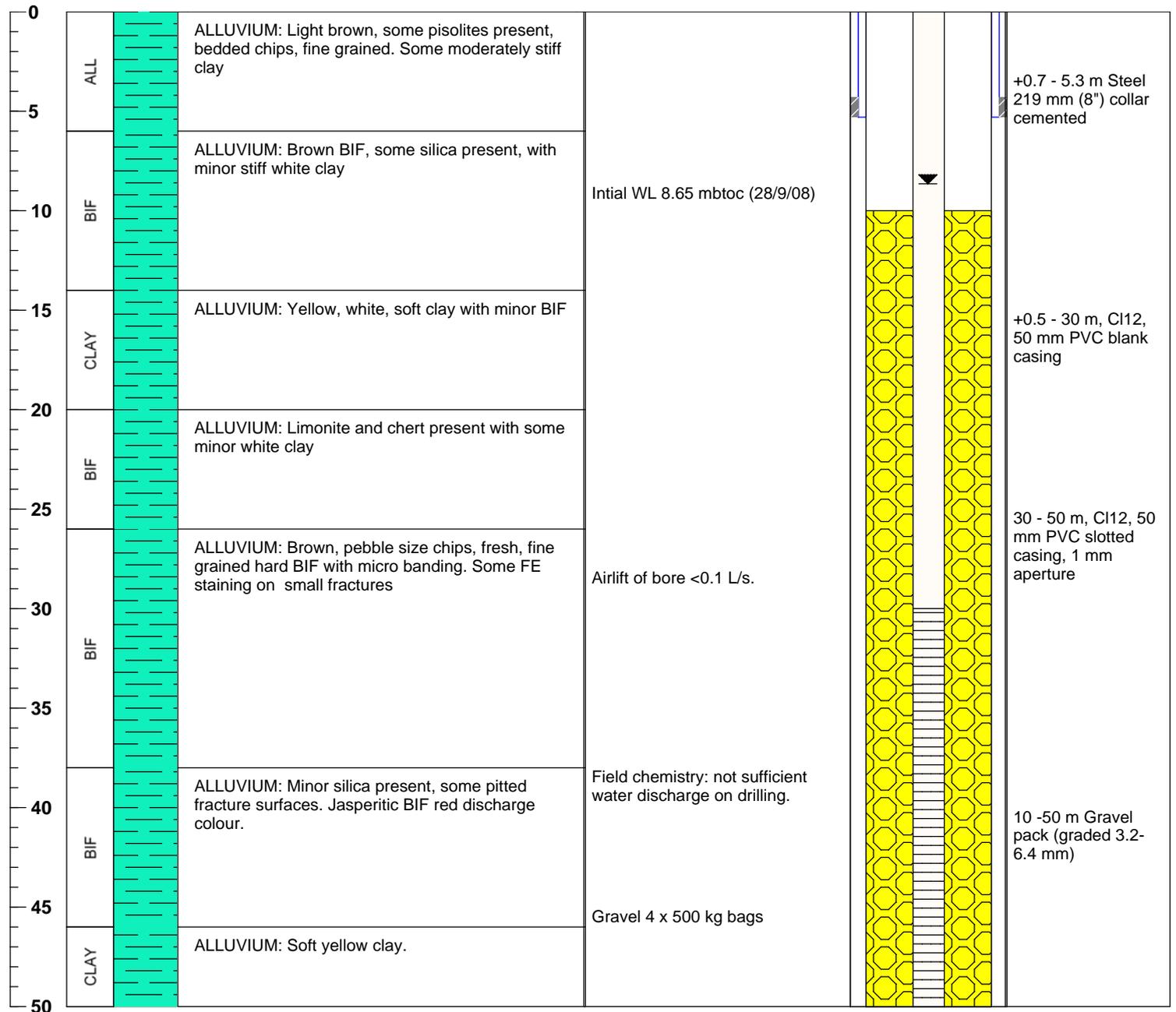
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 722687.95
<b>DATE COMMENCED:</b> 14-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478746.36
<b>DATE COMPLETED:</b> 17-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 528.29 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-50m.

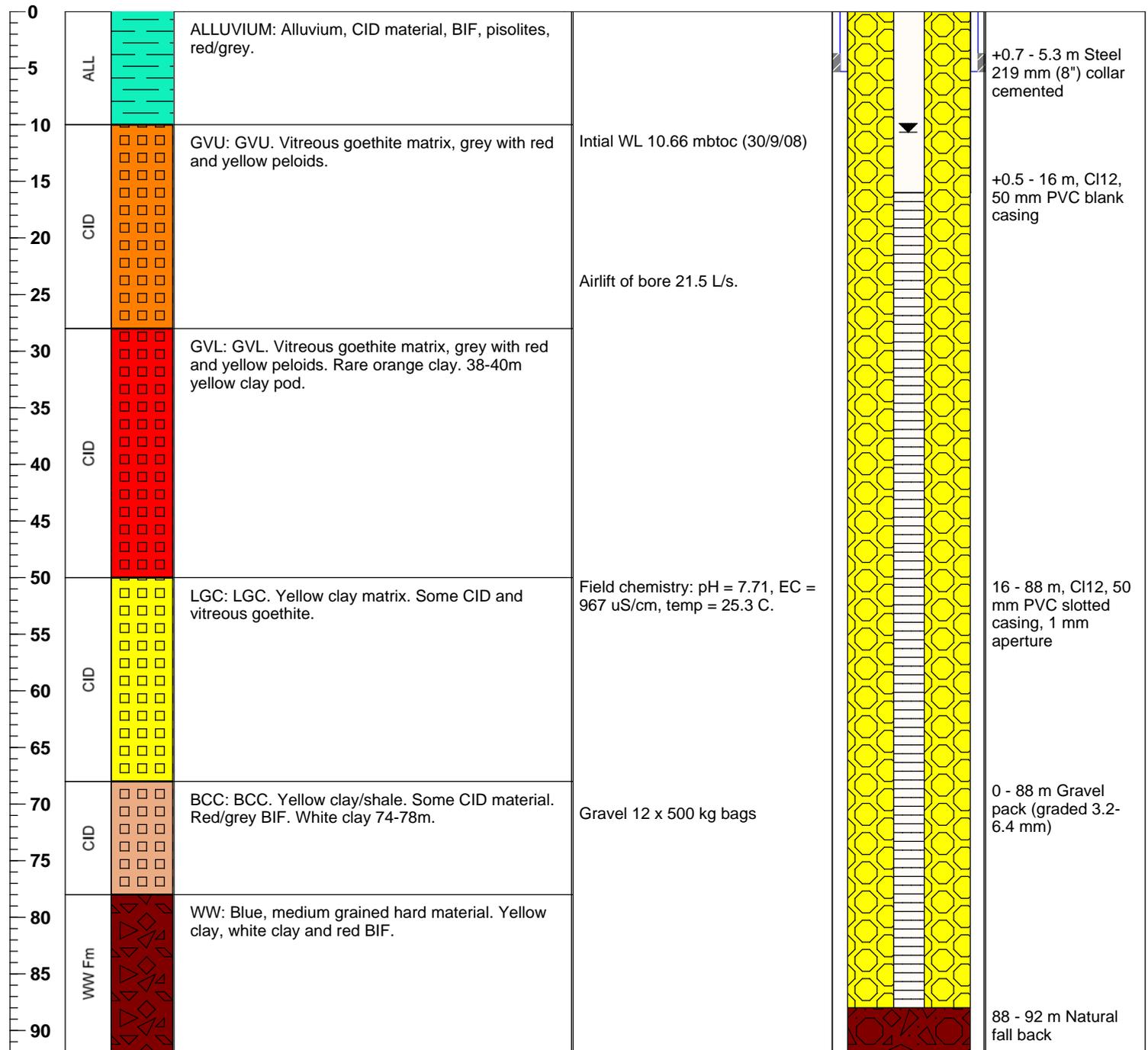
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Lee Martin	<b>EASTING:</b> 722494.86
<b>DATE COMMENCED:</b> 25-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478671
<b>DATE COMPLETED:</b> 26-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 530.46 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-92 m.

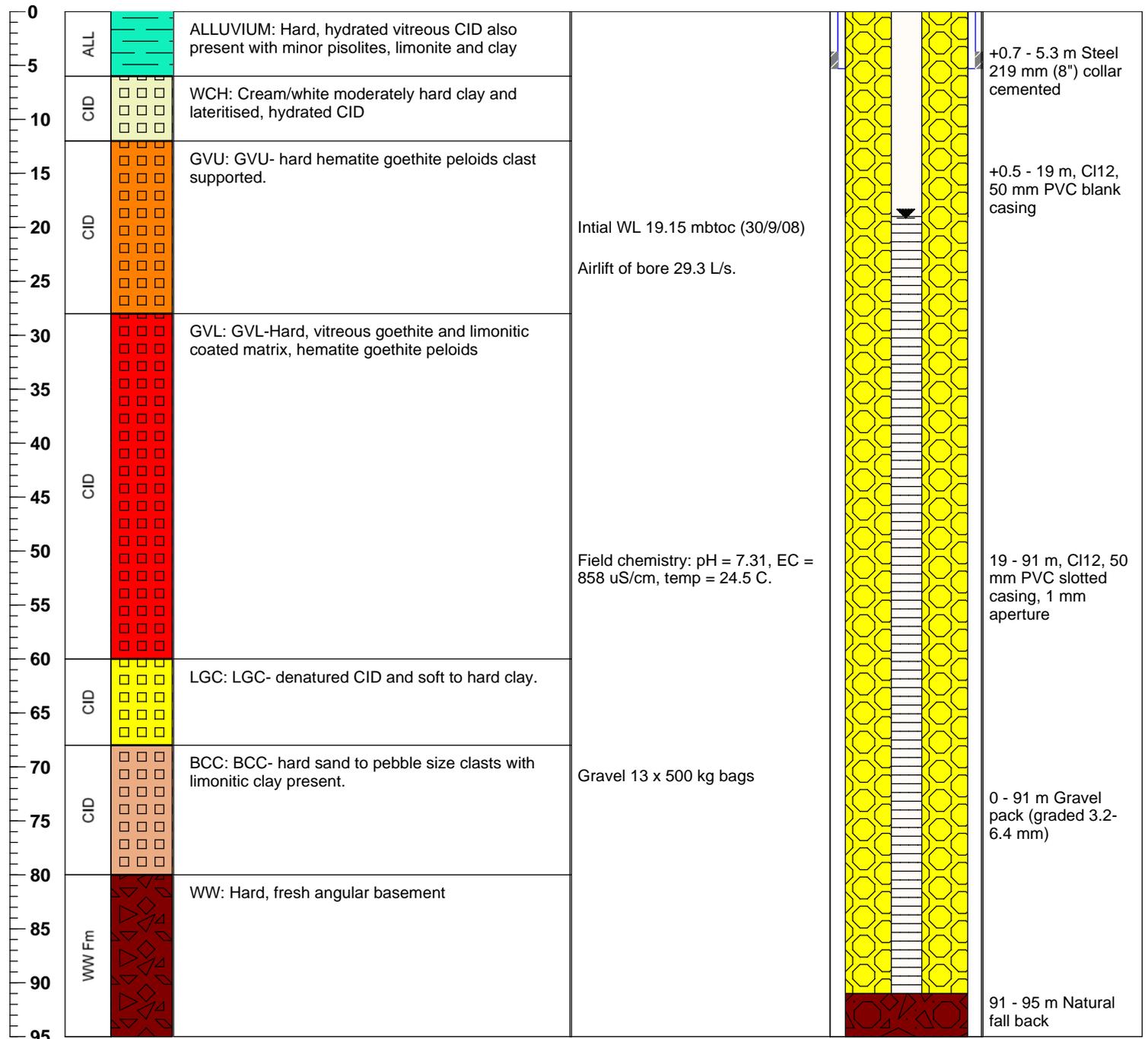
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Lee Martin	<b>EASTING:</b> 722368.09
<b>DATE COMMENCED:</b> 23-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478550.36
<b>DATE COMPLETED:</b> 25-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 538.77 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-95 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South West  
**DATE COMMENCED:** 22-Sept-08  
**DATE COMPLETED:** 23-Sept-08

### DRILLING DETAILS

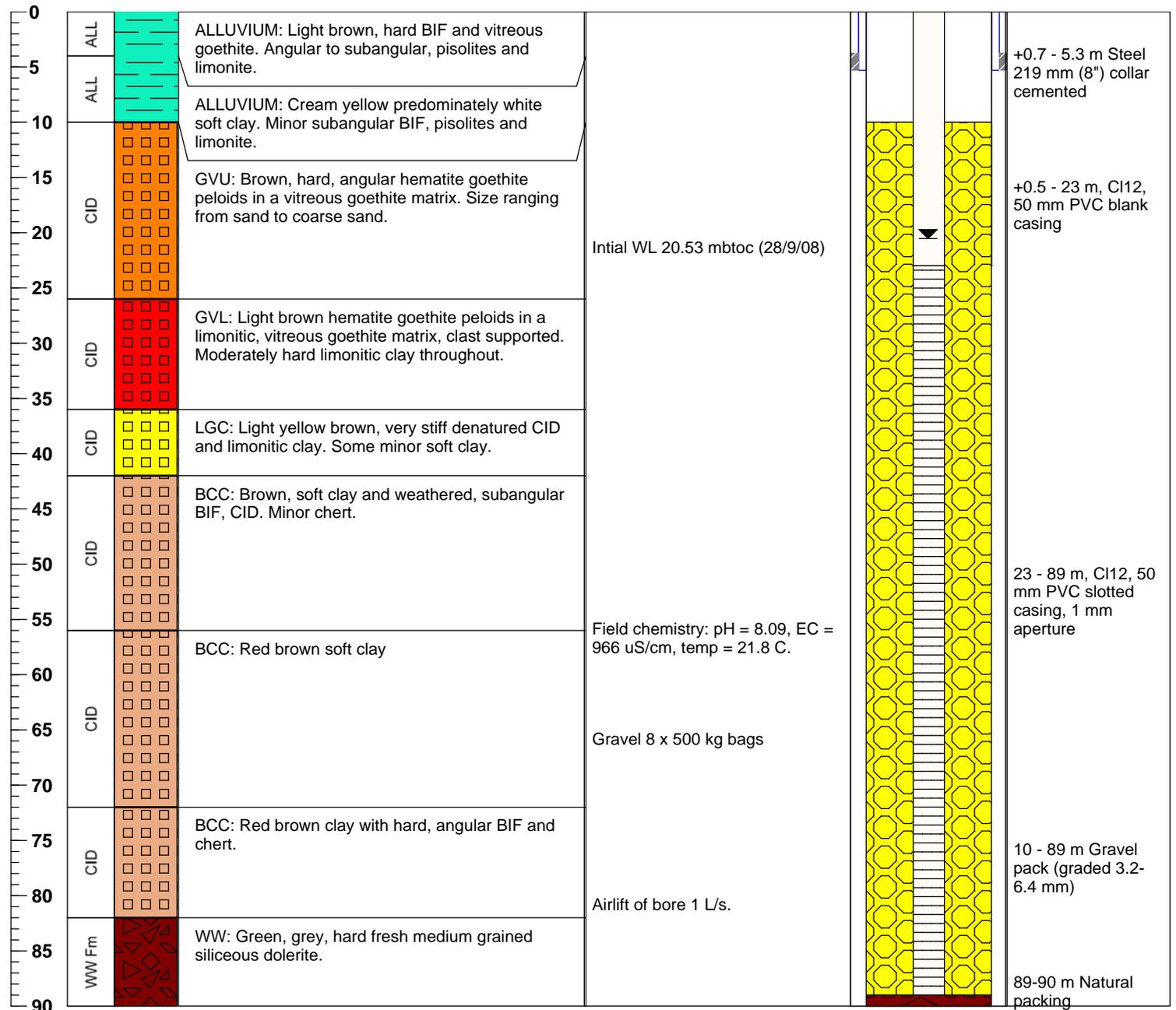
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Andrew McInerney  
**DRILLING METHOD:** Air/Hammer  
**HYDROGEOLOGIST(s):** Glenn Kirkpatrick

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 722289.97  
**NORTHING:** 7478479.96  
**ELEVATION:** 540.19 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-90 m.

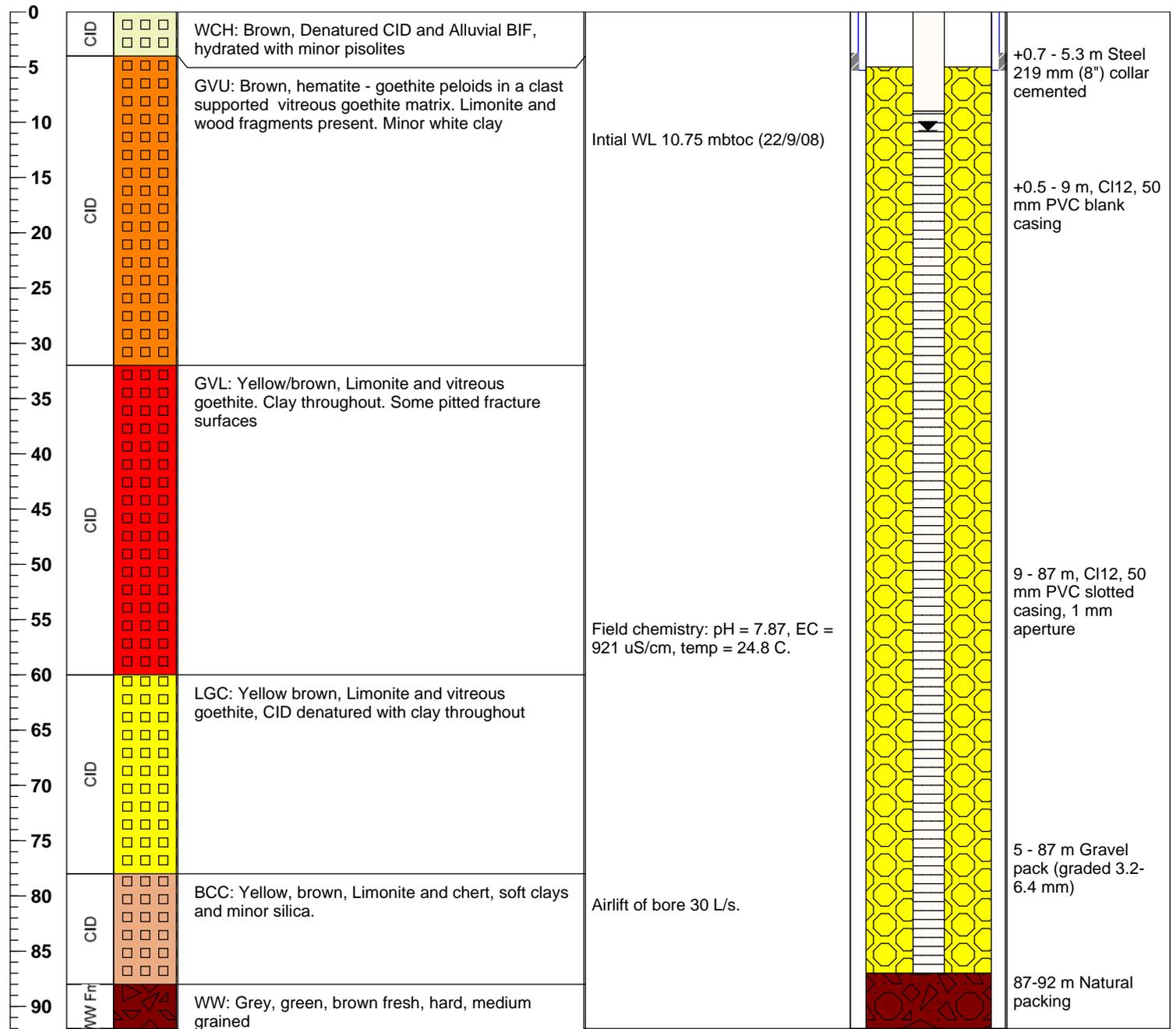
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 722517.46
<b>DATE COMMENCED:</b> 20-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478576.25
<b>DATE COMPLETED:</b> 22-Sept-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 530.09 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-92 m.

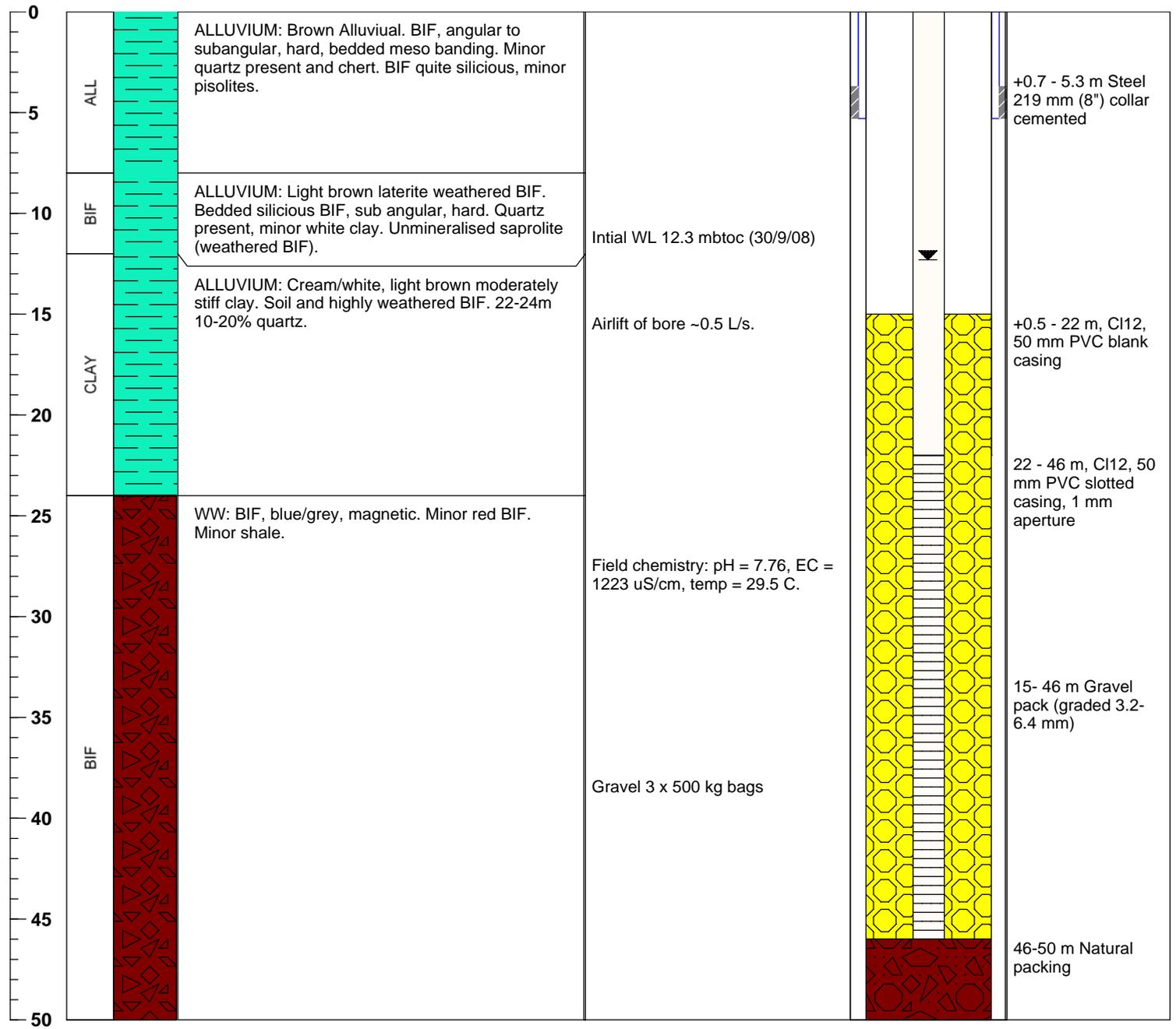
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 722533.54
<b>DATE COMMENCED:</b> 23-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478166.54
<b>DATE COMPLETED:</b> 25-Sept-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 532.86 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 7&7/8 inch hammer 5.3-50 m.

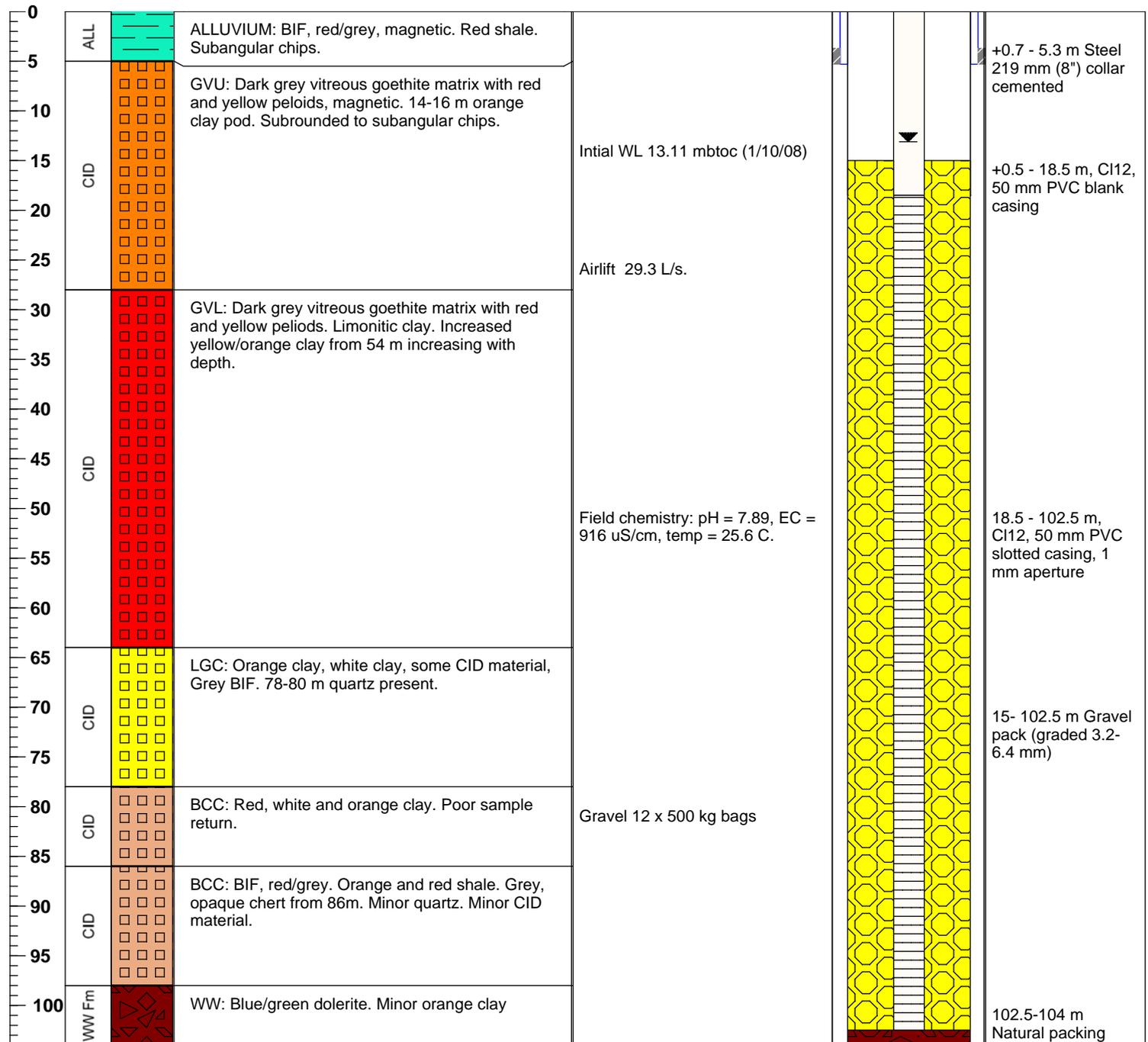
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 722852.94
<b>DATE COMMENCED:</b> 25-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478329.91
<b>DATE COMPLETED:</b> 28-Sept-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 532.14 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 7&7/8 inch hammer 5.3-104 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South West  
**DATE COMMENCED:** 3-April-09  
**DATE COMPLETED:** 4-April-09

### DRILLING DETAILS

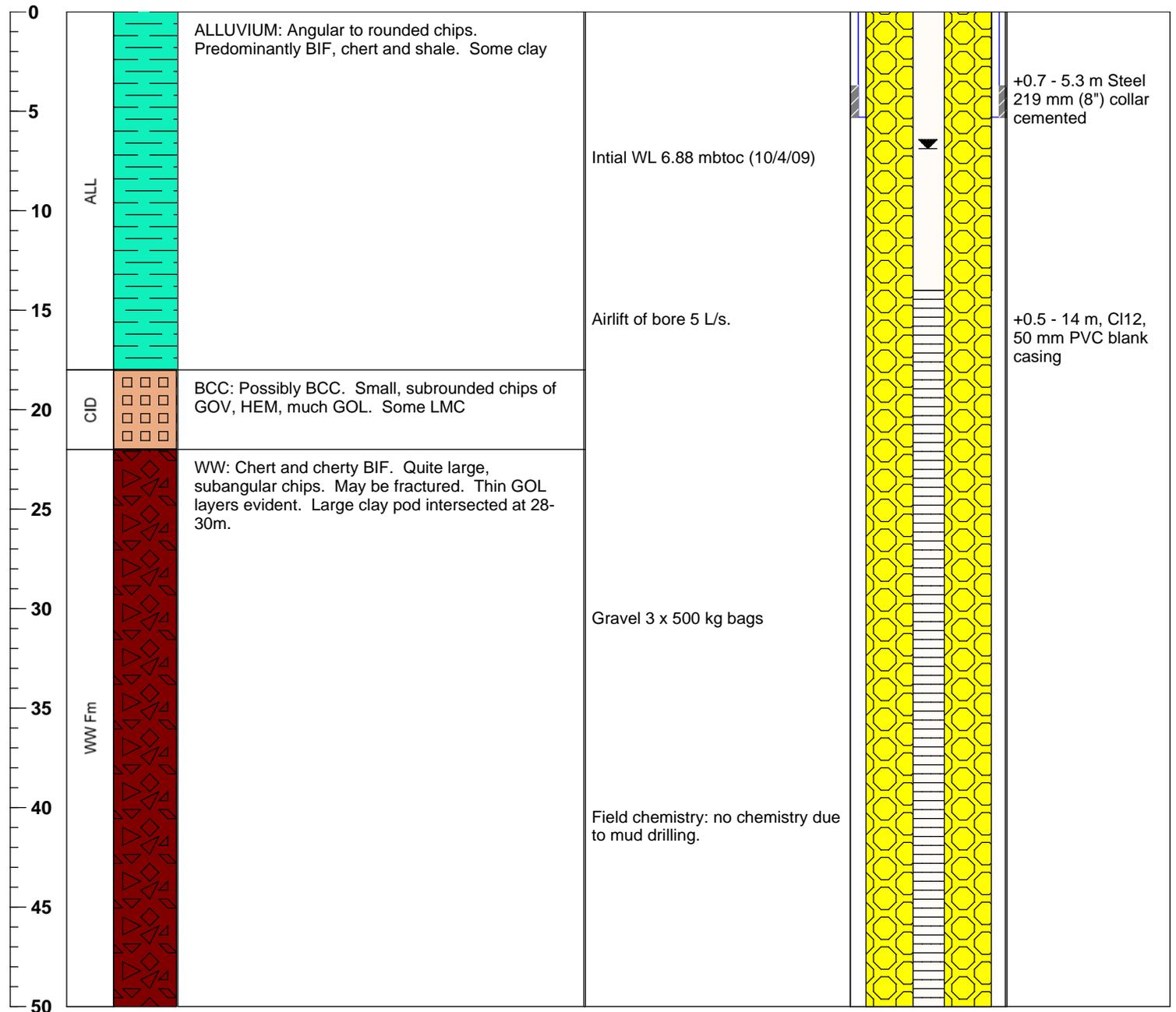
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Richard Attard  
**DRILLING METHOD:** Mud/Rotary  
**HYDROGEOLOGIST(s):** Niall Inverarity

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 723317.05  
**NORTHING:** 7478578.87  
**ELEVATION:** 526.46 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 6 inch mud/rotary 5.3-50 m.

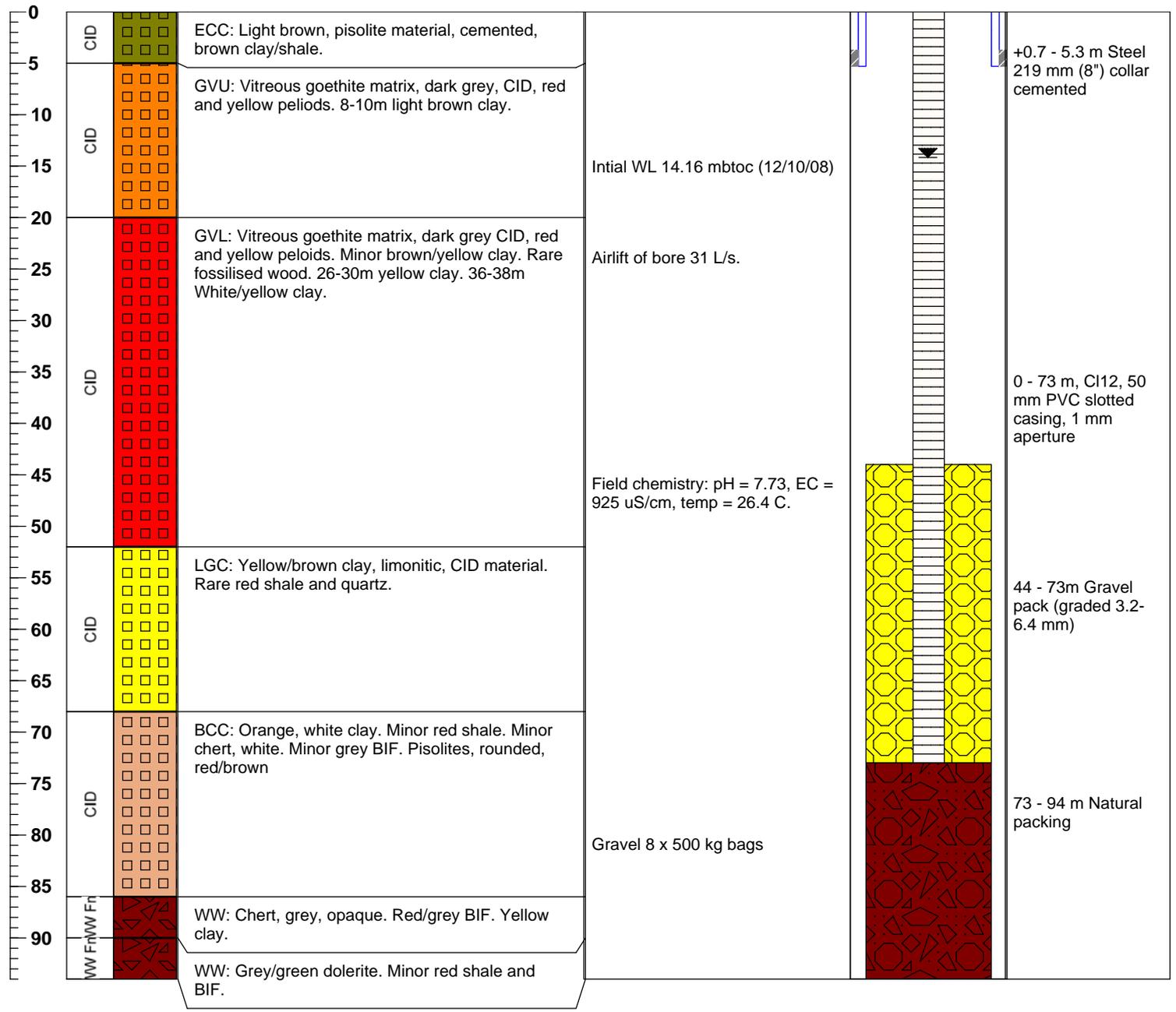
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Ross Pendelbury	<b>EASTING:</b> 723784.41
<b>DATE COMMENCED:</b> 9-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478725.17
<b>DATE COMPLETED:</b> 11-Oct-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 531.49 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 7&7/8 inch hammer 5.3-94 m.

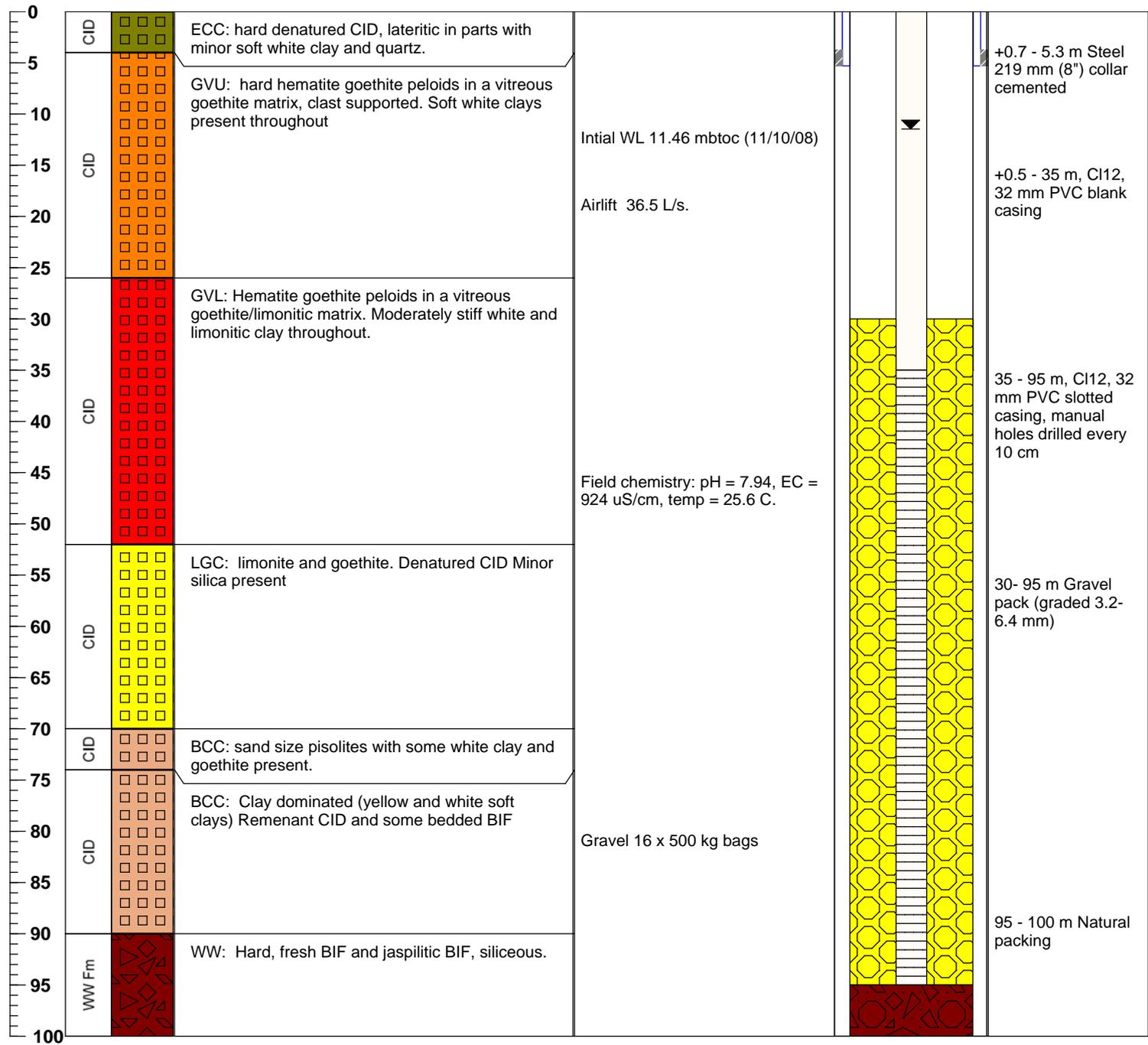
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Ross Pendelbury	<b>EASTING:</b> 723918.46
<b>DATE COMMENCED:</b> 6-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478823.63
<b>DATE COMPLETED:</b> 8-Oct-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 528.51 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 7&7/8 inch hammer 5.3-100 m.

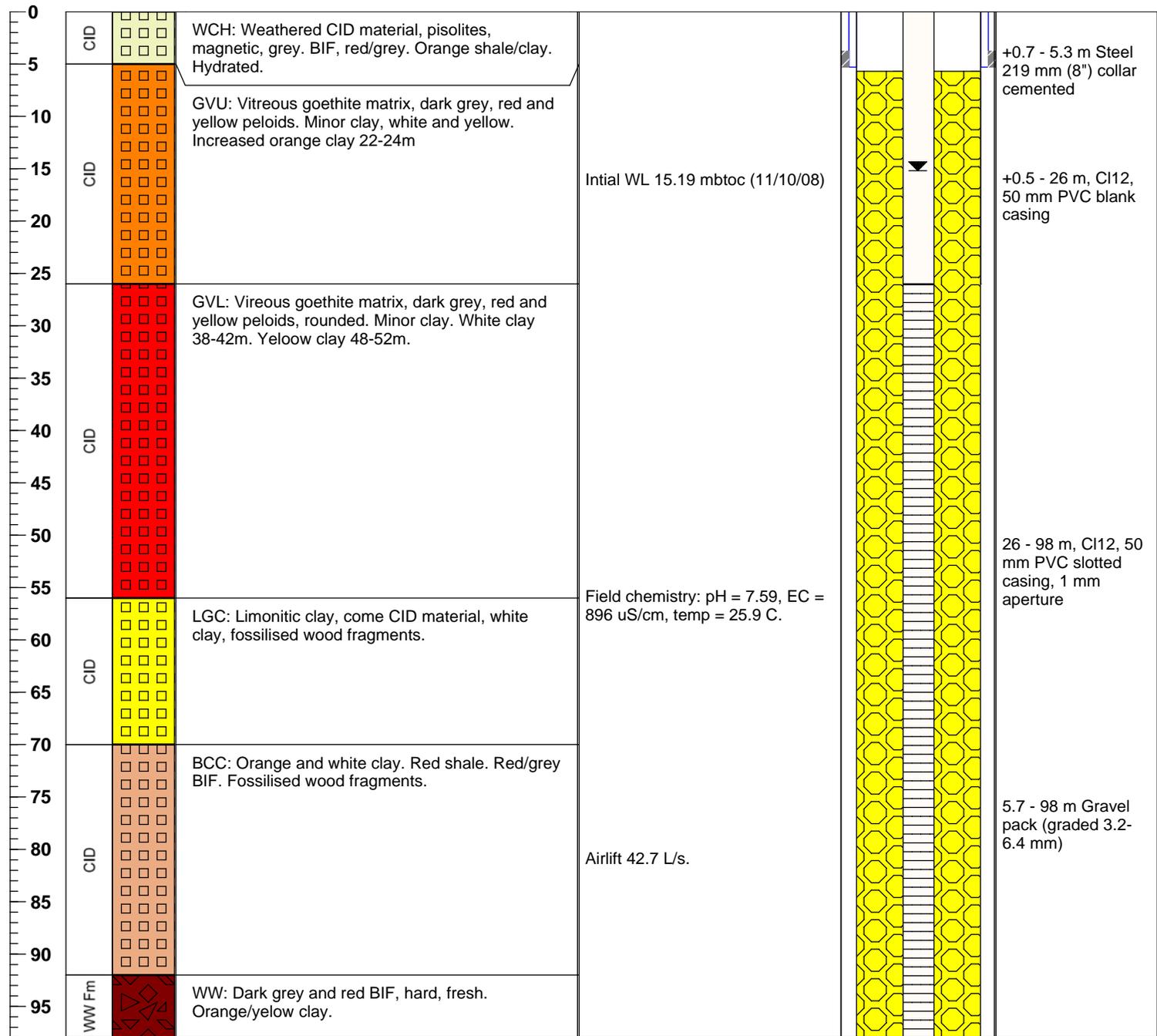
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Ross Pendelbury	<b>EASTING:</b> 724029.24
<b>DATE COMMENCED:</b> 30-Sept-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478873.93
<b>DATE COMPLETED:</b> 2-Oct-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 532.2 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-98 m.

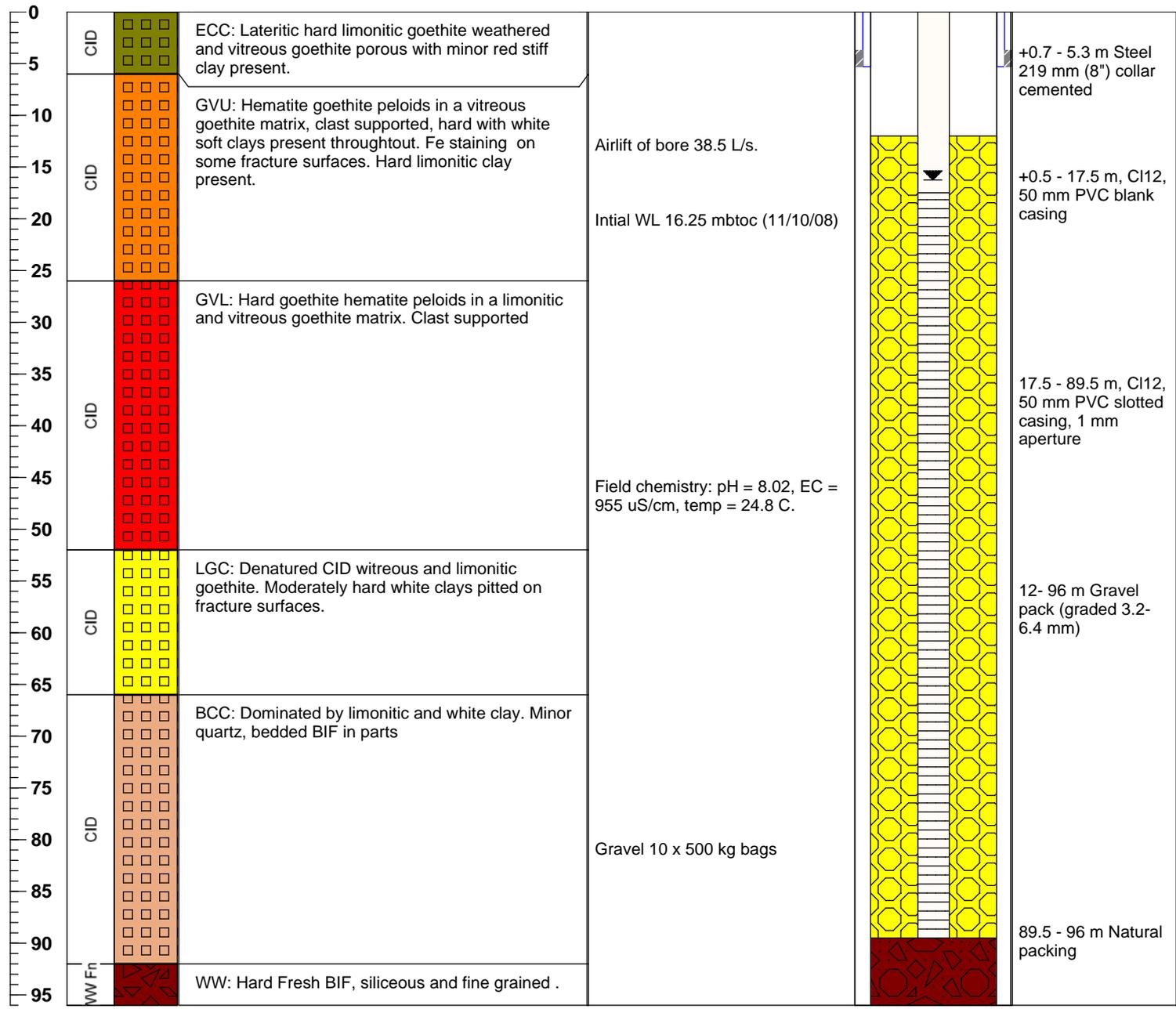
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Ross Pendelbury	<b>EASTING:</b> 723990.04
<b>DATE COMMENCED:</b> 5-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478765.97
<b>DATE COMPLETED:</b> 6-Oct-08	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 533.34 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 7&7/8 inch hammer 5.3-96 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South West  
**DATE COMMENCED:** 2-Oct-08  
**DATE COMPLETED:** 4-Oct-08

### DRILLING DETAILS

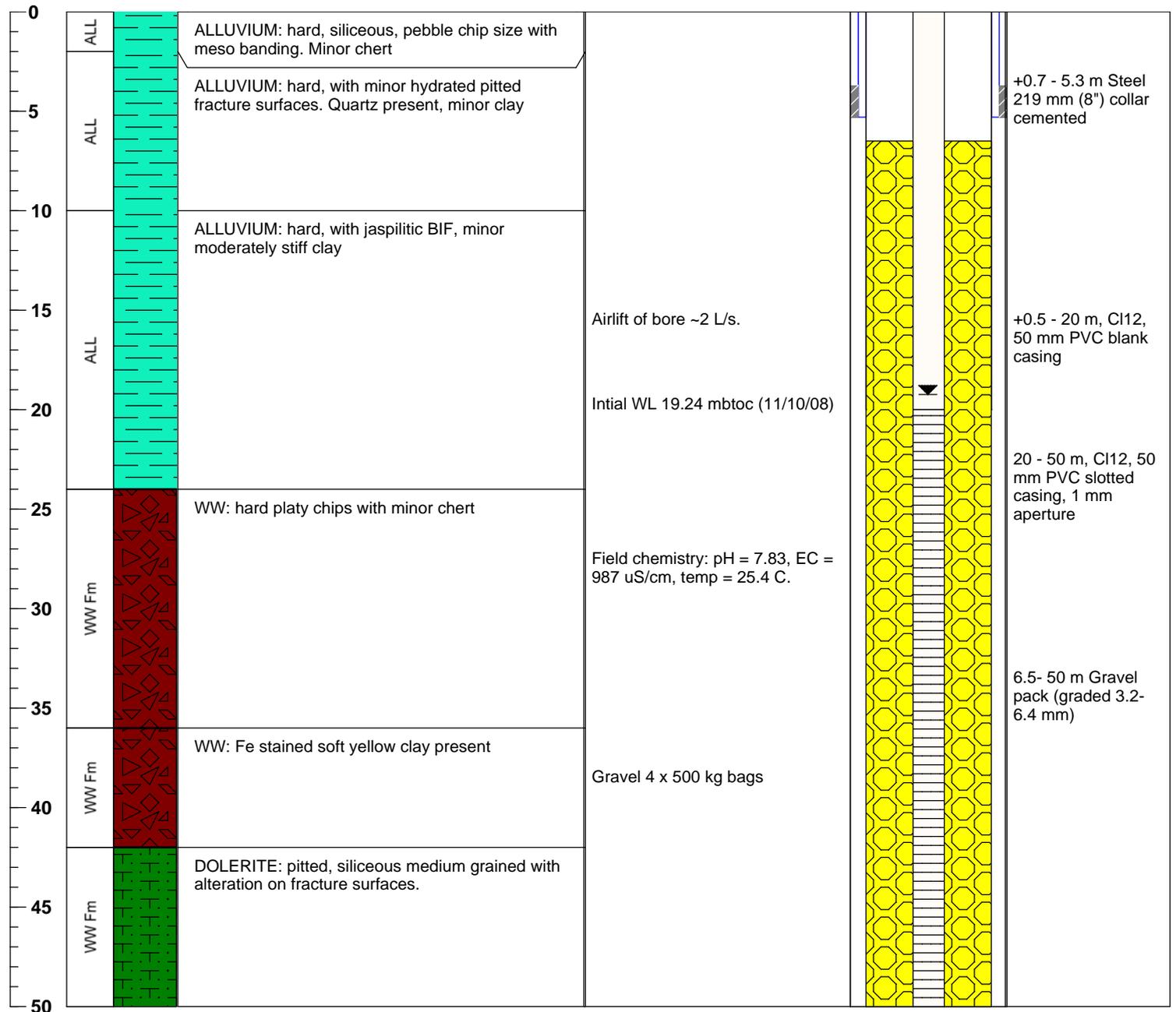
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Andrew McInerney  
**DRILLING METHOD:** Air/Hammer  
**HYDROGEOLOGIST(s):** Glenn Kirkpatrick

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 724262.5  
**NORTHING:** 7478749.68  
**ELEVATION:** 535.37 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 7&7/8 inch hammer 5.3-50 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South West  
**DATE COMMENCED:** 4-April-09  
**DATE COMPLETED:** 5-April-09

### DRILLING DETAILS

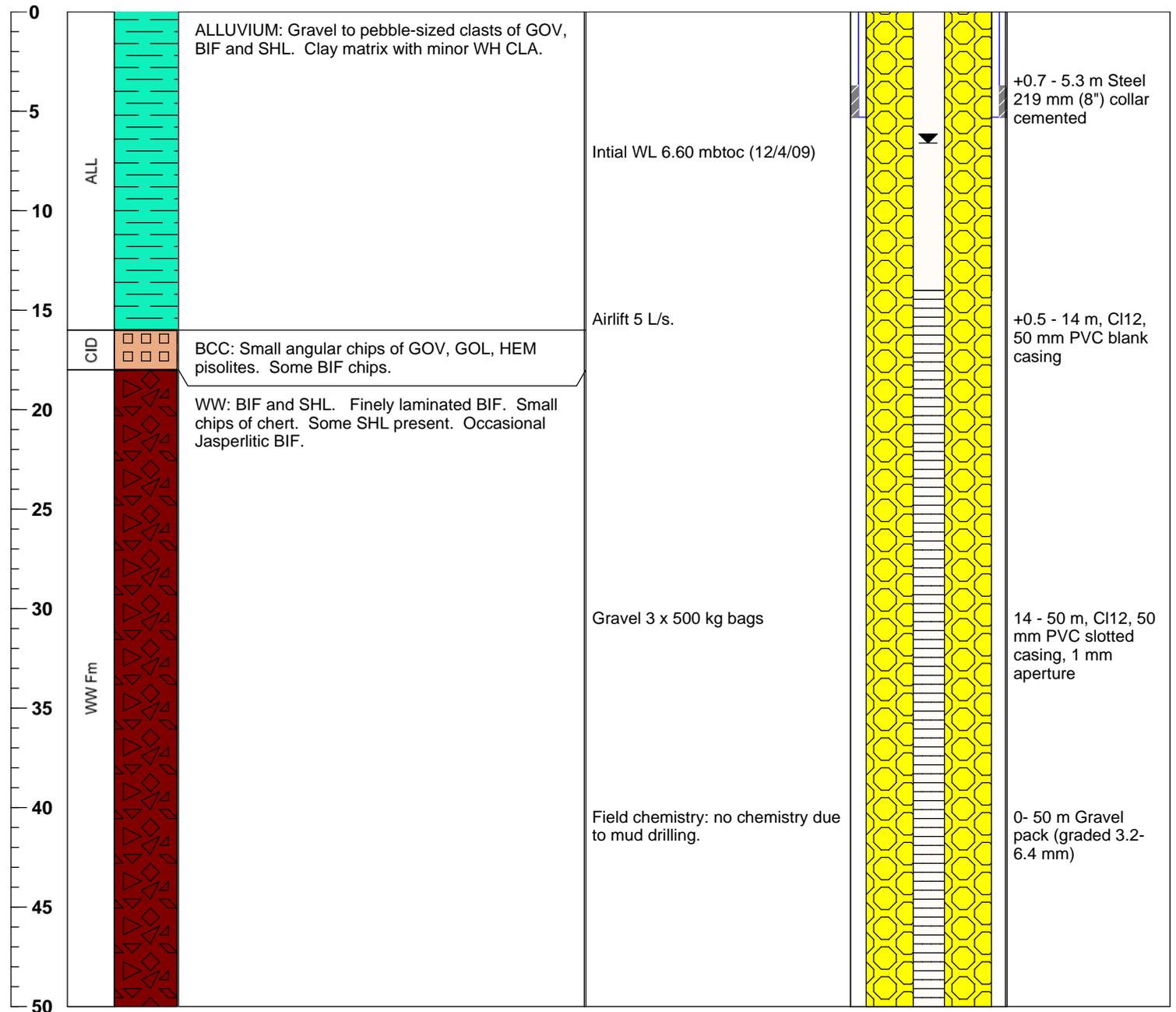
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Richard Attard  
**DRILLING METHOD:** Mud/Rotary  
**HYDROGEOLOGIST(s):** Niall Inverarity

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 724985.01  
**NORTHING:** 7479552.8  
**ELEVATION:** 519.95 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 6 inch mud/rotary 5.3-50 m.

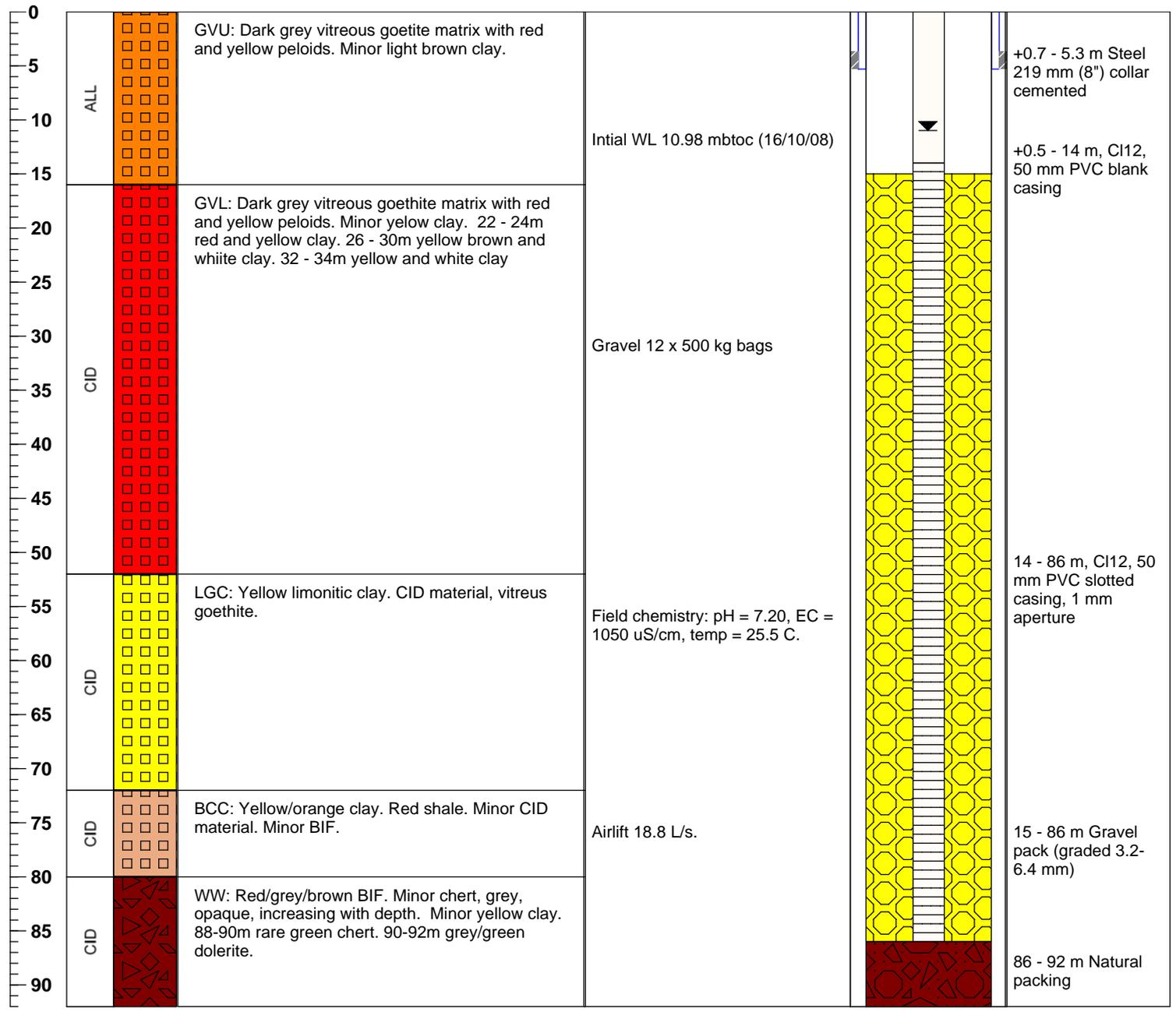
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Ross Pendlebury	<b>EASTING:</b> 724650.04
<b>DATE COMMENCED:</b> 13-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7479786.9
<b>DATE COMPLETED:</b> 15-Oct-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 523.97 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-92 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South West  
**DATE COMMENCED:** 11-Oct-08  
**DATE COMPLETED:** 13-Oct-08

### DRILLING DETAILS

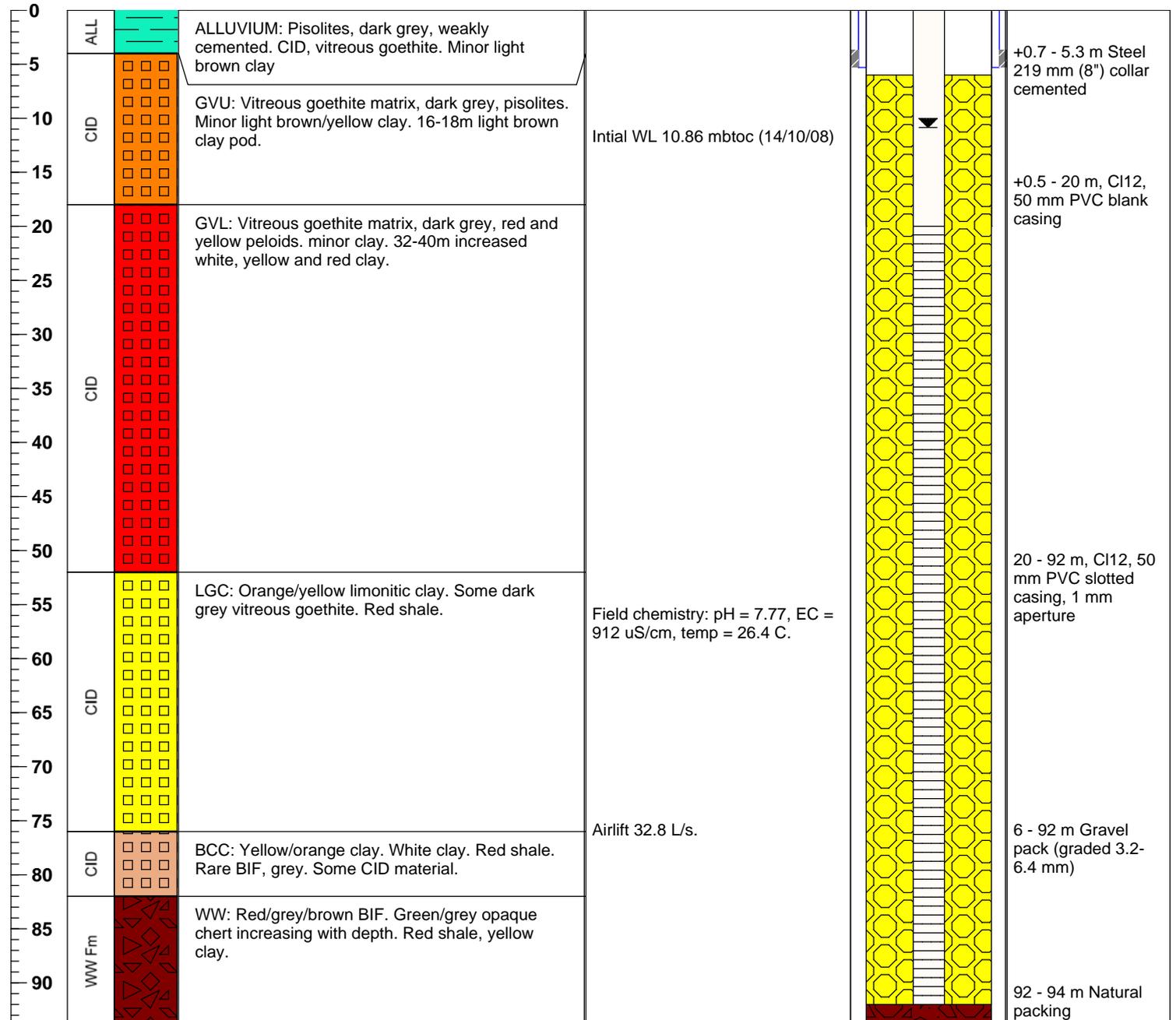
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Andrew McInerney  
**DRILLING METHOD:** Air/Hammer  
**HYDROGEOLOGIST(s):** Emma McGiven

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 724603.66  
**NORTHING:** 7479697.6  
**ELEVATION:** 524.06 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-94 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South West  
**DATE COMMENCED:** 15-Oct-08  
**DATE COMPLETED:** 17-Oct-08

### DRILLING DETAILS

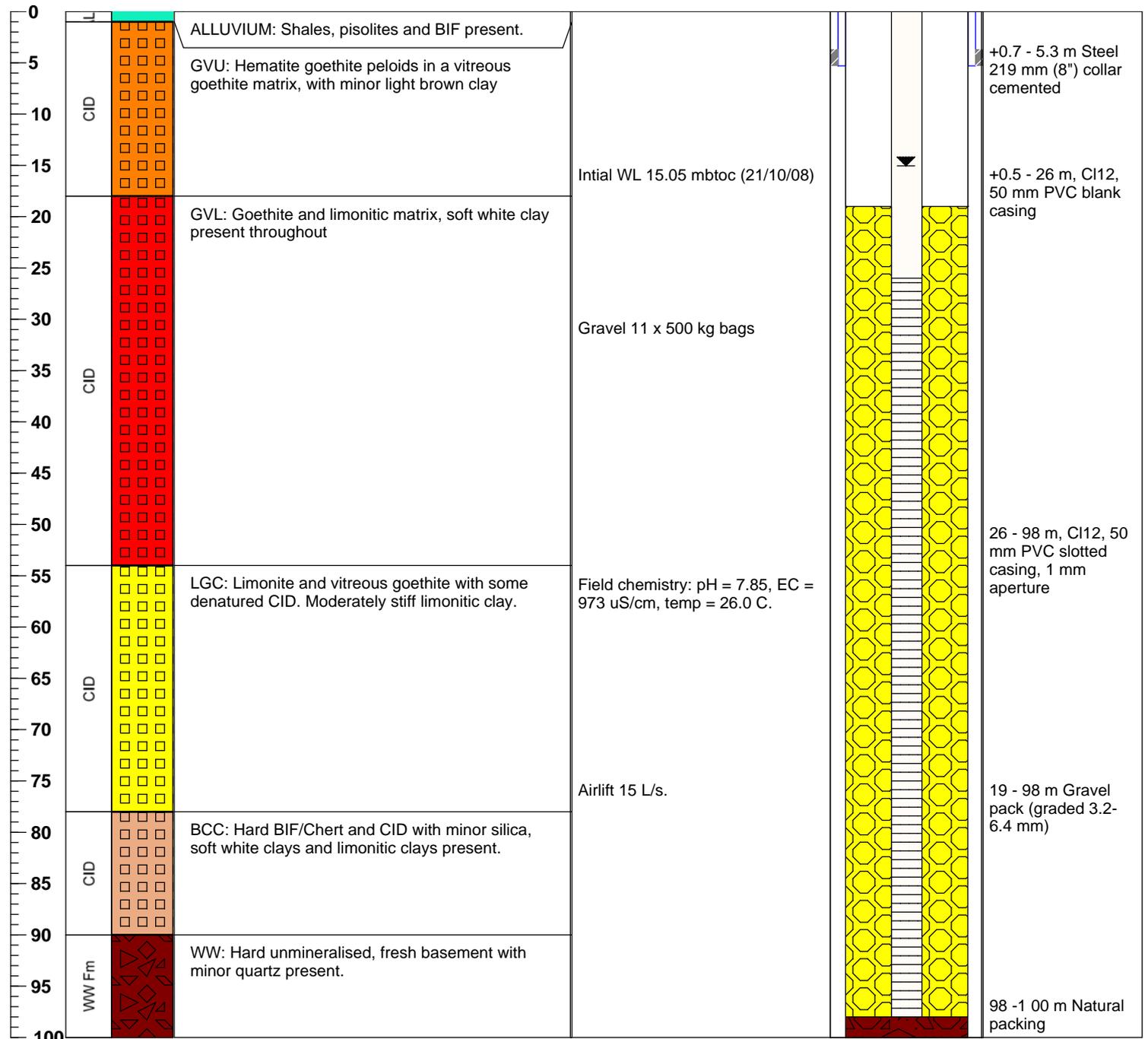
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Andrew McInerney  
**DRILLING METHOD:** Air/Hammer  
**HYDROGEOLOGIST(s):** Glenn Kirkpatrick

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 724818.83  
**NORTHING:** 7480338.17  
**ELEVATION:** 523.75 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-100 m.

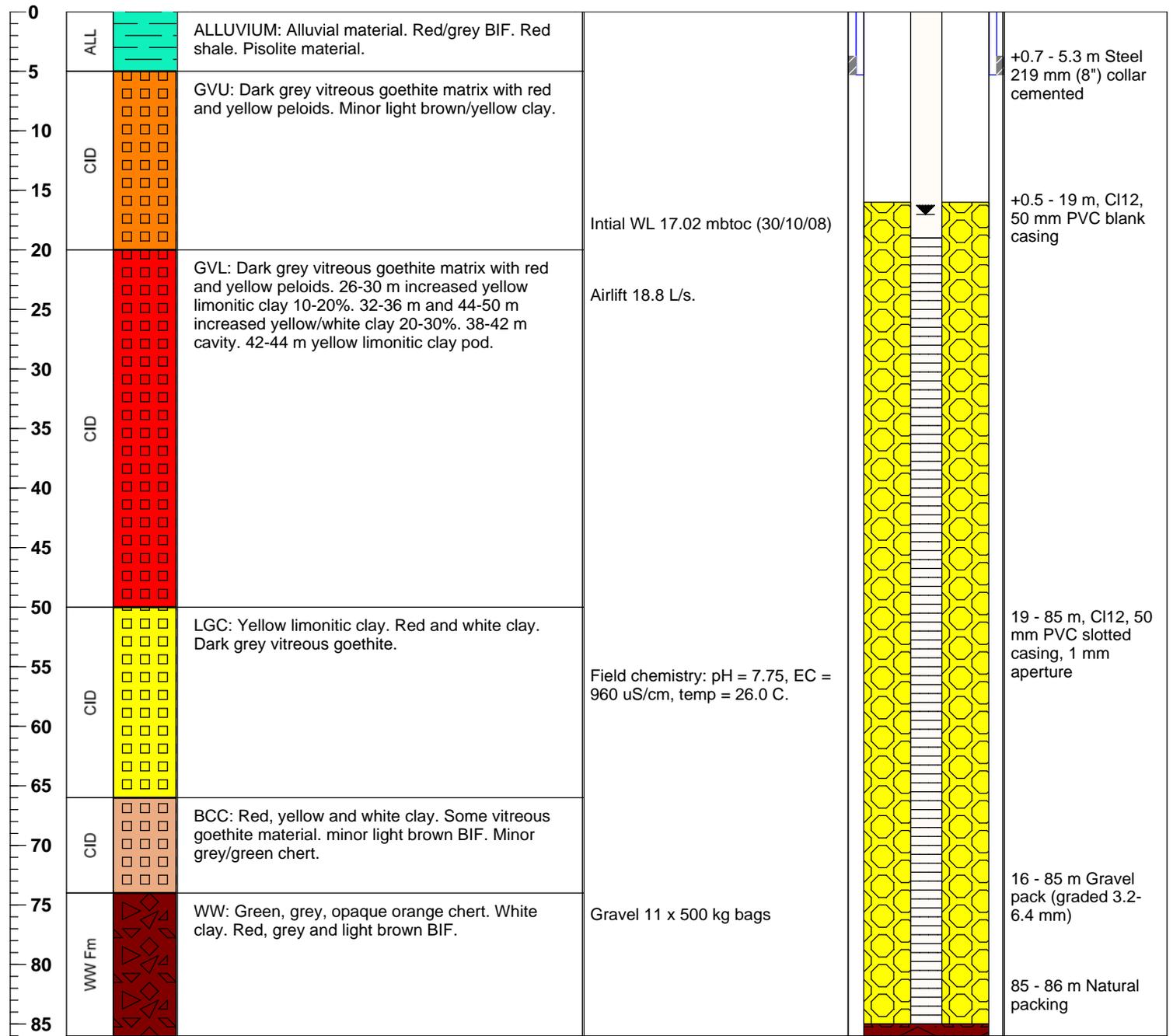
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 724815.64
<b>DATE COMMENCED:</b> 25-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7480446.67
<b>DATE COMPLETED:</b> 27-Oct-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 525.73 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-86 m.

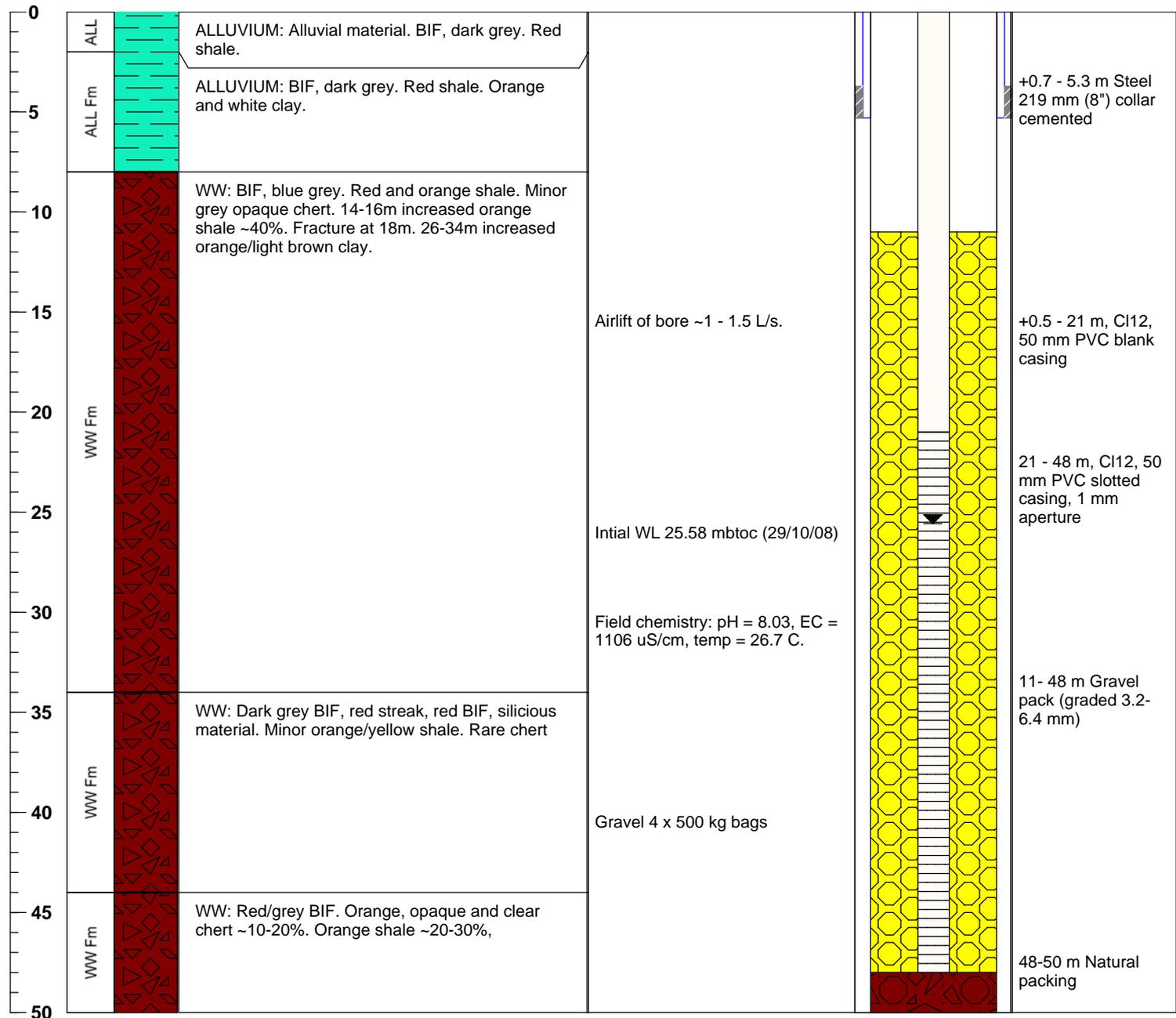
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 724728.41
<b>DATE COMMENCED:</b> 24-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7480567.46
<b>DATE COMPLETED:</b> 25-Oct-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 533.08 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3 m, 7&7/8 inch hammer 5.3-50 m.

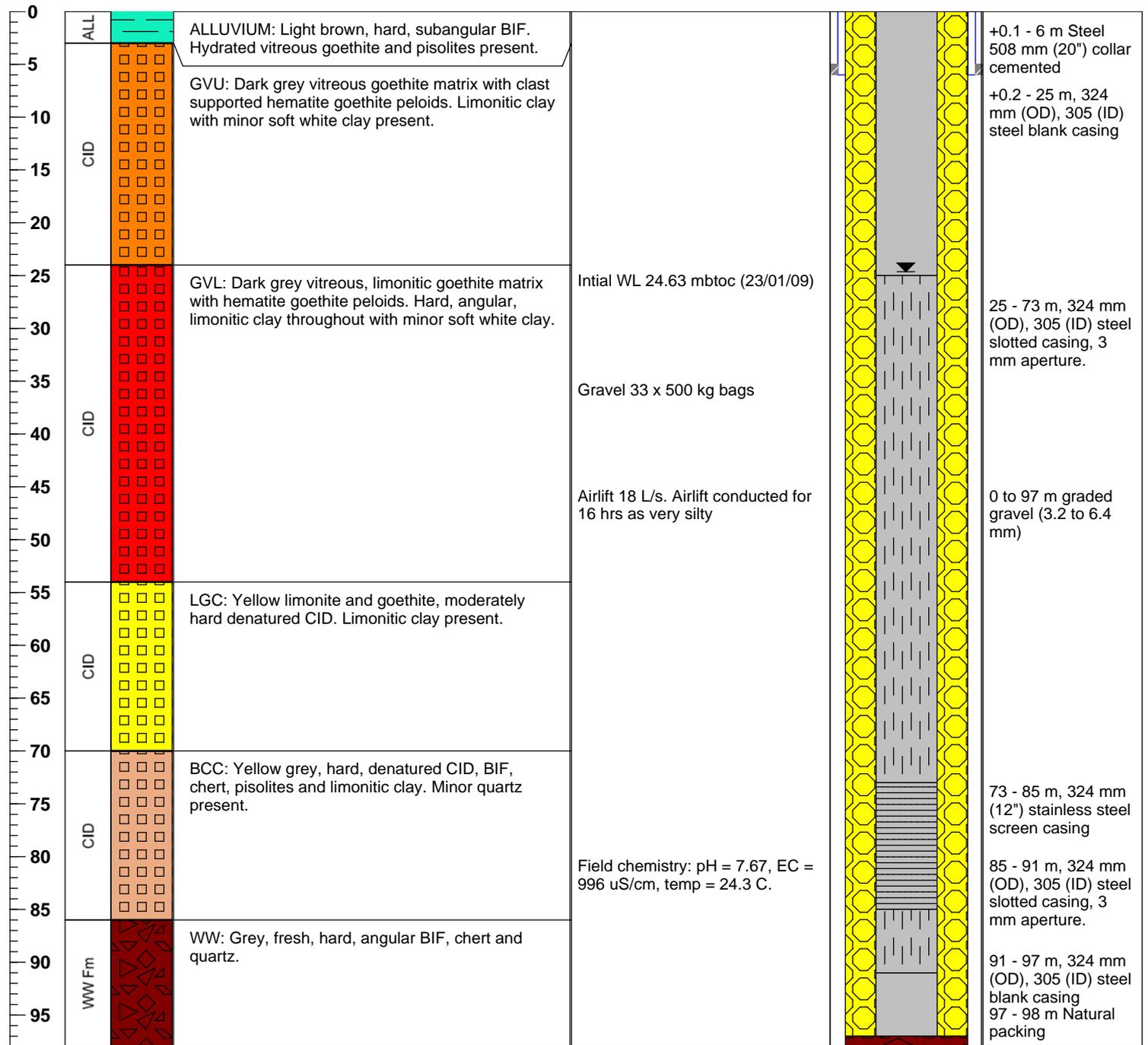
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South East	<b>DRILLER:</b> Lee Martin	<b>EASTING:</b> 731074.59
<b>DATE COMMENCED:</b> 23-Nov-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478278.34
<b>DATE COMPLETED:</b> 30-Nov-08	<b>HYDROGEOLOGIST(s):</b> Emma McGiven	<b>ELEVATION:</b> 508.84 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-6 m, 17.5 inch air/hammer 6-98 m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South East  
**DATE COMMENCED:** 6-Dec-08  
**DATE COMPLETED:** 7-Dec-08

### DRILLING DETAILS

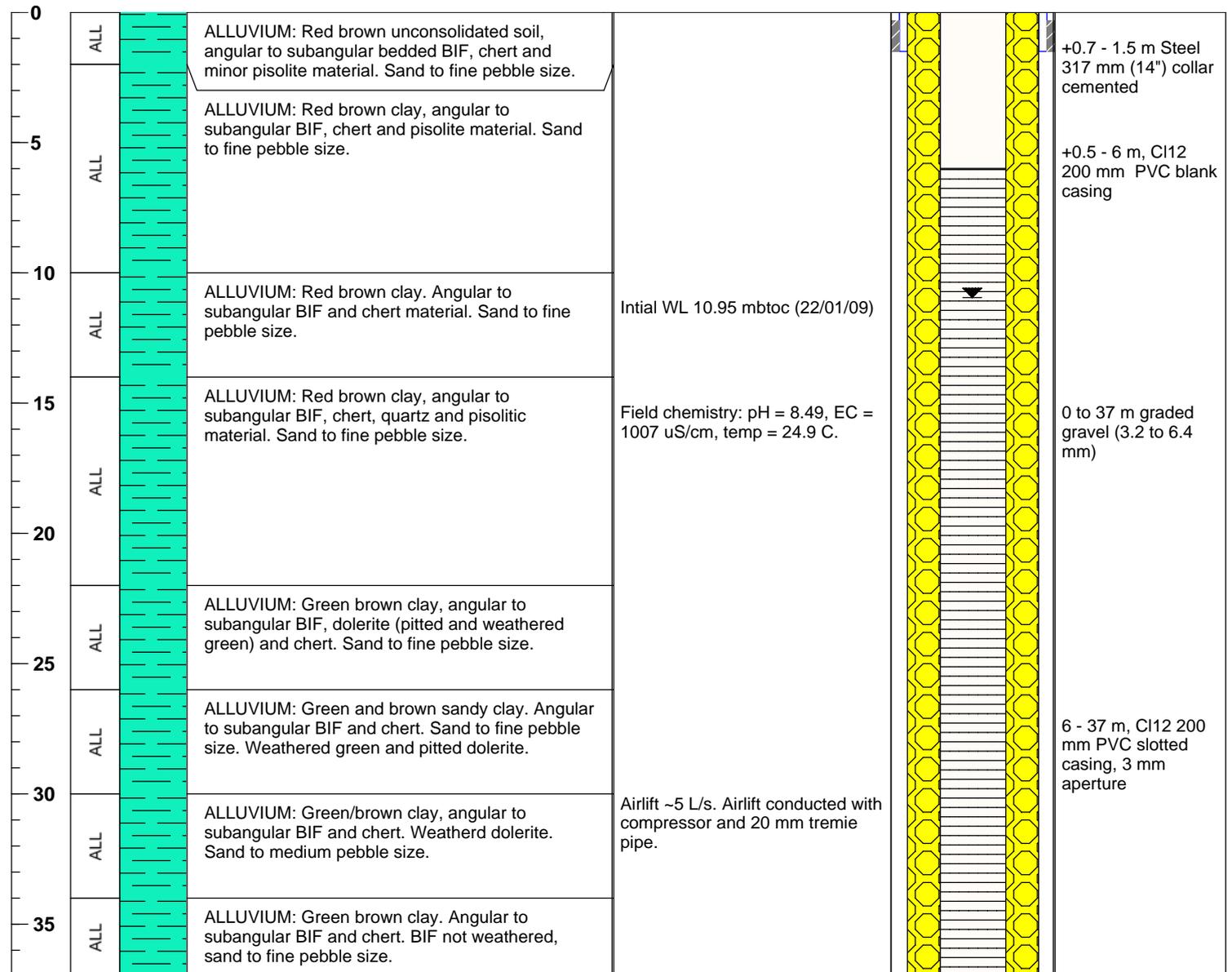
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Lee Martin  
**DRILLING METHOD:** Mud/Rotary  
**HYDROGEOLOGIST(s):** Simon Page

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 731867.37  
**NORTHING:** 7478031.14  
**ELEVATION:** 499.46 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-1.5 m, 12.5 inch mud/rotary 1.5-37 m.

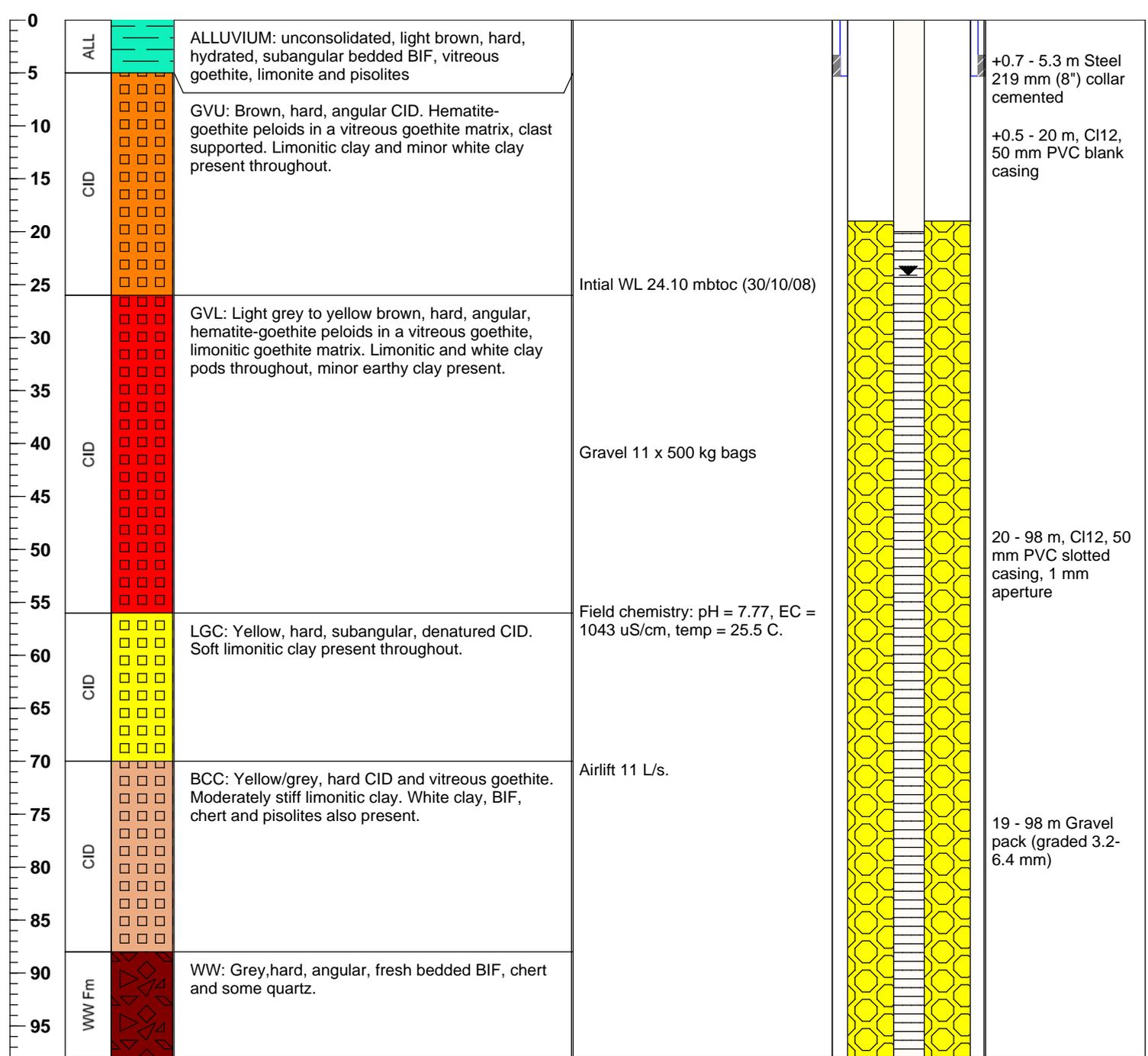
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South East	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 731085.55
<b>DATE COMMENCED:</b> 21-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478261.46
<b>DATE COMPLETED:</b> 22-Oct-09	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 508.98 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.3m, 7&7/8 inch hammer 5.3-98m.

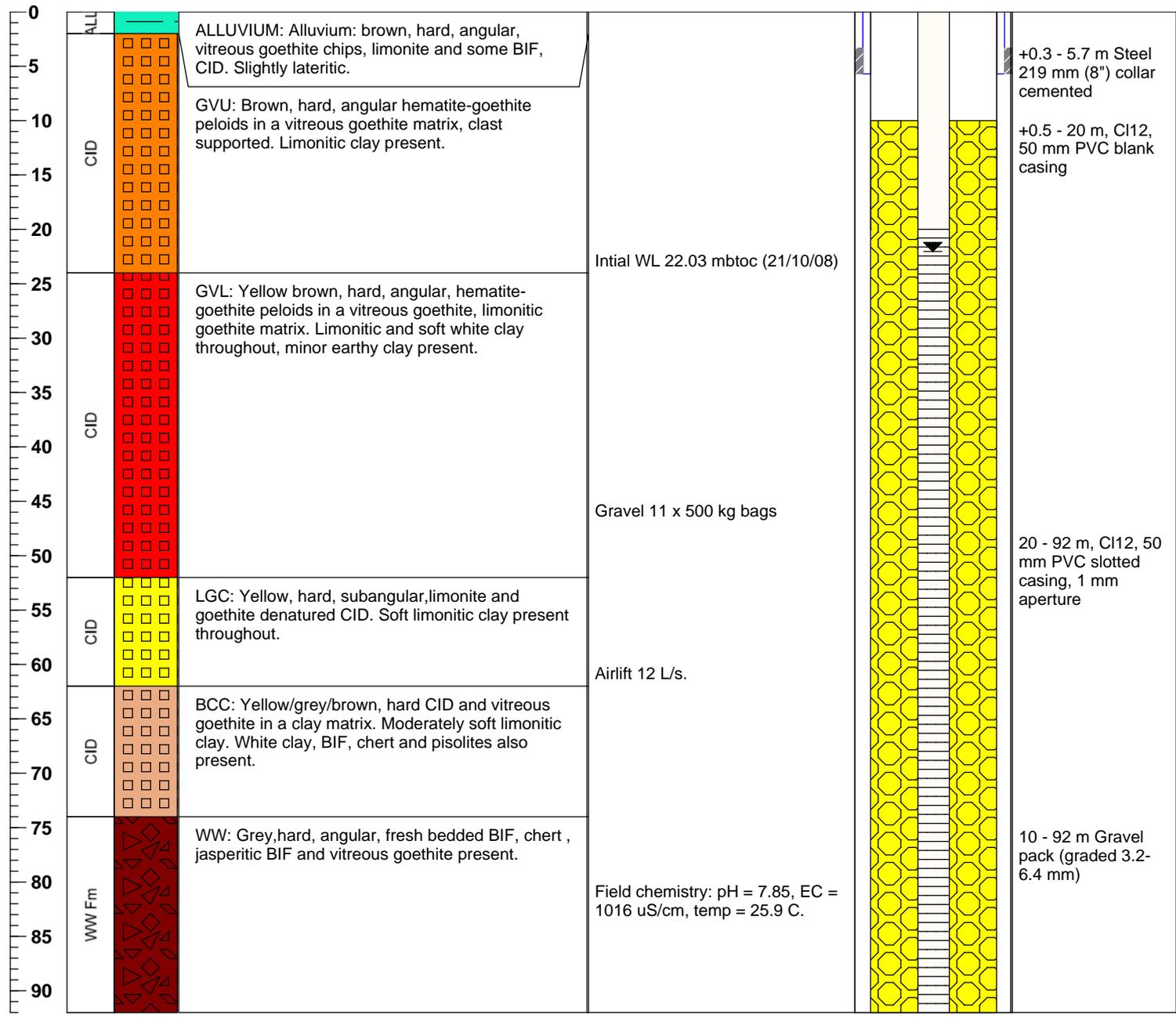
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South East	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 731173.6
<b>DATE COMMENCED:</b> 19-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478216.87
<b>DATE COMPLETED:</b> 21-Oct-09	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 508.65 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.7m, 7&7/8 inch hammer 5.7-92m.

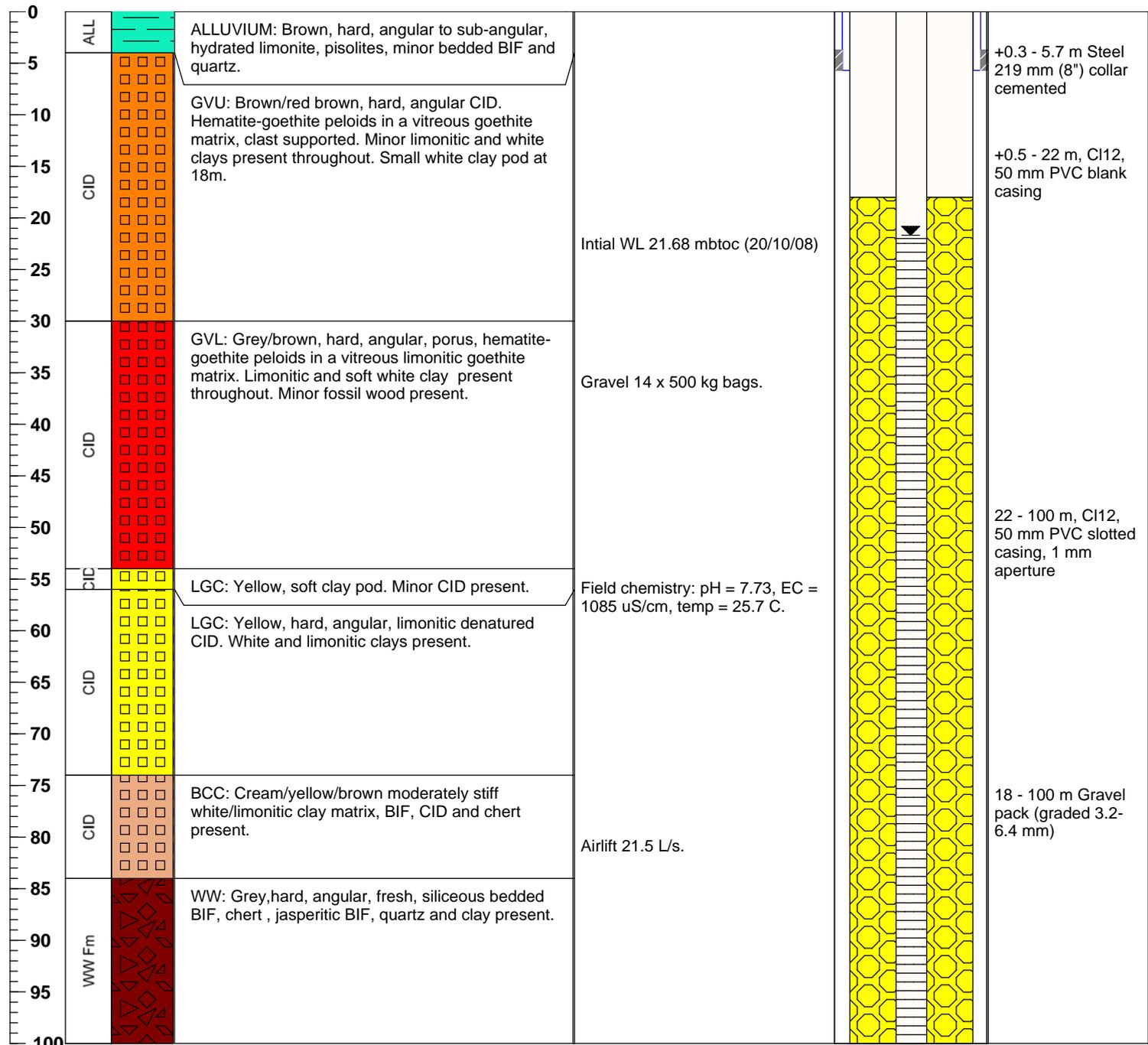
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South East	<b>DRILLER:</b> Andrew McInerney	<b>EASTING:</b> 730894.2
<b>DATE COMMENCED:</b> 17-Oct-08	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478385.95
<b>DATE COMPLETED:</b> 19-Oct-09	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 507.72 mRL (TOC)

Well Notes: Hole diameter 12 inch hammer 0-5.7m, 7&7/8 inch hammer 5.7-100m.

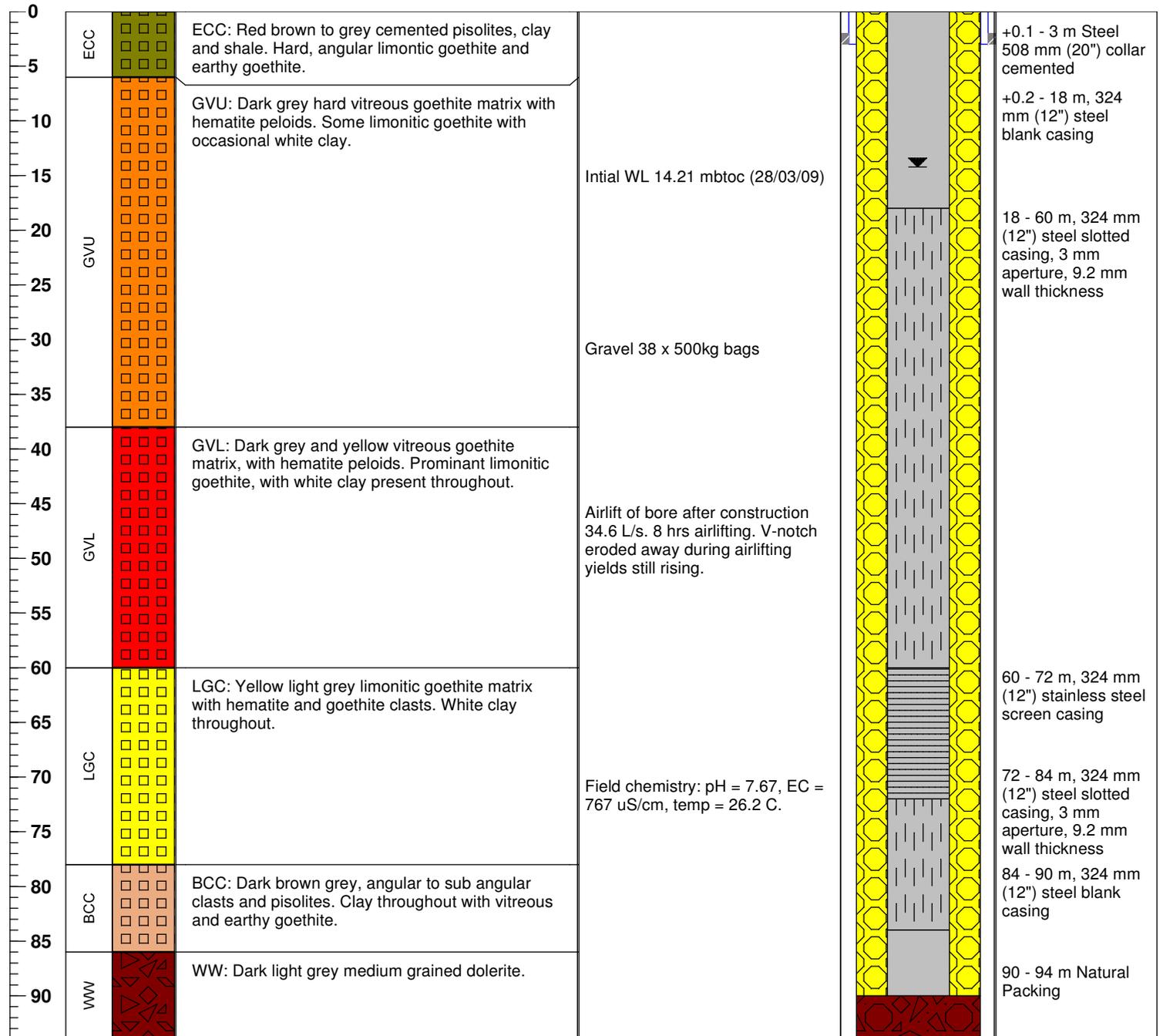
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South West	<b>DRILLER:</b> Michael Larkman	<b>EASTING:</b> 723778.163
<b>DATE COMMENCED:</b> 19-Mar-09	<b>DRILLING METHOD:</b> Air/Hammer	<b>NORTHING:</b> 7478714.397
<b>DATE COMPLETED:</b> 23-Mar-09	<b>HYDROGEOLOGIST(s):</b> Niall Inverarity	<b>ELEVATION:</b> 531.26 mRL (TOC)

Well Notes: Hole diameter 24 inch air/roller 0-3 m, 17.5 inch air/hammer 3-94m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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### HOLE DETAILS

**PROJECT:** Yandicoogina  
**LOCATION:** Junction South East  
**DATE COMMENCED:** 5-Dec-08  
**DATE COMPLETED:** 6-Dec-08

### DRILLING DETAILS

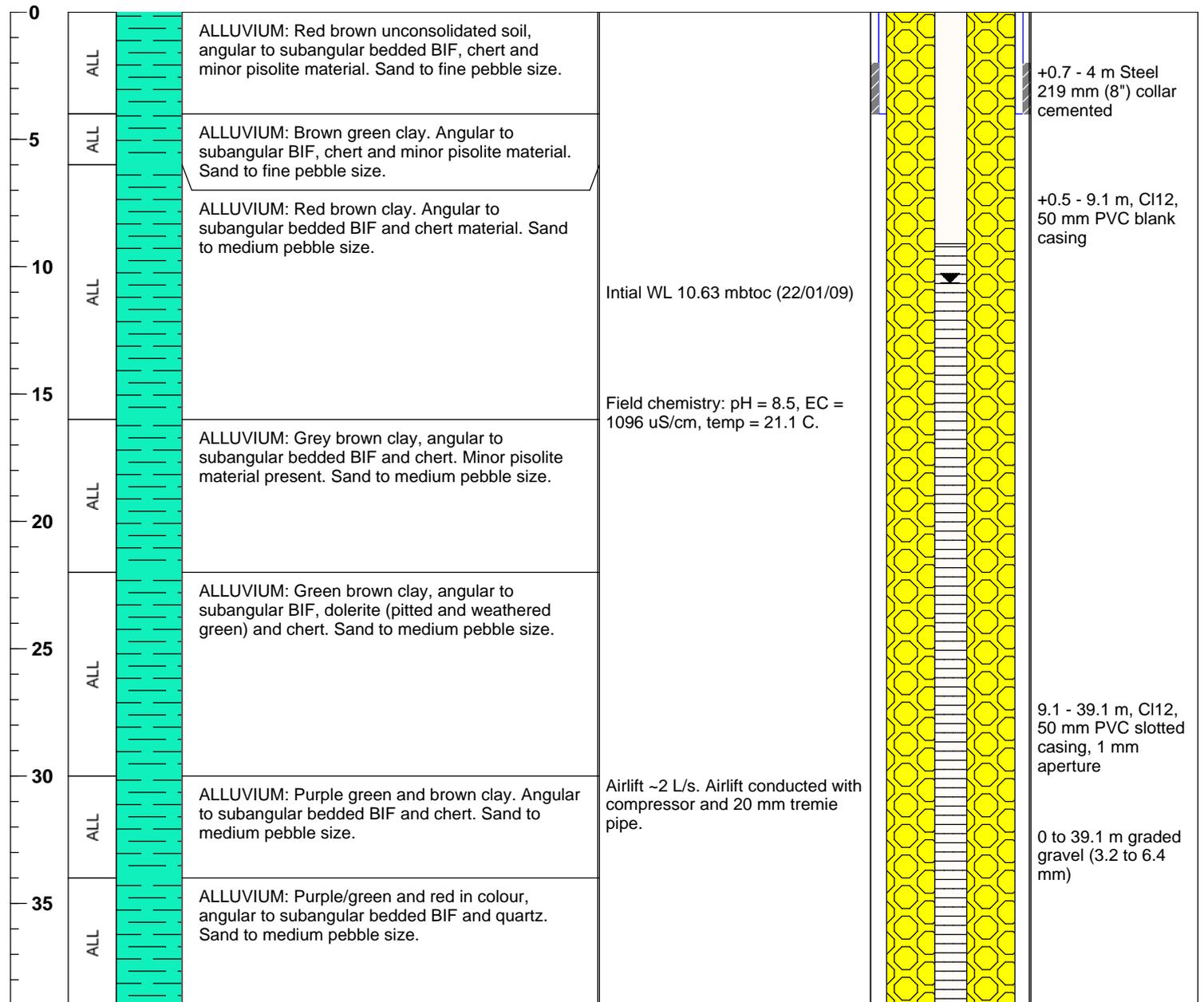
**DRILLING COMPANY:** Nudrill Pty Ltd  
**DRILLER:** Shaun McGinley  
**DRILLING METHOD:** Mud/Rotary  
**HYDROGEOLOGIST(s):** Simon Page

### LOCATION

**GRID NAME:** MGA94 Zone 50  
**EASTING:** 731931.96  
**NORTHING:** 7477992.82  
**ELEVATION:** 499.69 mRL (TOC)

Well Notes: Hole diameter 10 inch air/hammer 0-4 m, 6 inch mud/rotary 4-39.1m.

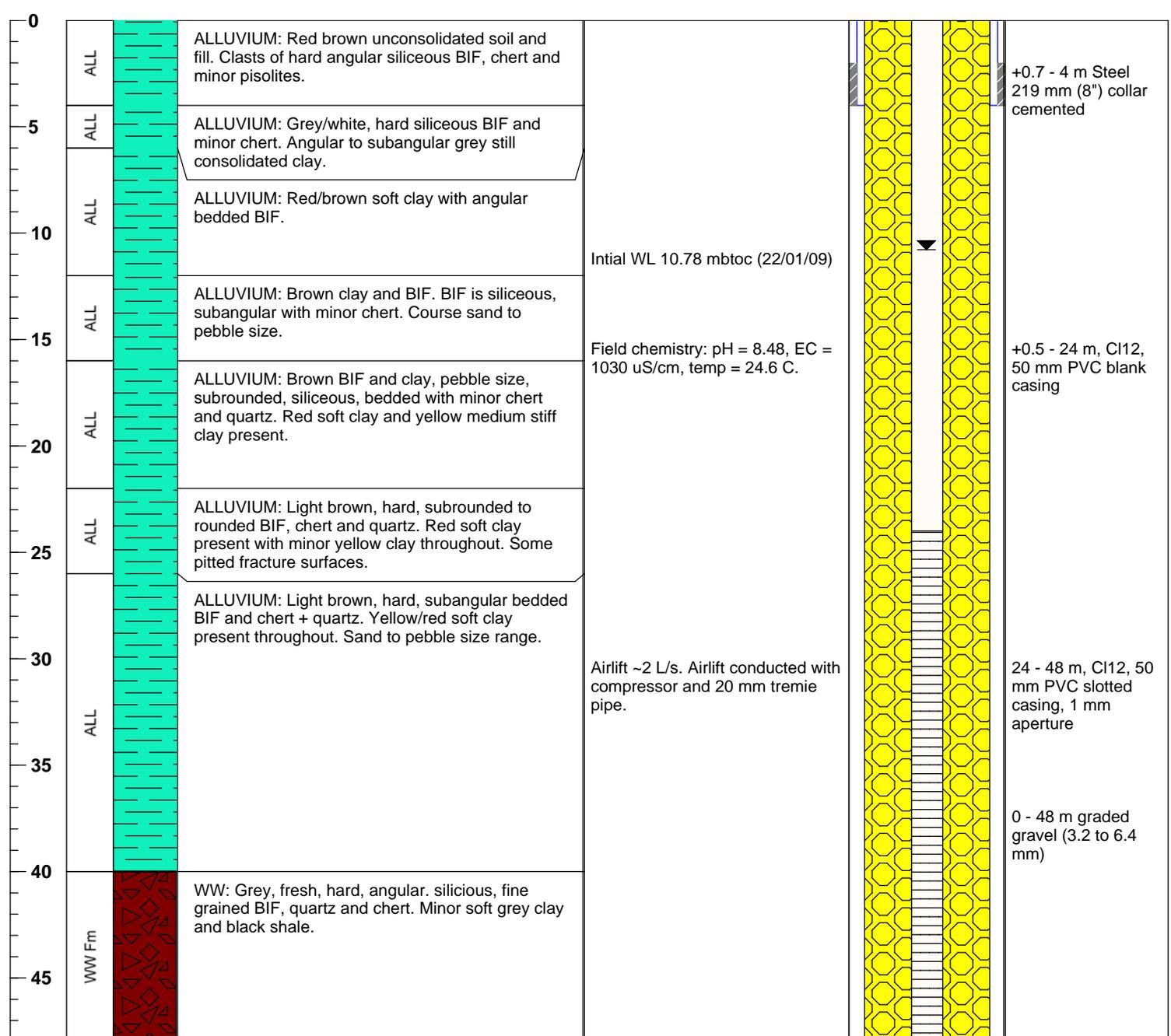
Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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HOLE DETAILS	DRILLING DETAILS	LOCATION
<b>PROJECT:</b> Yandicoogina	<b>DRILLING COMPANY:</b> Nudrill Pty Ltd	<b>GRID NAME:</b> MGA94 Zone 50
<b>LOCATION:</b> Junction South East	<b>DRILLER:</b> Shaun McGinley	<b>EASTING:</b> 732128.48
<b>DATE COMMENCED:</b> 3-Dec-08	<b>DRILLING METHOD:</b> Mud/Rotary	<b>NORTHING:</b> 7477866.39
<b>DATE COMPLETED:</b> 5-Dec-09	<b>HYDROGEOLOGIST(s):</b> Glenn Kirkpatrick	<b>ELEVATION:</b> 500.18 mRL (TOC)

Well Notes: Hole diameter 12.5 inch mud/rotary 0-4 m, 6 inch mud/rotary 4-48m.

Depth (mbgl)	Geology	Lithology	Lithological Description	Field Notes	Well Design	Well Construction
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## Appendix 2: Laboratory Analysis of Groundwater

	Units	WB09Y0XB001	WB08YJSW001	WB08YJSW02	WB09YJSW004	WB09YJSW05	WB09YJSW006	WB09YJSW007	WB08YJSE001	WB08YJSE002
<b>Chloride, Cl</b>	mg/L	84.9	94.7	94	99	87	98	110	100	110
<b>Sulphate, SO4</b>	mg/L	41.1	41.4	33	41.8	41	38.2	50	49	54
<b>Bicarbonate, HCO3</b>	mg/L	315	261	290	263	310	257	290	350	310
<b>Calcium, Ca</b>	mg/L	42.141	41.012	37	41.643	37	40.723	42	50	44
<b>Magnesium, Mg</b>	mg/L	42.847	40.011	36	40.284	38	39.582	39	44	41
<b>Potassium, K</b>	mg/L	6.049	6.39	5.7	6.213	5.3	6.228	5.8	7.2	6.6
<b>Sodium, Na</b>	mg/L	72.282	63.162	63	62.023	64	60.38	63	73	64
<b>Bromine, Br</b>	mg/L	0.3931	0.447	0.5	0.4311	0.4	0.446	0.5	0.4	0.5
<b>Silica, Si</b>	mg/L	20.76	20.35	22	20.14	22	20.03	23	26	0.42
<b>Soluble Iron, Fe</b>	mg/L	0.031	0.09	< 0.02	< 0.02	< 0.02	< 0.02	0.12	< 0.02	0.39

### Appendix 3: Measured redox potential for the production bores during pumping test

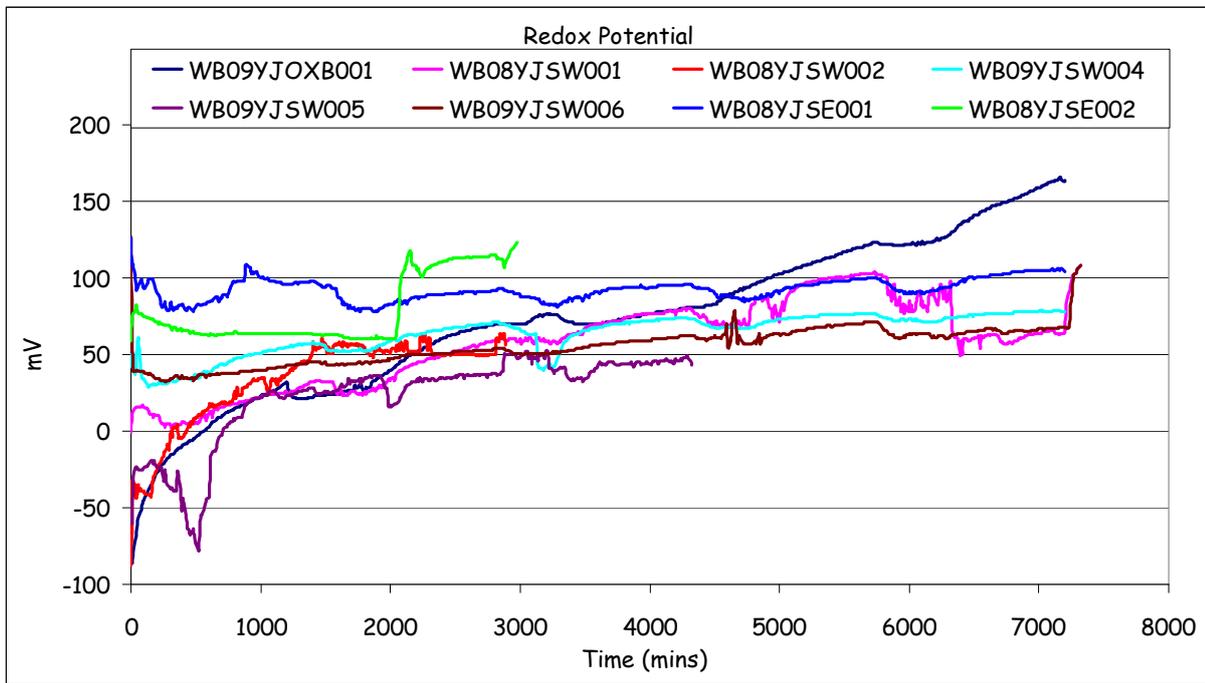
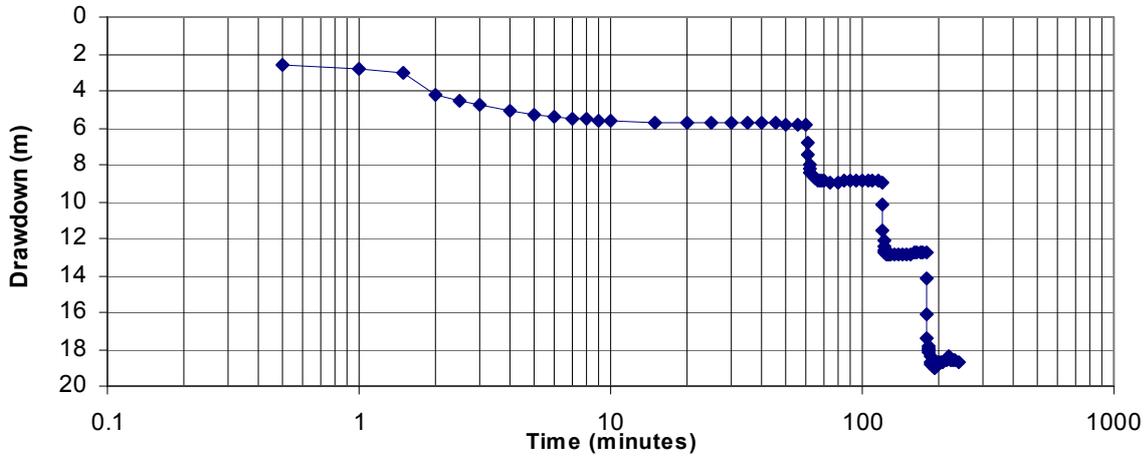


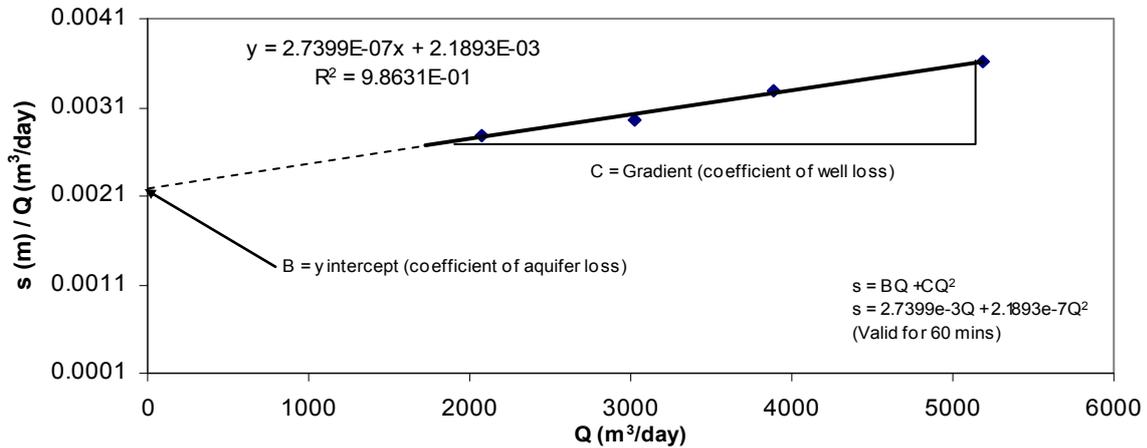
Figure 16 Redox potential measurements for the all test pumping bores

## Appendix 4: Step test analysis

**WB09YOXB001 Step Discharge Pumping Test (continuous)**

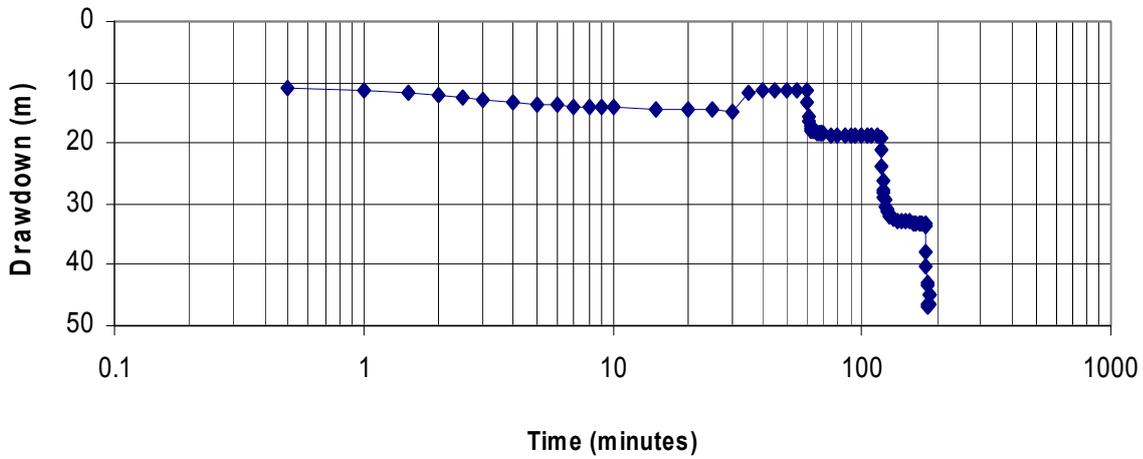


**Analytical Plot of s/Q vs Q**

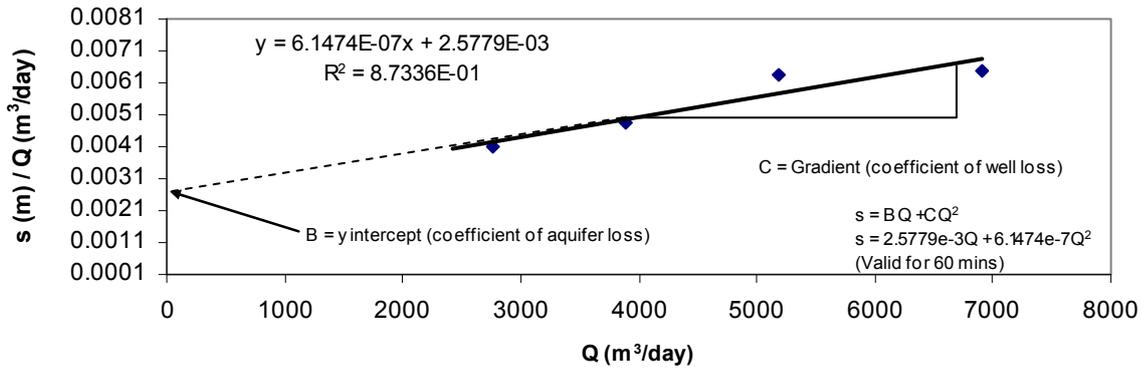


$s_{w(n)} = BQ_n + CQ_n^2$ (Rorabaugh's equation)						
Where, B	= Intercept with y axis (coefficient of aquifer loss or laminar flow)					
C	= Gradient (coefficient of turbulent flow loss or apparent well loss)					
s	= Drawdown in the borehole					
P	= Value determined using Rorabaugh's method of superposition					
Components of the equation BQ and CQ <sup>2</sup> are termed the aquifer loss and apparent well loss respectively. They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^2)} \times 100$						
E <sub>w</sub> or Well Efficiency represents the proportion of drawdown caused by laminar flow						
Comparison of Observed and Predicted Drawdowns						
Step	Discharge	Discharge (Q)	Observed	Predicted		Apparent
(60 min steps)	(L/s)	(m <sup>3</sup> /d)	Corrected	Drawdown	s/Q	Efficiency
			Drawdown	(metres)		(E <sub>w</sub> )
			(s)			(%)
			(metres)			
1	24	2074	5.785	5.72	0.00279	79.40
2	35	3024	8.94	9.13	0.002956	72.55
3	45	3888	12.75	12.65	0.003279	67.27
4	60	5184	18.735	18.71	0.003614	60.65

**WB08YJSW001 Step Discharge Pumping Test (continuous)**

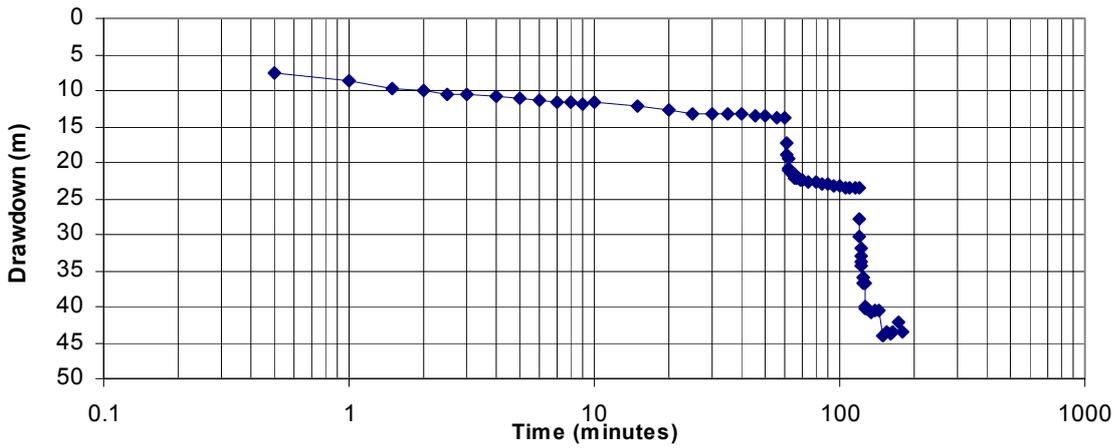


**Analytical Plot of s/Q vs Q**

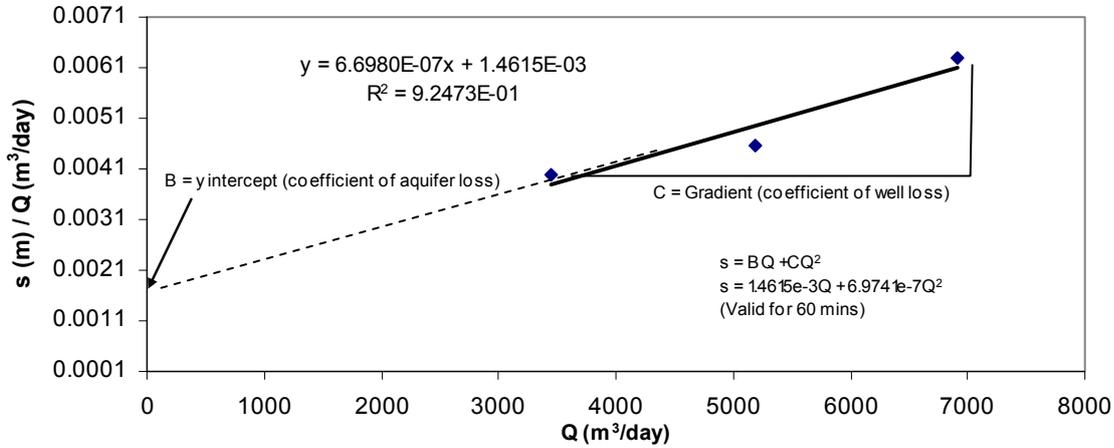


$s_{w(n)} = BQ_n + CQ_n^P$ (Rorabaugh's equation)						
Where, B	= Intercept with y axis (coefficient of aquifer loss or laminar flow)					
C	= Gradient (coefficient of turbulent flow loss or apparent well loss)					
s	= Drawdown in the borehole					
P	= Value determined using Rorabaugh's method of superposition					
Components of the equation $BQ$ and $CQ^2$ are termed the aquifer loss and apparent well loss respectively. They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^P)} \times 100$						
Ew or Well Efficiency represents the proportion of drawdown caused by laminar flow						
Comparison of Observed and Predicted Drawdowns						
Step	Discharge	Discharge (Q)	Observed	Predicted		Apparent
(60 min steps)	(L/s)	(m3/d)	Corrected	Drawdown	s/Q	Efficiency
			Drawdown	(metres)		(Ew)
			(metres)			(%)
1	32	2765	11.315	11.83	0.004093	60.27
2	45	3888	18.95	19.32	0.004874	51.89
3	60	5184	33.04	29.88	0.006373	44.72
4	80	6912	44.91	47.19	0.006497	37.76

**WB08YJSW002 Step Discharge Pumping Test (continuous)**

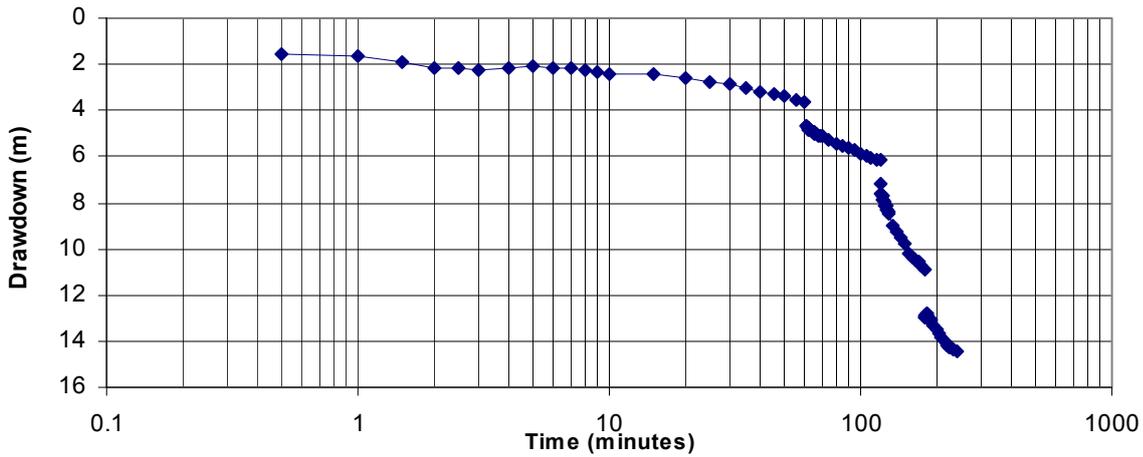


**Analytical Plot of s/Q vs Q**

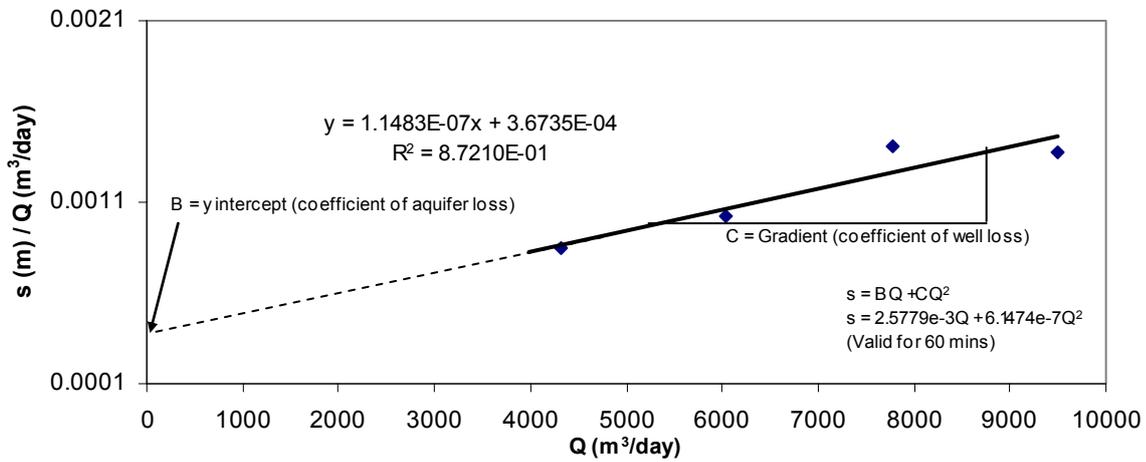


$s_{w(n)} = BQ_n + CQ_n^P$ (Rorabaugh's equation)						
Where, B = Intercept with y axis (coefficient of aquifer loss or laminar flow)						
C = Gradient (coefficient of turbulent flow loss or apparent well loss)						
s = Drawdown in the borehole						
P = Value determined using Rorabaugh's method of superposition						
Components of the equation BQ and CQ <sup>2</sup> are termed the aquifer loss and apparent well loss respectively.						
They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^P)} \times 100$						
E <sub>w</sub> or Well Efficiency represents the proportion of drawdown caused by laminar flow						
Comparison of Observed and Predicted Drawdowns						
			Observed	Predicted		Apparent
Step	Discharge	Discharge (Q)	Corrected	Drawdown	s/Q	Efficiency
(60 min steps)	(L/s)	(m3/d)	Drawdown	(metres)		(E <sub>w</sub> )
			(metres)			%
1	40	3456	13.71	13.38	0.003967	37.75
2	60	5184	23.6	26.32	0.004552	28.79
3	80	6912	43.42	43.42	0.006282	23.26

**WB09YJSW004 Step Discharge Pumping Test (continuous)**

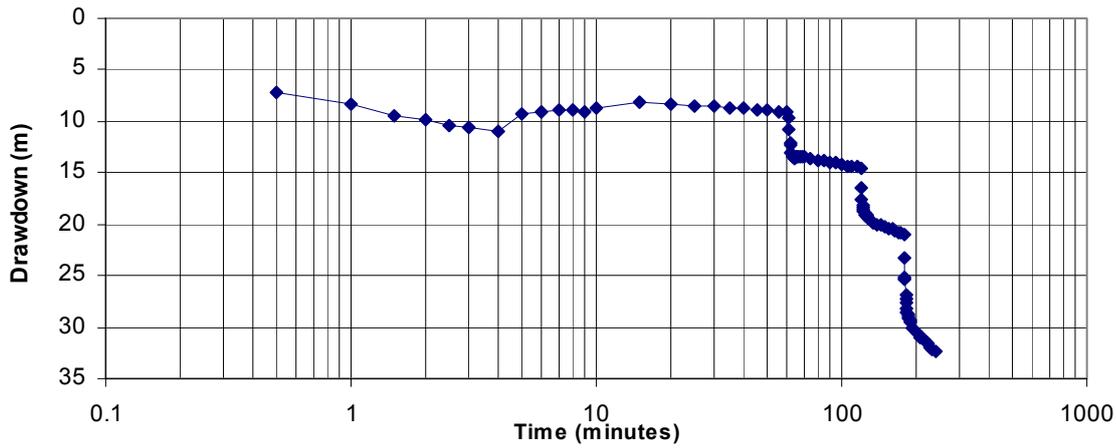


**Analytical Plot of s/Q vs Q**

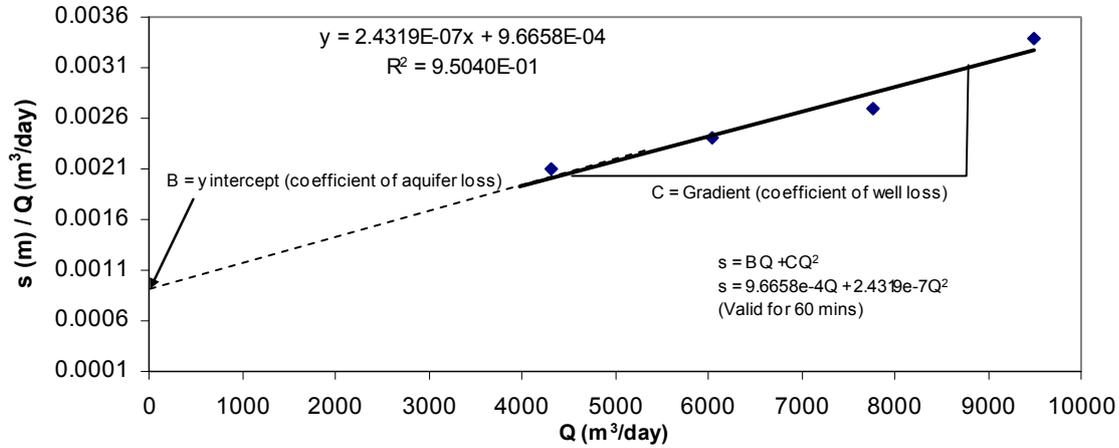


$s_{w(t)} = BQ_n + CQ_n^P$ (Rorabaugh's equation)						
Where, B	= Intercept with y axis (coefficient of aquifer loss or laminar flow)					
C	= Gradient (coefficient of turbulent flow loss or apparent well loss)					
s	= Drawdown in the borehole					
P	= Value determined using Rorabaugh's method of superposition					
Components of the equation BQ and CQ <sup>2</sup> are termed the aquifer loss and apparent well loss respectively.						
They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^P)} \times 100$						
E <sub>w</sub> or Well Efficiency represents the proportion of drawdown caused by laminar flow						
Comparison of Observed and Predicted Drawdowns						
Step	Discharge	Discharge (Q)	Observed	Predicted		Apparent
(60 min steps)	(L/s)	(m3/d)	Corrected	Drawdown	s/Q	Efficiency
			Drawdown	(metres)		(E <sub>w</sub> )
			(metres)			(%)
1	50	4320	3.65	3.73	0.000845	42.55
2	70	6048	6.16	6.42	0.001019	34.60
3	90	7776	10.905	9.80	0.001402	29.15
4	110	9504	13.1	13.86	0.001378	25.18

**WB09YJSW005 Step Discharge Pumping Test (continuous)**

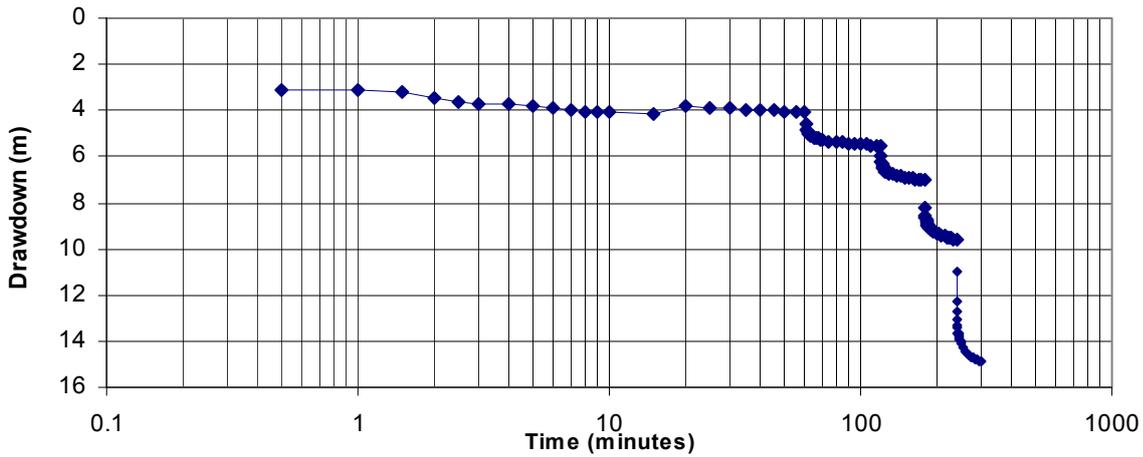


**Analytical Plot of s/Q vs Q**

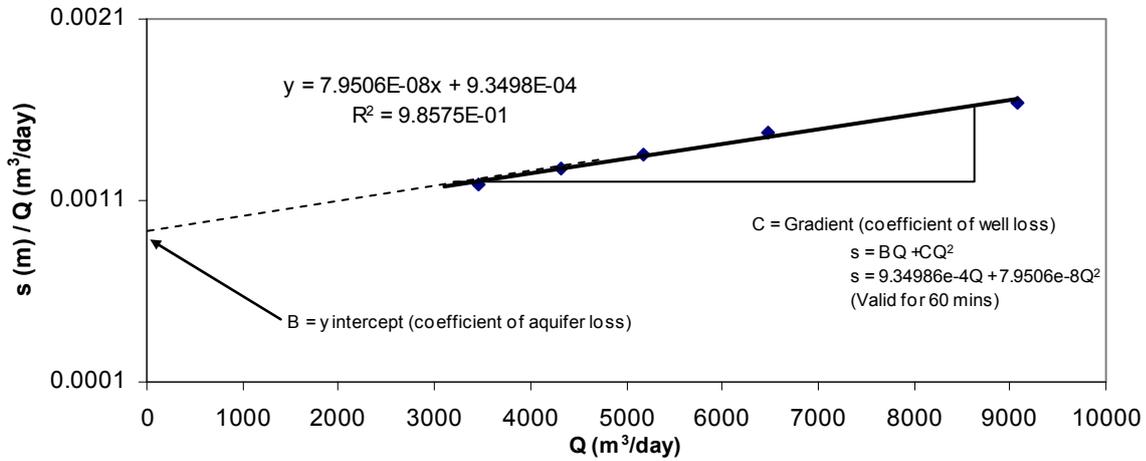


$s_{w(n)} = BQ_n + CQ_n^P$ (Rorabaugh's equation)						
Where, B	= Intercept with y axis (coefficient of aquifer loss or laminar flow)					
C	= Gradient (coefficient of turbulent flow loss or apparent well loss)					
s	= Drawdown in the borehole					
P	= Value determined using Rorabaugh's method of superposition					
Components of the equation $BQ$ and $CQ^2$ are termed the aquifer loss and apparent well loss respectively. They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^2)} \times 100$						
Ew or Well Efficiency represents the proportion of drawdown caused by laminar flow						
Comparison of Observed and Predicted Drawdowns						
Step	Discharge	Discharge (Q)	Observed	Predicted		Apparent
(60 min steps)	(L/s)	(m3/d)	Corrected	Drawdown	s/Q	Efficiency
			Drawdown	(metres)		(Ew)
			(metres)			(%)
1	50	4320	9.04	8.71	0.002093	47.92
2	70	6048	14.54	14.74	0.002404	39.66
3	90	7776	20.98	22.22	0.002698	33.82
4	110	9504	32.27	31.15	0.003395	29.49

**WB09YJSW006 Step Discharge Pumping Test (continuous)**

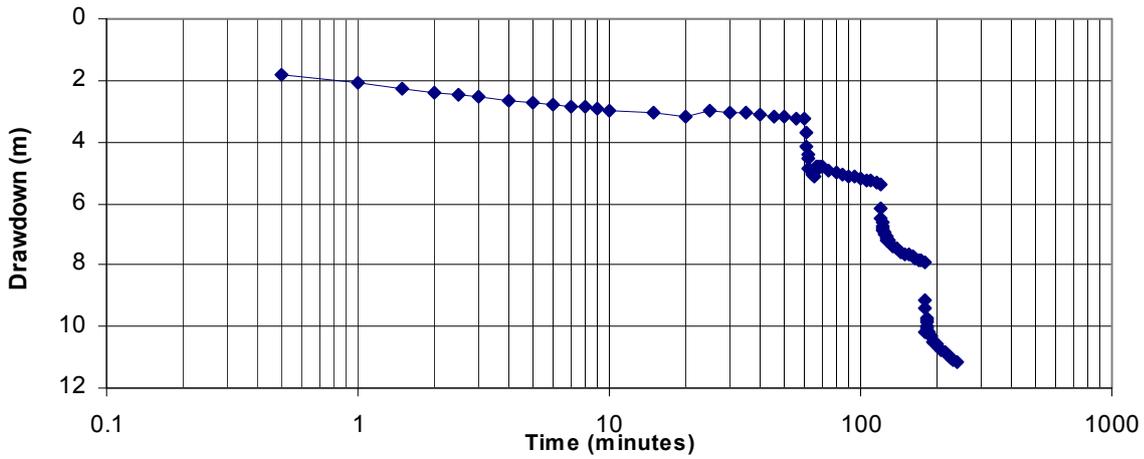


**Analytical Plot of s/Q vs Q**

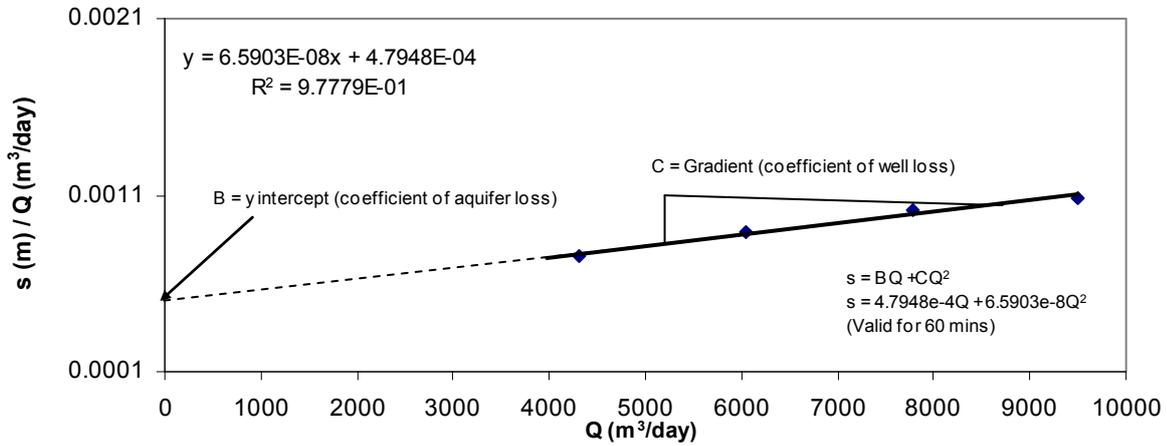


$s_{w(t)} = BQ_n + CQ_n^P$ (Rorabaugh's equation)						
Where, B	= Intercept with y axis (coefficient of aquifer loss or laminar flow)					
C	= Gradient (coefficient of turbulent flow loss or apparent well loss)					
s	= Drawdown in the borehole					
P	= Value determined using Rorabaugh's method of superposition					
Components of the equation BQ and CQ <sup>2</sup> are termed the aquifer loss and apparent well loss respectively.						
They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^P)} \times 100$						
E <sub>w</sub> or Well Efficiency represents the proportion of drawdown caused by laminar flow						
<b>Comparison of Observed and Predicted Drawdowns</b>						
Step	Discharge	Discharge (Q)	Observed	Predicted		Apparent
(60 min steps)	(L/s)	(m3/d)	Corrected	Drawdown	s/Q	Efficiency
			Drawdown	(metres)		(E <sub>w</sub> )
			(metres)			(%)
1	40	3456	4.105	4.18	0.001188	77.29
2	50	4320	5.53	5.52	0.00128	73.13
3	60	5184	7.04	6.98	0.001358	69.40
4	75	6480	9.58	9.40	0.001478	64.47
5	105	9072	14.855	15.03	0.001637	56.45

**WB09YJSW007 Step Discharge Pumping Test (continuous)**

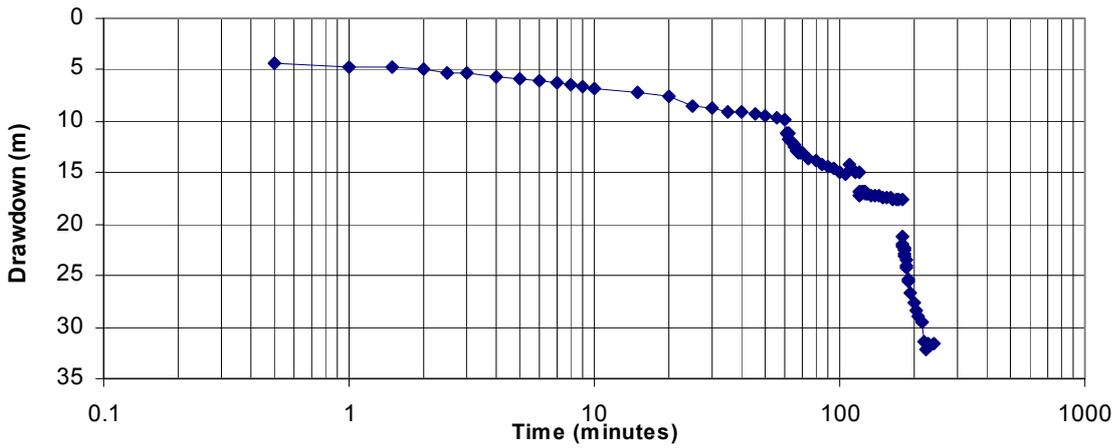


**Analytical Plot of s/Q vs Q**

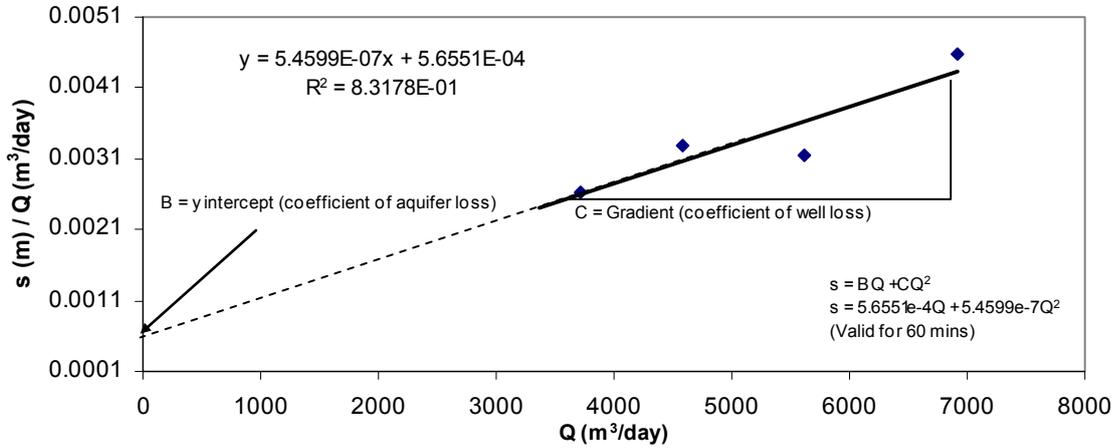


$s_{w(n)} = BQ_n + CQ_n^2$ (Rorabaugh's equation)						
Where, B	= Intercept with y axis (coefficient of aquifer loss or laminar flow)					
C	= Gradient (coefficient of turbulent flow loss or apparent well loss)					
s	= Drawdown in the borehole					
P	= Value determined using Rorabaugh's method of superposition					
Components of the equation BQ and CQ <sup>2</sup> are termed the aquifer loss and apparent well loss respectively. They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = (BQ / (BQ + CQ^2)) \times 100$						
E <sub>w</sub> or Well Efficiency represents the proportion of drawdown caused by laminar flow						
Comparison of Observed and Predicted Drawdowns						
Step	Discharge	Discharge (Q)	Observed	Predicted		Apparent
(60 min steps)	(L/s)	(m <sup>3</sup> /d)	Corrected	Drawdown	s/Q	Efficiency
			Drawdown	(metres)		(E <sub>w</sub> )
			(metres)			(%)
1	50	4320	3.24	3.30	0.00075	62.74
2	70	6048	5.355	5.31	0.000885	54.60
3	90	7776	7.93	7.71	0.00102	48.33
4	110	9504	10.31	10.51	0.001085	43.36

**WB08YJSE001 Step Discharge Pumping Test (continuous)**

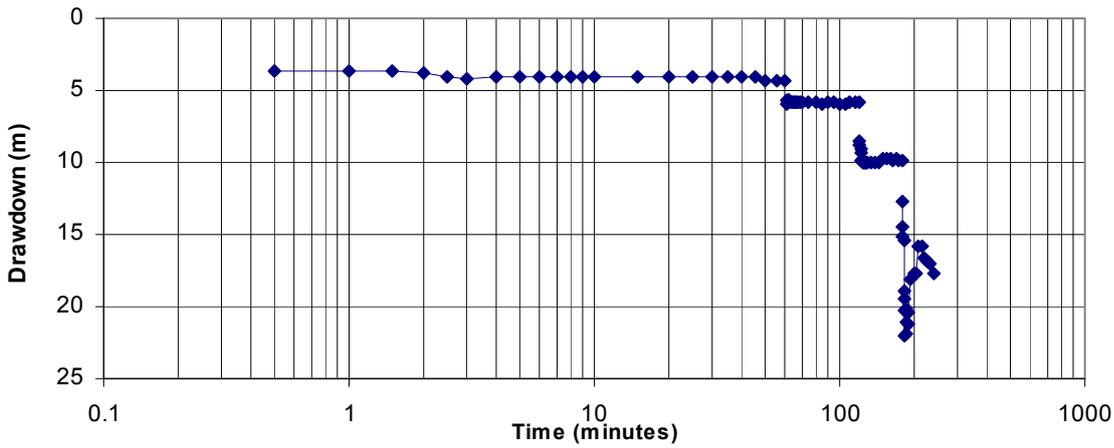


**Analytical Plot of s/Q vs Q**

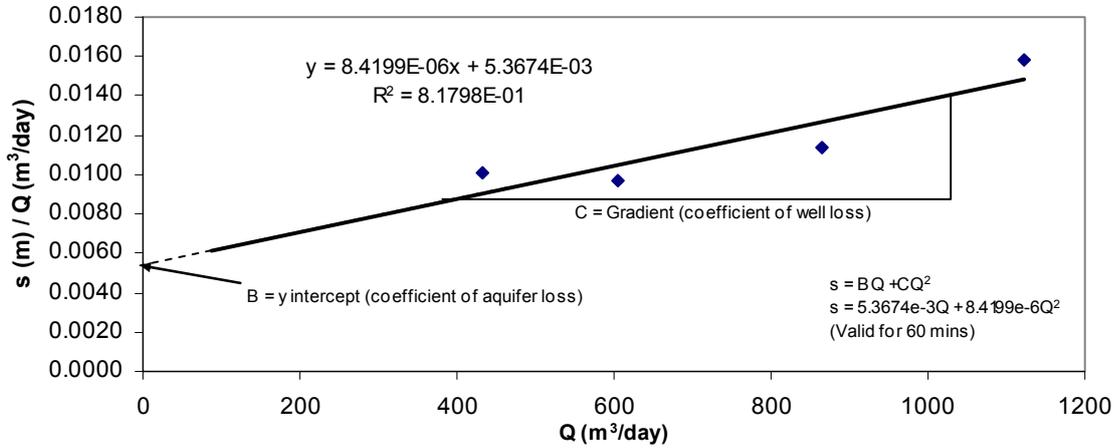


$s_{w(Q)} = BQ_n + CQ^n$ (Rorabaugh's equation)						
Where, B	= Intercept with y axis (coefficient of aquifer loss or laminar flow)					
C	= Gradient (coefficient of turbulent flow loss or apparent well loss)					
s	= Drawdown in the borehole					
P	= Value determined using Rorabaugh's method of superposition					
Components of the equation $BQ$ and $CQ^2$ are termed the aquifer loss and apparent well loss respectively. They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^2)} \times 100$						
Ew or Well Efficiency represents the proportion of drawdown caused by laminar flow						
Comparison of Observed and Predicted Drawdowns						
Step	Discharge	Discharge (Q)	Observed	Predicted	s/Q	Apparent Efficiency (Ew)
			Corrected Drawdown (s)	Drawdown		
(60 min steps)	(L/s)	(m3/d)	(metres)	(metres)		%
1	43	3715	9.795	9.69	0.002636	21.68
2	53	4579	15.02	14.12	0.00328	18.34
3	65	5616	17.64	20.52	0.003141	15.48
4	80	6912	31.61	30.18	0.004573	12.95

**WB08YJSE002 Step Discharge Pumping Test (continuous)**



**Analytical Plot of s/Q vs Q**



$s_{w(n)} = BQ_n + CQ_n^P$ (Rorabaugh's equation)						
Where, B = Intercept with y axis (coefficient of aquifer loss or laminar flow)						
C = Gradient (coefficient of turbulent flow loss or apparent well loss)						
s = Drawdown in the borehole						
P = Value determined using Rorabaugh's method of superposition						
Components of the equation BQ and CQ <sup>2</sup> are termed the aquifer loss and apparent well loss respectively.						
They give an indication of the proportion of total drawdown caused by laminar and turbulent flow.						
$E_w = \frac{BQ}{(BQ + CQ^P)} \times 100$						
E <sub>w</sub> or Well Efficiency represents the proportion of drawdown caused by laminar flow						
<b>Comparison of Observed and Predicted Drawdowns</b>						
			Observed	Predicted		Apparent
Step	Discharge	Discharge (Q)	Corrected	Drawdown	s/Q	Efficiency (E <sub>w</sub> )
(60 min steps)	(L/s)	(m3/d)	Drawdown (s)	(metres)		%
			(metres)			
1	5	432	4.35	3.89	0.010069	59.61
2	7	605	5.85	6.33	0.009673	51.31
3	10	864	9.83	10.92	0.011377	42.46
4	13	1123	17.76	16.65	0.015812	36.21

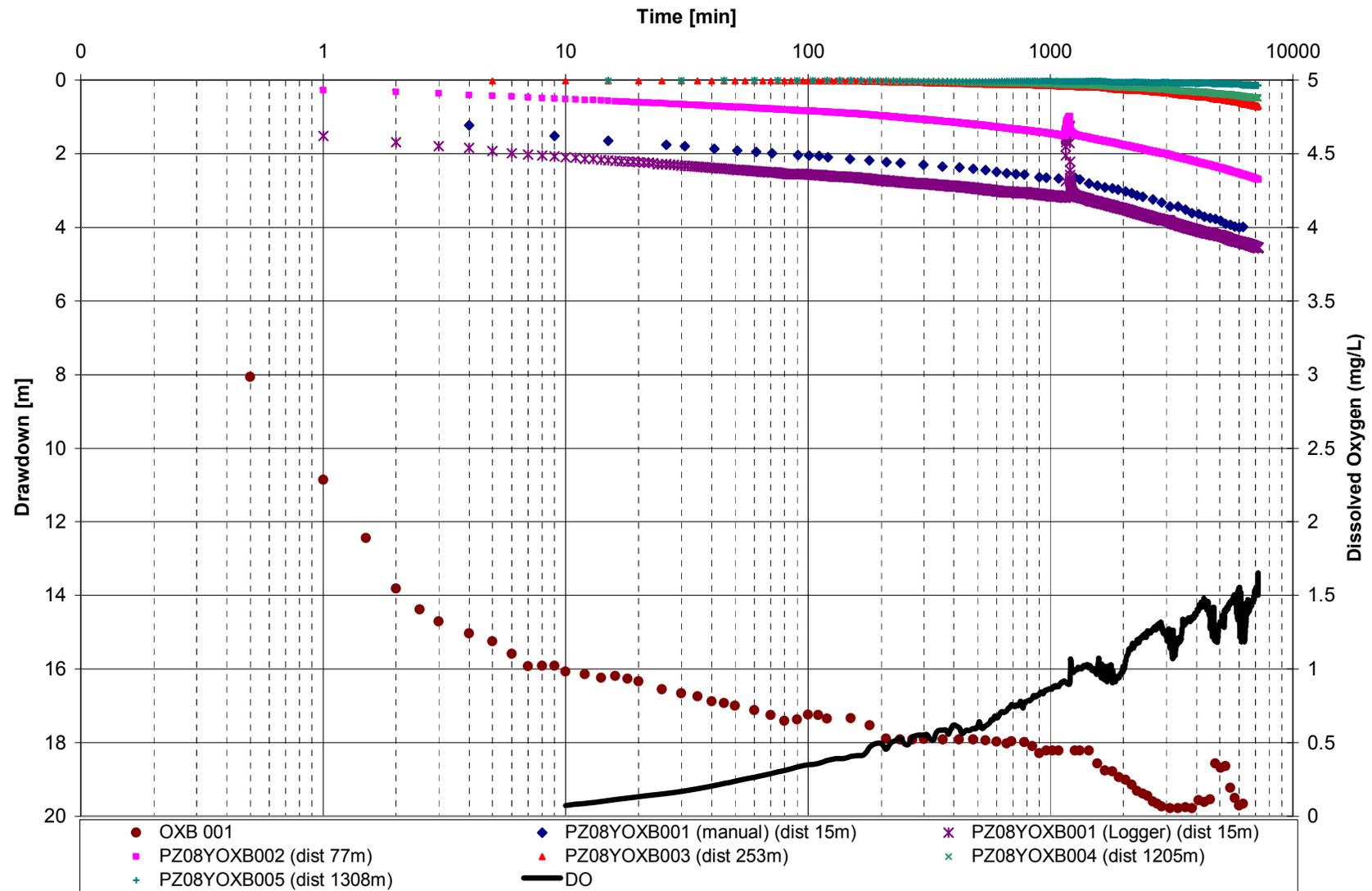
## Appendix 5: Test pumping results

### WB09YOXB001 Aquifer Parameters

Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>v</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB09YOXB001	CID	Cooper-Jacob	0	37.79	57.66	-	-	1300	18.5	350
		Logan				-	-	364	5.2	
		Neuman				-	-	369	6.2	
PZ08YOXB001	CID	Neuman	10	10.95	10.21	8.22x10 <sup>-4</sup>	5.2x10 <sup>-1</sup>	519	8.65	600
		Cooper-Jacob (Middle)				1.45x10 <sup>-2</sup>	-	1880	28	
		Cooper-Jacob (Late)				-	-	516	7.7	
		Theis				2.33x10 <sup>-1</sup>	-	923	13.7	
PZ08YOXB002	CID	Neuman (manual)	108	10.38	6.293	3.43x10 <sup>-4</sup>	2.04x10 <sup>-3</sup>	1360	21	1500
		Cooper-Jacob (Middle)				3.74x10 <sup>-2</sup>	-	2130	31.9	
		Cooper-Jacob Late)				1.61x10 <sup>-1</sup>	-	630	9.4	
		Theis				1.65x10 <sup>-2</sup>	-	2600	38	
PZ08YOXB003	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman (manual)	175	18.99	0.713	8.53x10 <sup>-4</sup>	8.53x10 <sup>-2</sup>	3270	48.9	3500
		Cooper-Jacob (Middle)				5.73x10 <sup>-2</sup>	-	5380	80.4	

		Cooper-Jacob Late)				$9.69 \times 10^{-1}$	-	1220	18.3	
		Theis				$6.56 \times 10^{-1}$	-	2920	43.5	
PZ08YOXB004	CID	Neuman	1205	27.55	0.488	$6.84 \times 10^{-4}$	$2.31 \times 10^{-1}$	2920	43.5	3000
		Cooper-Jacob (Middle)				$5.31 \times 10^{-2}$	-	7250	108	
		Cooper-Jacob Late)				$8.88 \times 10^{-2}$	-	1910	28.8	
		Theis				$6.56 \times 10^{-1}$	-	4620	69	
PZ08YOXB005	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	1308	18.16	0.143	$8.59 \times 10^{-4}$	$3.52 \times 10^{-2}$	2920	61.5	3500
		Cooper-Jacob				$6.24 \times 10^{-2}$	-	9710	144	
		Theis				$5.85 \times 10^{-3}$	-	8230	122	

### WB09YOXB001 Constant Rate Test (Semi-Log)



# WB08YJSW001 Recovery Test

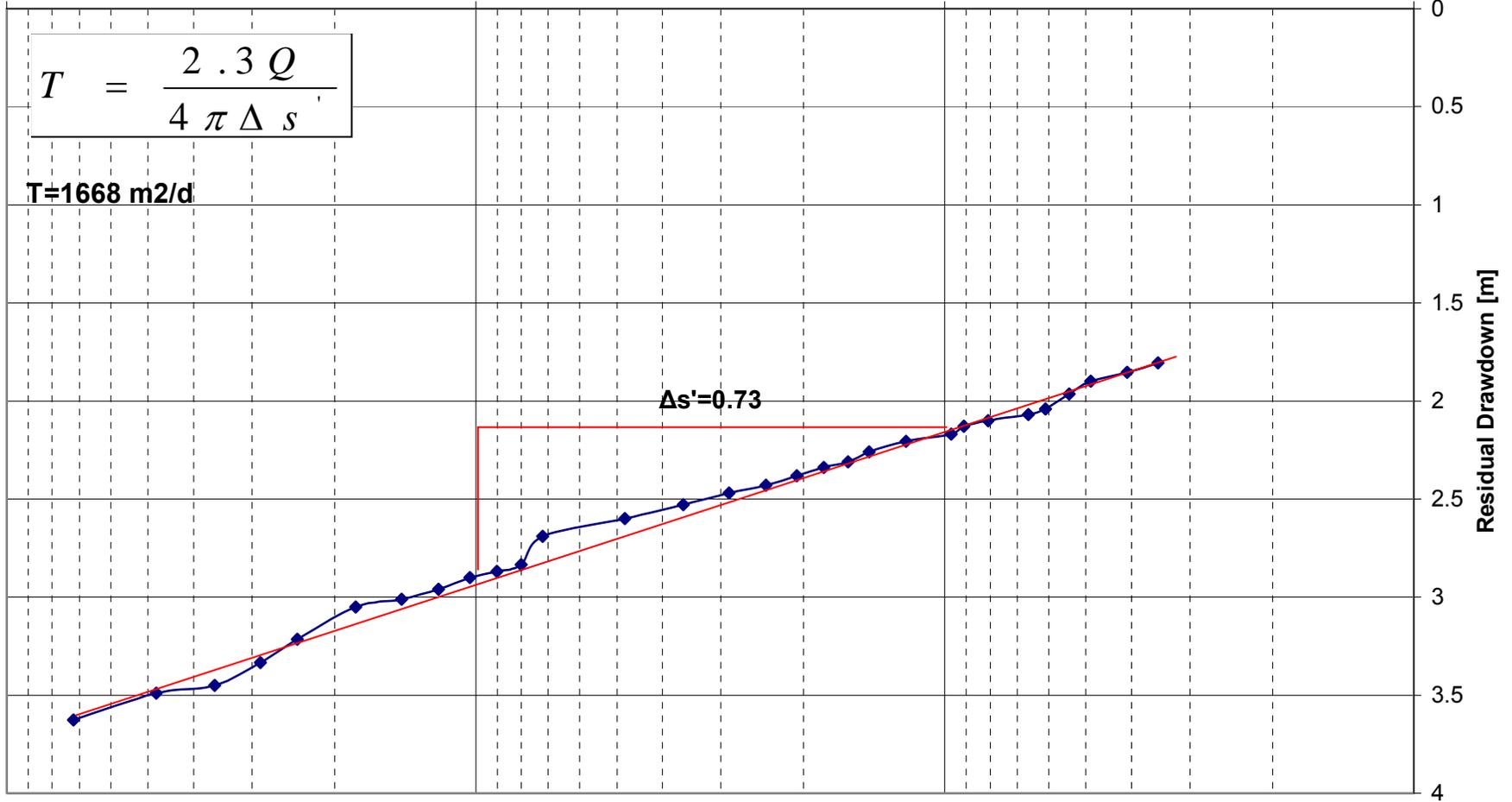
$t/t'$  [-]

10000.0

1000.0

100.0

10.0

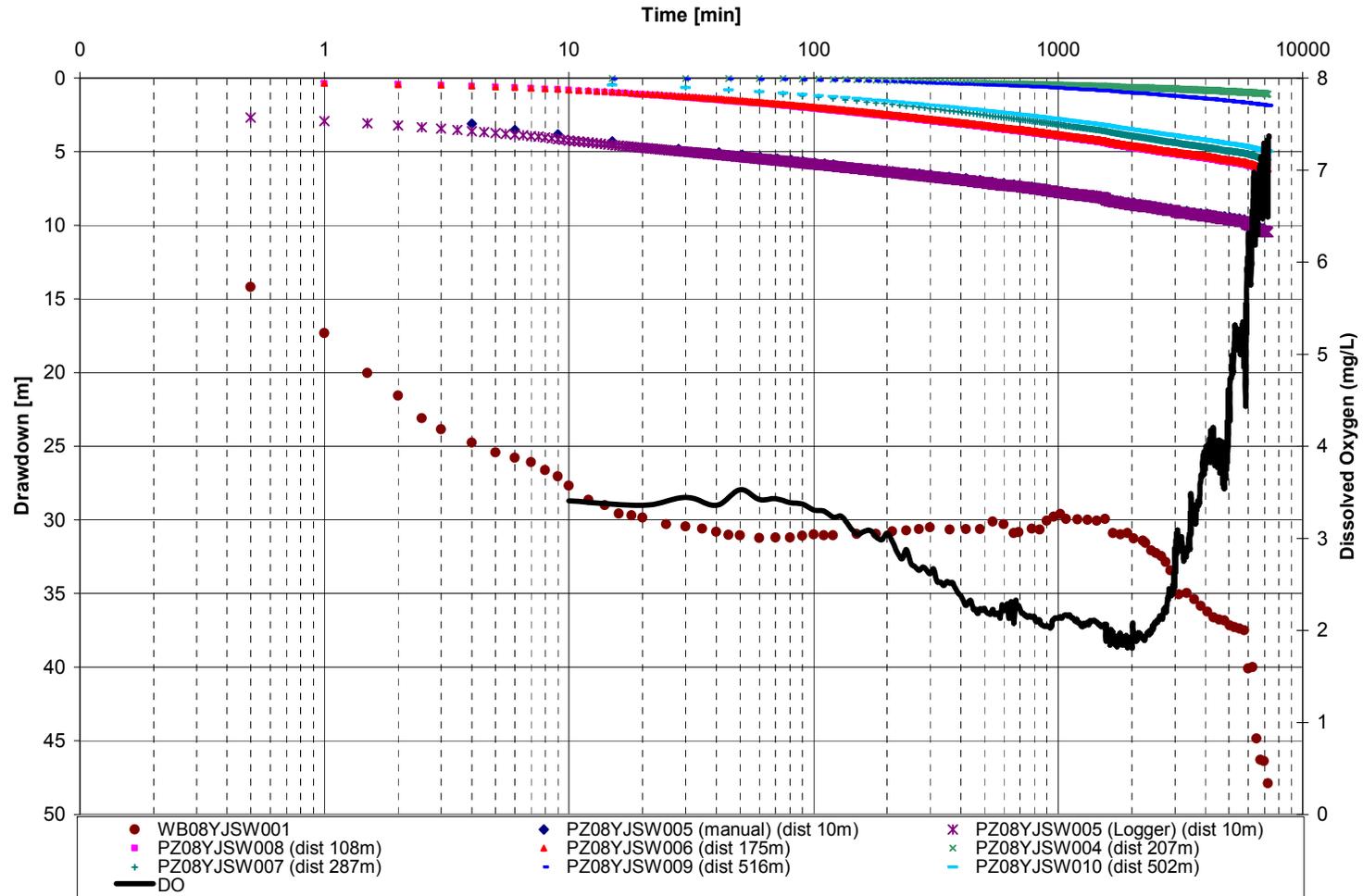


WB08YJSW001 Aquifer Parameters

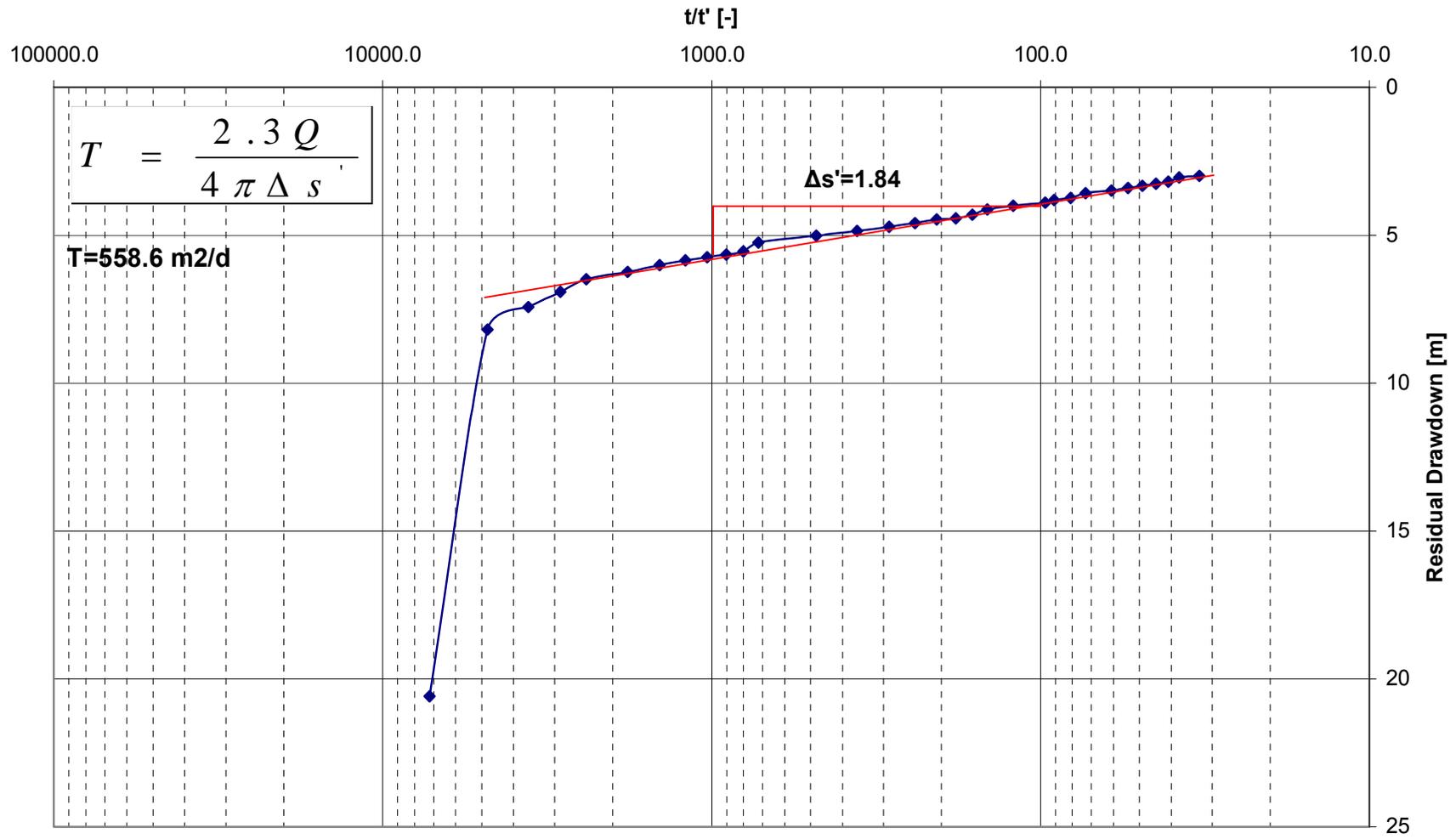
Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>y</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB08YJSW001	CID	Cooper-Jacob	0	11.15	47.88	-	-	558	1.3	250
		Logan				-	-	250	3.57	
		Neuman (ATP)				-	-	223	3.18	
PZ08YJSW005	CID	Neuman (manual)	10	10.95	10.21	9.1x10 <sup>-4</sup>	6.2x10 <sup>-3</sup>	319	4.9	350
		Cooper-Jacob (All)				2.17x10 <sup>-1</sup>	-	482	7.2	
		Theis				3.56x10 <sup>-1</sup>	-	398	5.94	
PZ08YJSW008	CID	Neuman (manual)	108	10.35	6.29	1.26x10 <sup>-5</sup>	5.25x10 <sup>-4</sup>	398	5.94	450
		Cooper-Jacob (Middle)				7.75x10 <sup>-1</sup>	-	555	8.28	
		Cooper-Jacob Late)				2.21x10 <sup>-1</sup>	-	429	6.41	
		Theis				7.11x10 <sup>-1</sup>	-	891	13.3	
PZ08YJSW006	CID	Neuman	175	18.99	6.17	2.37x10 <sup>-4</sup>	2.37x10 <sup>-1</sup>	455	6.50	460
		Cooper-Jacob (Middle)				2.32x10 <sup>-1</sup>	-	445	6.36	
		Cooper-Jacob Late)				4.80x10 <sup>-2</sup>	-	655	9.36	
		Theis				5.74x10 <sup>-7</sup>	-	486	6.94	
PZ08YJSW007	CID	Neuman	287	20.33	5.54	3.85x10 <sup>-5</sup>	3.85x10 <sup>-1</sup>	501	7.48	500
		Cooper				2.08x10 <sup>-1</sup>	-	405	7.30	

		Jacob (Middle)								
		Cooper-Jacob Late)				$6.39 \times 10^{-1}$	-	401	5.99	
		Theis				$2.83 \times 10^{-1}$	-	501	7.48	
PZ08YJSW010	CID	Neuman	502	12.66	4.99	$1.0 \times 10^{-5}$	$1.0 \times 10^{-1}$	631	9.42	600
		Cooper-Jacob (Middle)				$1.41 \times 10^{-1}$	-	562	8.4	
		Cooper-Jacob Late)				$2.63 \times 10^{-1}$	-	431	6.44	
		Theis				$1.0 \times 10^{-1}$	-	562	8.39	
PZ08YJSW004	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman (manual)	207	8.10	1.08	$5.77 \times 10^{-3}$	$1.01 \times 10^{-3}$	1990	29.5	1900
		Cooper-Jacob (All)				$1.21 \times 10^{-2}$	-	1970	29.4	
		Theis				$1.41 \times 10^{-2}$	-	1770	26.5	
PZ08YJSW009	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	516	11.95	1.88	$1.5 \times 10^{-4}$	$1.5 \times 10^{-2}$	1250	18.7	1250
		Cooper-Jacob (Middle)				$1.19 \times 10^{-1}$	-	1333	19.8	
		Cooper-Jacob Late)				$2.24 \times 10^{-1}$	-	754	11.2	
		Theis				$1.26 \times 10^{-1}$	-	1410	21	

### WB08YJSW001 Constant Rate Test (Semi-Log)



### WB08YJSW001 Recovery Test

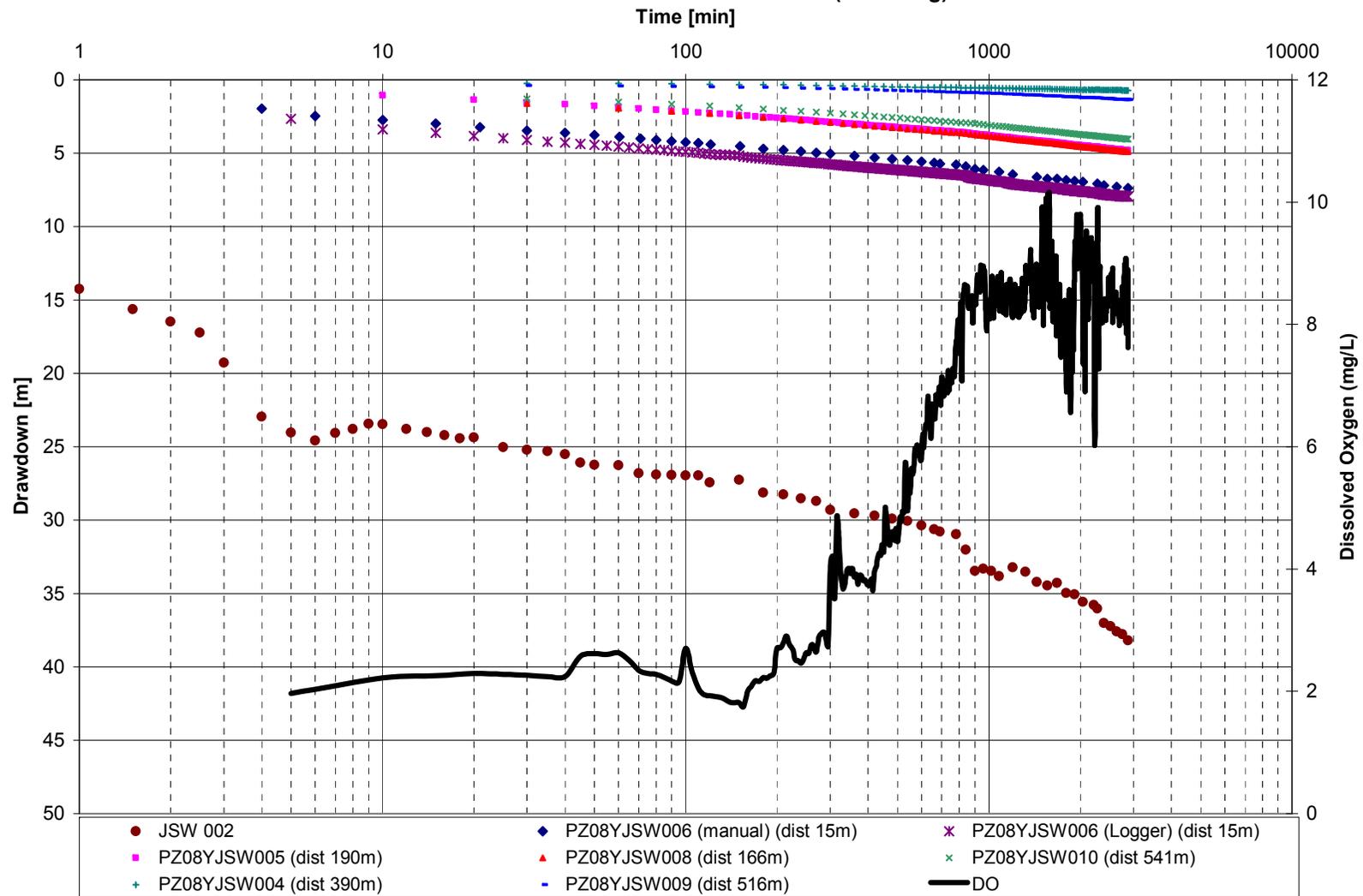


WB08YJSW002 Aquifer Parameters

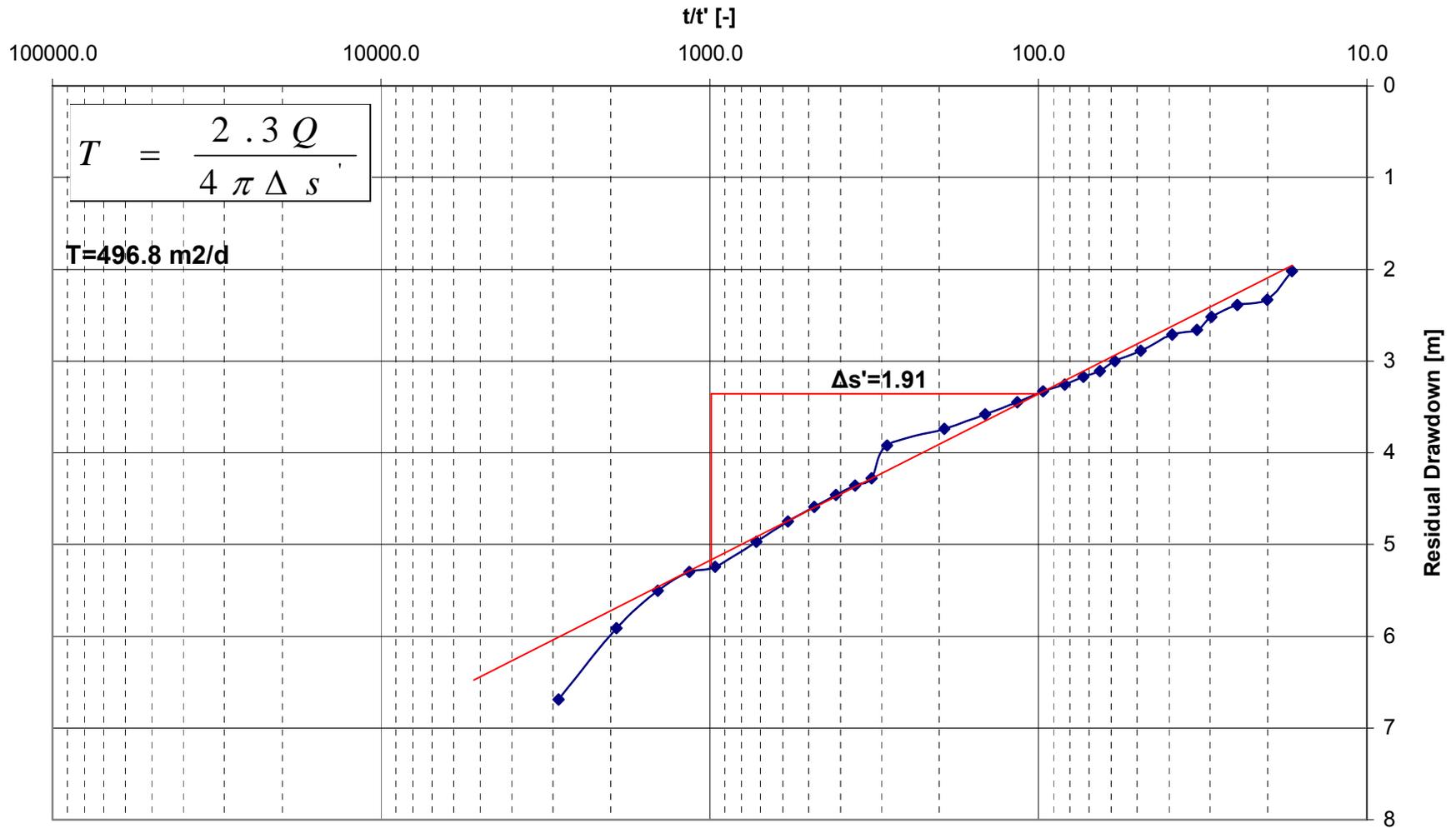
Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>y</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB08YJSW002	CID	Cooper-Jacob (Recovery)	0	20.78	38.19	-	-	497	8.3	250
		Logan				-	-	241	4	
		Neuman (ATP)				-	-	130	2.2	
PZ08YJSW006	CID	Neuman	15	19.62	7.95	7.33x10 <sup>-4</sup>	7.33x10 <sup>-2</sup>	292	4.3	350
		Cooper-Jacob (All)				9.97x10 <sup>-1</sup>	-	527	7.87	
		Theis				2.33x10 <sup>0</sup>	-	412	6.15	
PZ08YJSW005	CID	Neuman	190	11.11	4.8	1.44x10 <sup>-4</sup>	1.62x10 <sup>-1</sup>	589	9.06	600
		Cooper-Jacob (Middle)				1.62x10 <sup>-1</sup>	-	607	9.06	
		Cooper-Jacob Late)				4.55x10 <sup>-1</sup>	-	442	6.6	
		Theis				1.16x10 <sup>-1</sup>	-	653	9.75	
PZ08YJSW008	CID	Neuman (manual)	166	11.02	4.9	2.67x10 <sup>-4</sup>	1.19x10 <sup>-4</sup>	260	3.88	460
		Cooper-Jacob (Middle)				2.41x10 <sup>-1</sup>	-	577	8.61	
		Cooper-Jacob Late)				5.78x10 <sup>-1</sup>	-	438	6.54	
		Theis				2.33x10 <sup>-1</sup>	-	519	7.75	
PZ08YJSW010	CID	Neuman	541	13.45	4.04	3.99x10 <sup>-2</sup>	1.0x10 <sup>-1</sup>	462	6.9	500
		Cooper-				4.87x10 <sup>-2</sup>	-	616	9.19	

		(Middle)								
		Cooper-Jacob Late)				1.04x10 <sup>-1</sup>	-	471	7.04	
		Theis				5.21x10 <sup>-2</sup>	-	519	7.75	
PZ08YJSW004	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	390	8.68	0.7	4.84x10 <sup>-1</sup>	9.66x10 <sup>-1</sup>	2310	34.6	3000
		Cooper-Jacob (All)				5.6x10 <sup>-1</sup>	-	3250	48.6	
		Theis				5.21x10 <sup>-1</sup>	-	3270	48.9	
PZ08YJSW009	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	516	12.77	1.35	2.76x10 <sup>-2</sup>	1.55x10 <sup>-1</sup>	462	6.9	550
		Cooper-Jacob (All)				5.99x10 <sup>-2</sup>	-	578	8.63	
		Theis				4.14x10 <sup>-2</sup>	-	653	9.75	

### WB08YJSW002 Constant Rate Test (Semi-Log)



### WB08YJSW002 Recovery Test

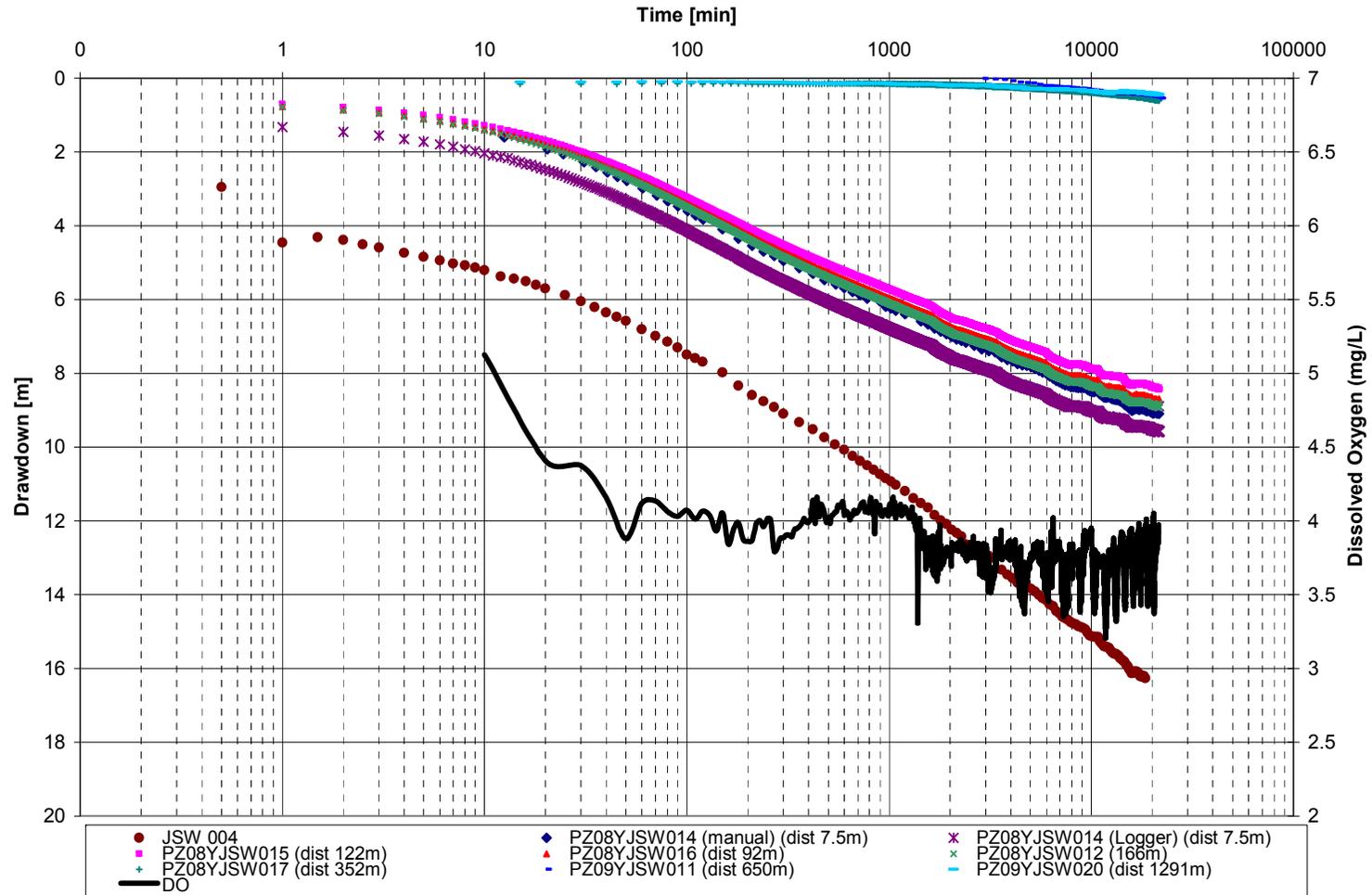


WB09YJSW004 Aquifer Parameters

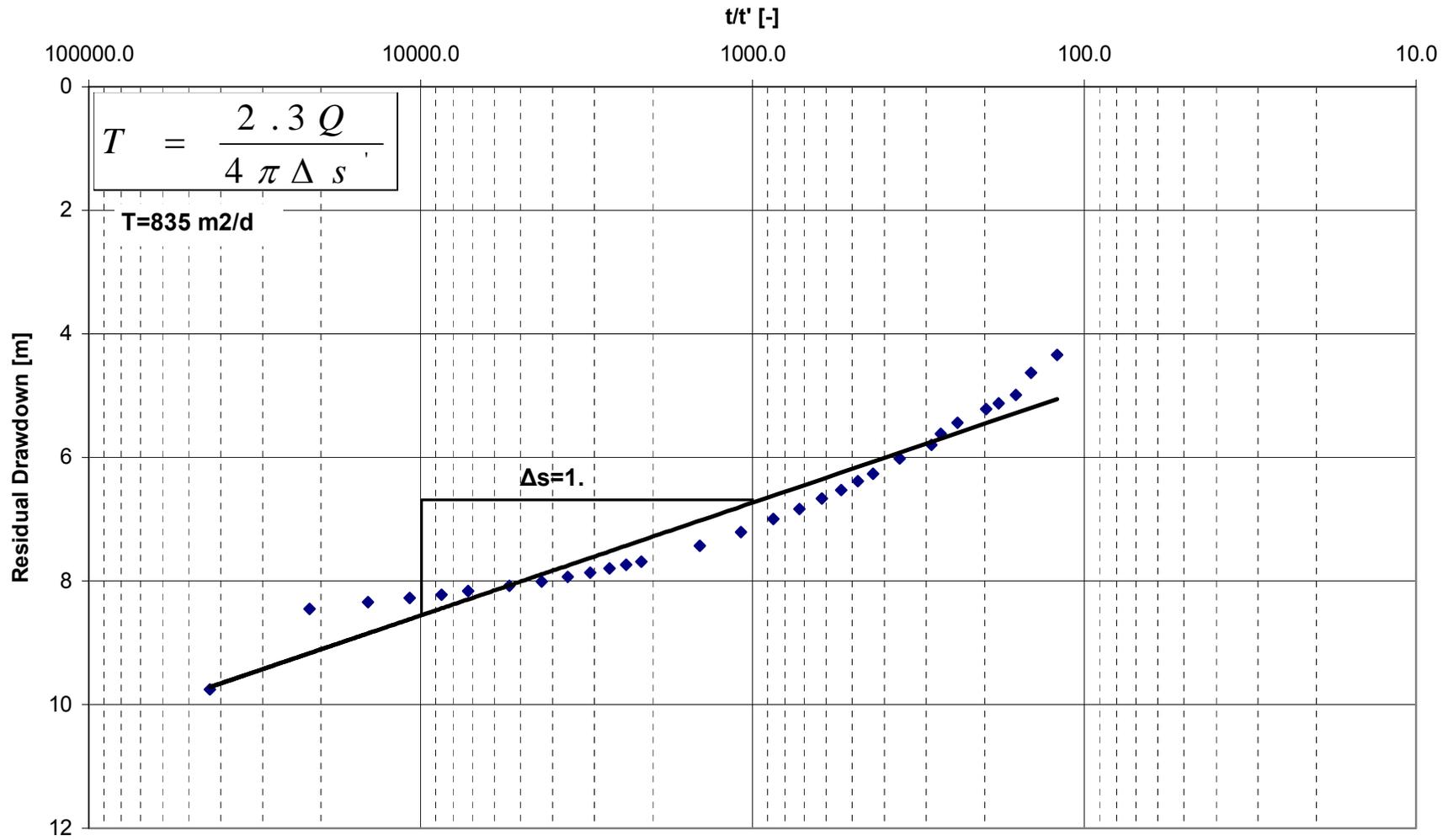
Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>v</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB09YJSW004	CID	Cooper-Jacob	0	13.08	16.53	-	-	834	11.9	500
		Logan				-	-	684	9.7	
		Neuman (ATP)				-	-	231	3.3	
PZ08YJSW005	CID	Neuman (manual)	7.5	12.6	9.1	1.66x10 <sup>-2</sup>	3.87x10 <sup>-2</sup>	582	8.68	600
		Cooper-Jacob (All)				7.16x10 <sup>-1</sup>	-	627	9.35	
		Theis				9.27x10 <sup>-1</sup>	-	653	9.74	
PZ08YJSW015	CID	Neuman	108	10.35	6.29	4.98x10 <sup>-5</sup>	1.98x10 <sup>-1</sup>	518	7.74	550
		Cooper-Jacob (All)				4.1x10 <sup>-1</sup>	-	631	9.43	
		Theis				4.14x10 <sup>-1</sup>	-	582	8.68	
PZ08YJSW016	CID	Neuman	92	15.56	8.69	9.76x10 <sup>-5</sup>	9.76x10 <sup>-2</sup>	462	6.9	500
		Cooper-Jacob (All)				6.99x10 <sup>-1</sup>	-	601	8.97	
		Theis				4.64x10 <sup>-1</sup>	-	732	10.9	
PZ08YJSW012	CID	Neuman	166	15.72	8.81	2.12x10 <sup>-5</sup>	2.12x10 <sup>-1</sup>	462	6.9	500
		Cooper-Jacob (Middle)				2.27x10 <sup>-1</sup>	-	562	8.38	
		Cooper-Jacob (Late)				8.59x10 <sup>-2</sup>	-	686	10.2	
		Theis				1.46x10 <sup>-1</sup>	-	582	8.68	
PZ08YJSW017	ALLUVIAL/INSITU WEATHERERD	Neuman	502	12.66	4.99	6.32x10 <sup>-3</sup>	6.32x10 <sup>-2</sup>	4620	69	4500

	BEDROCK	Cooper-Jacob (All)				$8.2 \times 10^{-1}$	-	3900	58.3	
		Theis				$8.26 \times 10^{-1}$	-	4120	61.5	
PZ09YJSW011	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	650	7.56	0.538	$5.51 \times 10^{-4}$	$5.51 \times 10^{-3}$	1460	21.8	1500
		Cooper-Jacob (All)				-	-	2820	42	
		Theis				-	-	1160	17.3	
PZ09YJSW020	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	1291	5.93	0.446	$9.89 \times 10^{-5}$	$9.89 \times 10^{-3}$	9220	137	10000
		Cooper-Jacob (Middle)				$4.71 \times 10^{-1}$	-	14800	221	
		Cooper-Jacob Late)				-	-	6760	100	
		Theis				$7.36 \times 10^{-1}$	-	11600	173	

### WB09YJSW004 Constant Rate Test (Semi-Log)



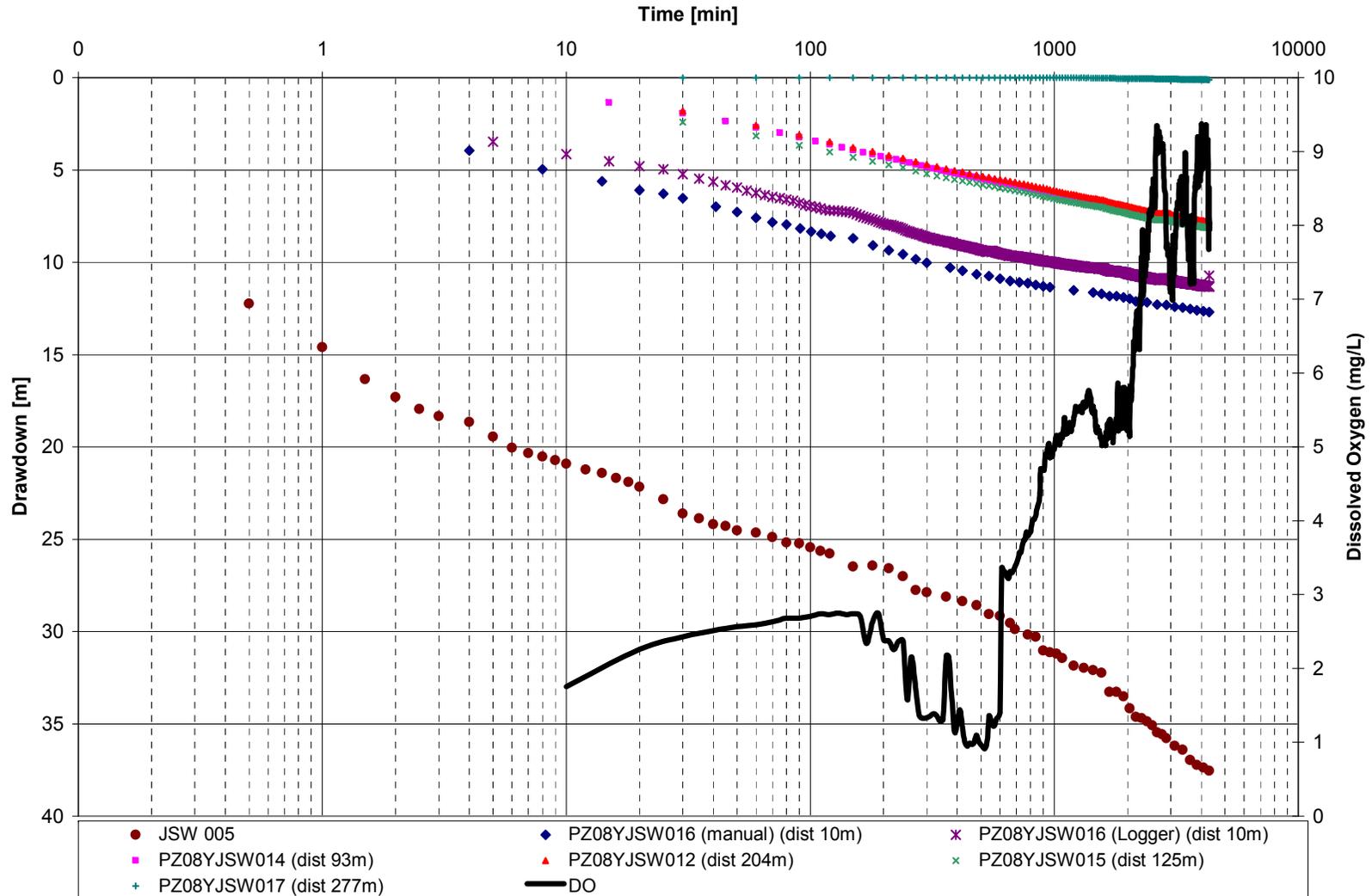
### WB09YJSW004 Recovery Test



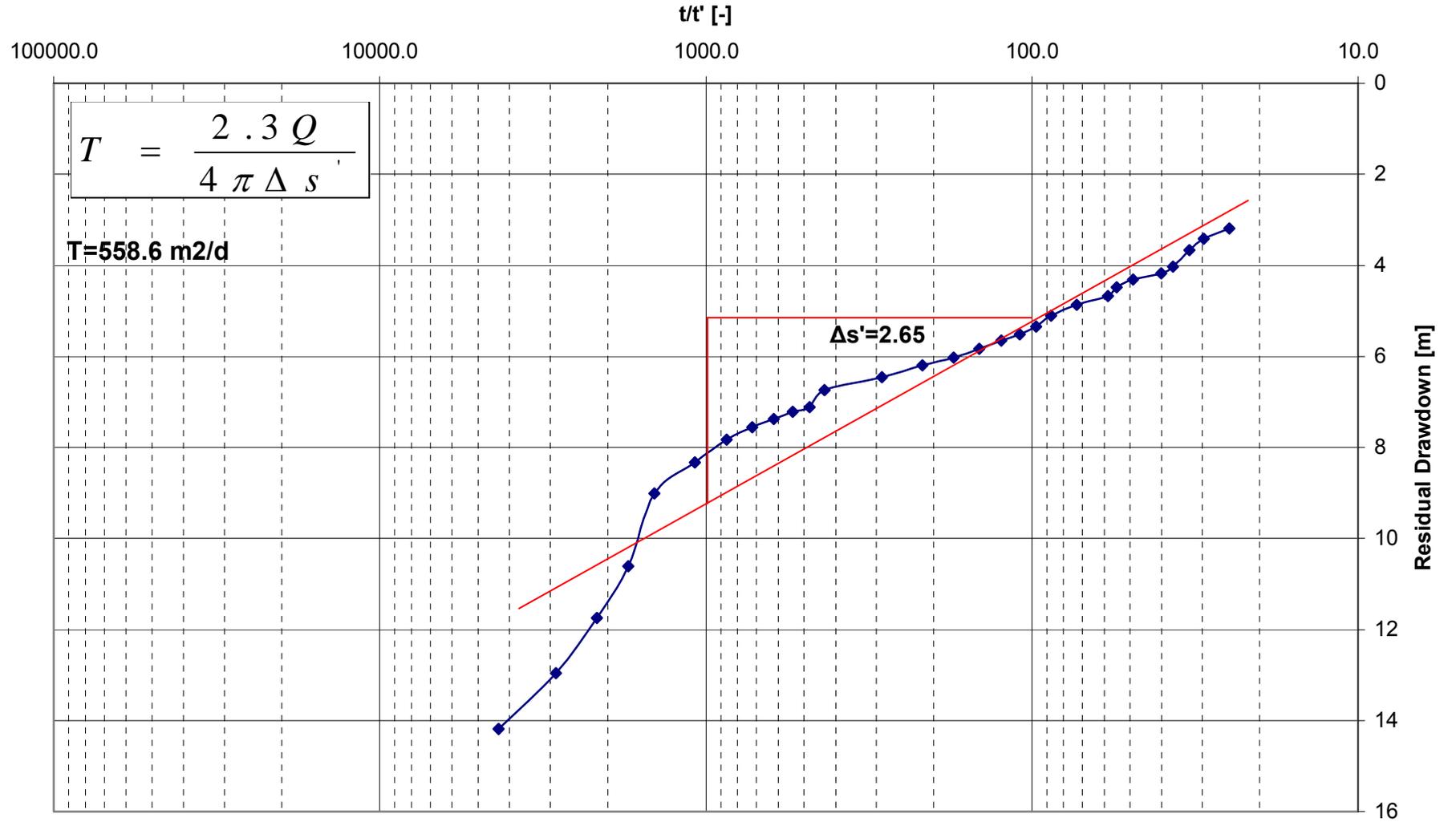
WB09YJSW005 Aquifer Parameters

Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>y</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB09YJSW005	CID	Theis Recovery (manual)	0	17.82	37.55	-	-	597	9.95	400
		Logan (manual)				-	-	406	6.76	
		Neuman				-	-	365	6.08	
PZ08YJSW016	CID	Neuman (manual)	10	16.79	10.7	7.0x10 <sup>-3</sup>	3.88x10 <sup>-2</sup>	307	4.58	400
		Cooper-Jacob (All)				-	-	615	9.18	
		Theis				-	-	486	6.9	
PZ08YJSW014	CID	Neuman	93	12.2	7.94	7.11x10 <sup>-1</sup>	1.96x10 <sup>-1</sup>	612	9.14	600
		Cooper-Jacob (All)				8.32x10 <sup>-1</sup>	-	567	8.46	
		Theis				9.75x10 <sup>-1</sup>	-	546	8.15	
PZ08YJSW012	CID	Neuman	204	14.74	7.78	3.31x10 <sup>-1</sup>	2.95x10 <sup>-1</sup>	546	8.15	550
		Cooper-Jacob (All)				1.94x10 <sup>-1</sup>	-	569	8.5	
		Theis				2.45x10 <sup>-1</sup>	-	546	8.15	
PZ08YJSW015	CID	Neuman	125	15.55	7.98	5.56x10 <sup>-1</sup>	2.0x10 <sup>-1</sup>	546	8.15	600
		Cooper-Jacob (All)				2.89x10 <sup>-1</sup>	-	598	8.93	
		Theis				3.46x10 <sup>-1</sup>	-	612	9.14	

### WB09YJSW005 Constant Rate Test (Semi-Log)



### WB09YJSW005 Recovery Test

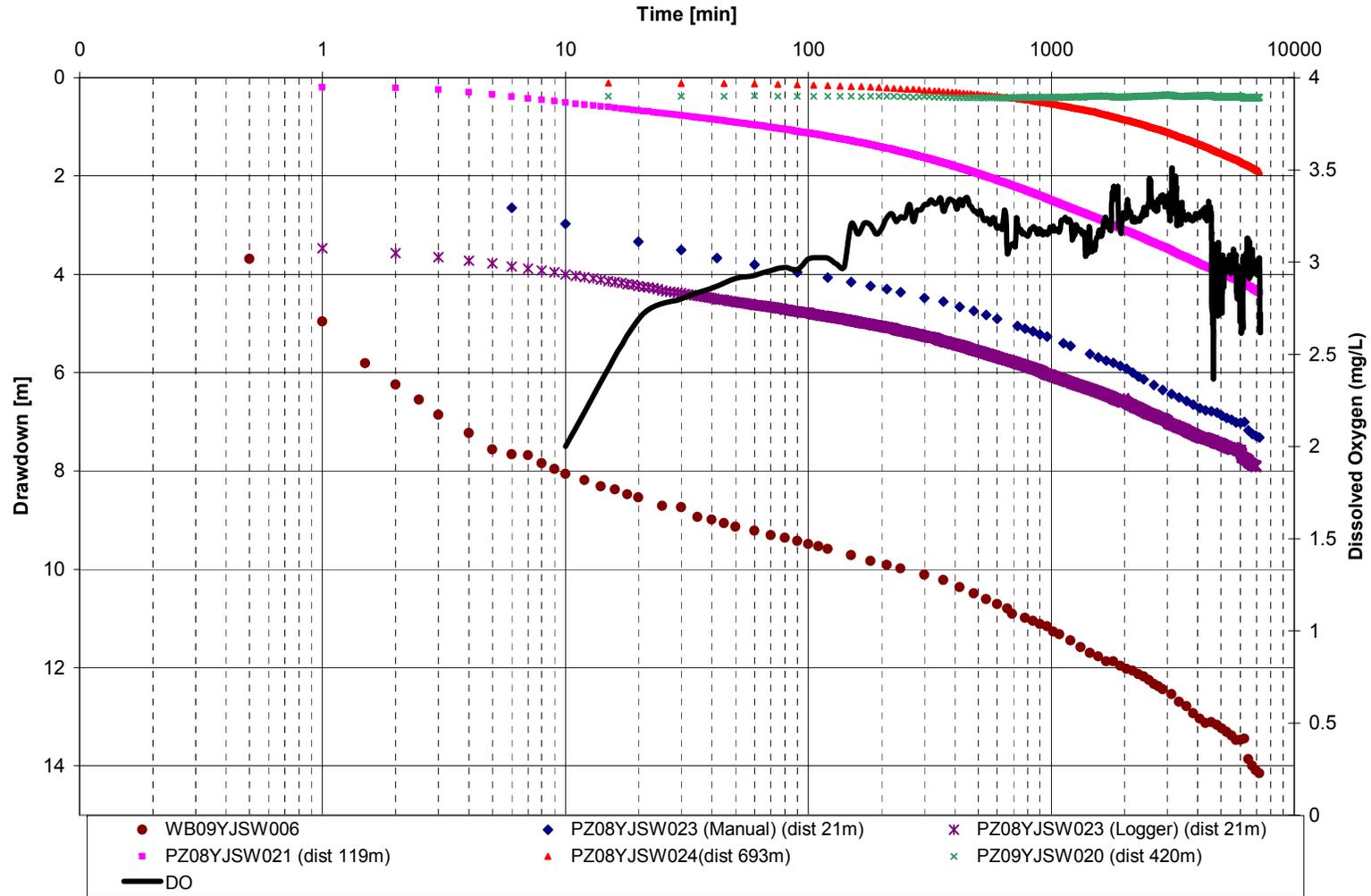


WB09YJSW006 Aquifer Parameters

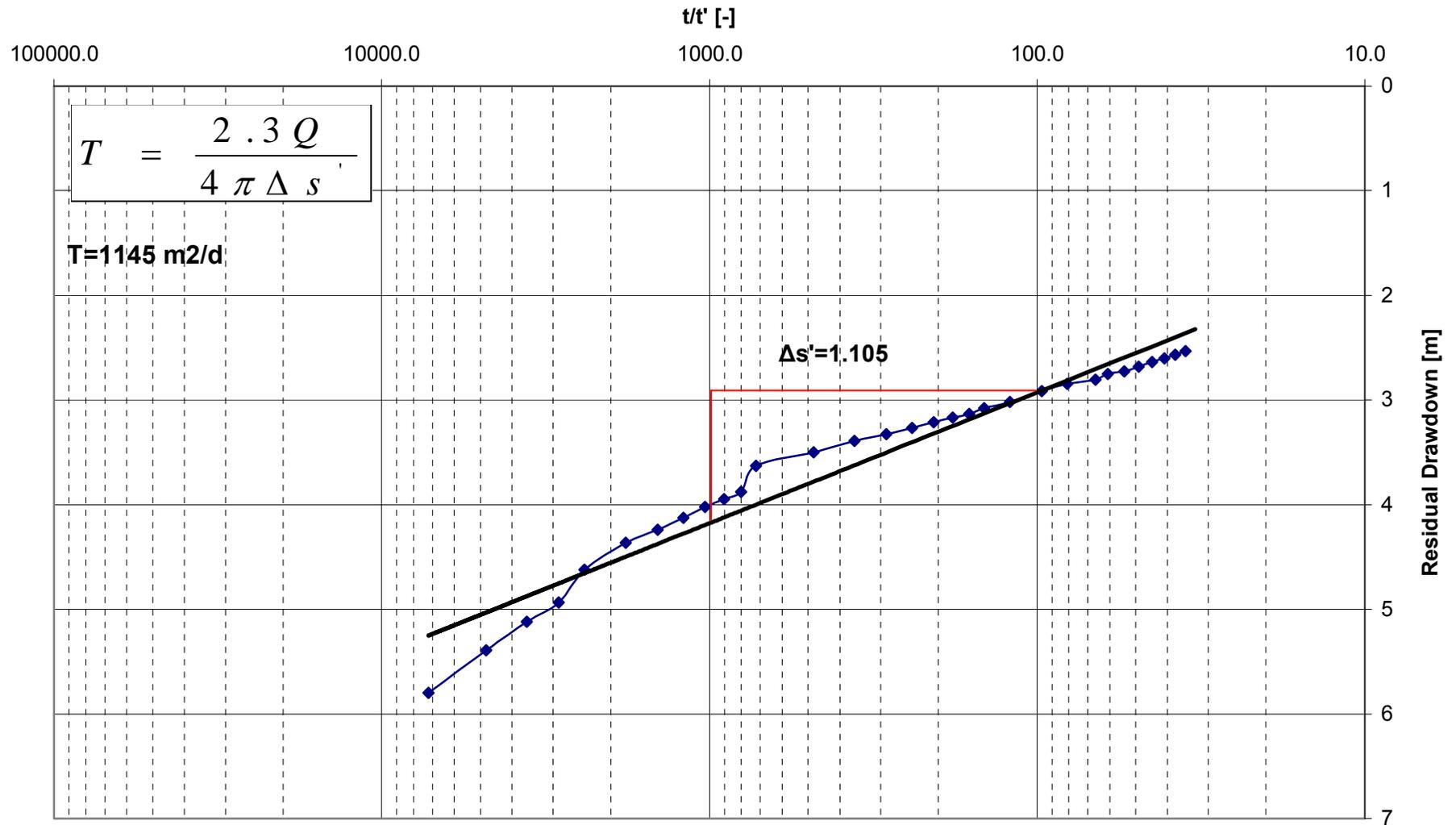
Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Estimated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>y</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB09YJSW006	CID	Theis Recovery (manual)	0	12.2	14.15	-	-	1145	16.3	450
		Logan (manual)				-	-	660	3.57	
		Neuman				-	-	389	5.56	
PZ08YJSW023	CID	Neuman (manual)	21	10.88	7.32	6.92x10 <sup>-3</sup>	4.99x10 <sup>-3</sup>	550	4.86	550
		Cooper-Jacob (Middle)				8.64x10 <sup>-2</sup>	-	1210	18.1	
		Cooper-Jacob Late)				-	-	592	8.83	
		Theis				-	-	776	11.5	
PZ08YJSW021	CID	Neuman	119	10.85	4.37	5.51x10 <sup>-5</sup>	5.51x10 <sup>-1</sup>	1550	23.1	1600
		Cooper-Jacob (Middle)				6.39x10 <sup>-1</sup>	-	1860	27.7	
		Cooper-Jacob Late)				-	-	631	9.4	
		Theis				4.38x10 <sup>-1</sup>	-	2180	32.6	
PZ08YJSW024	CID	Neuman	693	14.97	1.906	2.89x10 <sup>-4</sup>	2.89x10 <sup>-2</sup>	776	11.5	800
		Cooper-Jacob (Middle)				9.37x10 <sup>-1</sup>	-	2130	31.8	
		Cooper-Jacob Late)				-	-	729	10.8	

		Theis				$9.76 \times 10^{-1}$	-	1230	18.3	
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### WB09YJSW006 Constant Rate Test (Semi-Log)



### WB09YJSW006 Recovery Test

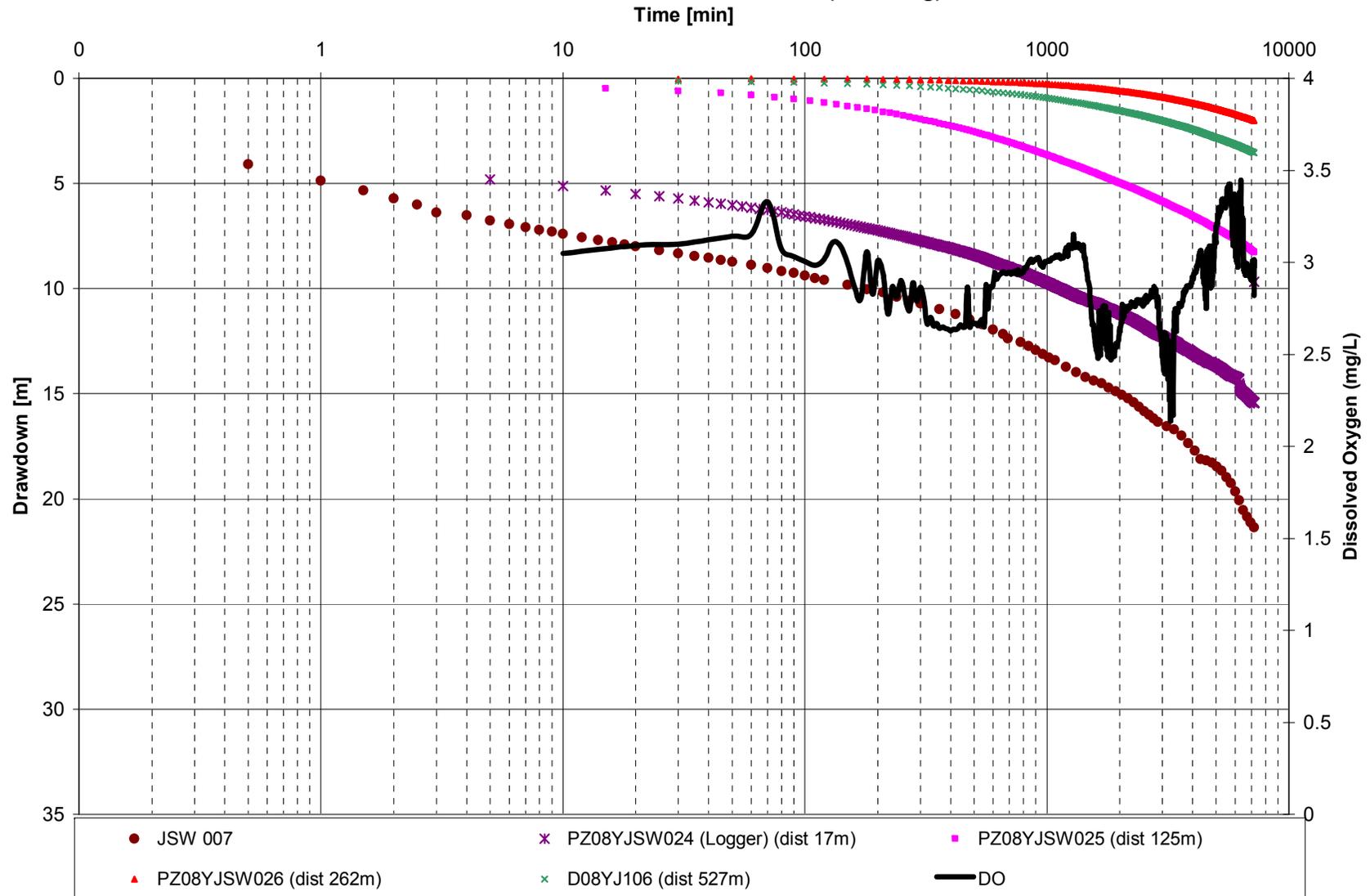


WB09YJSW007 Aquifer Parameters

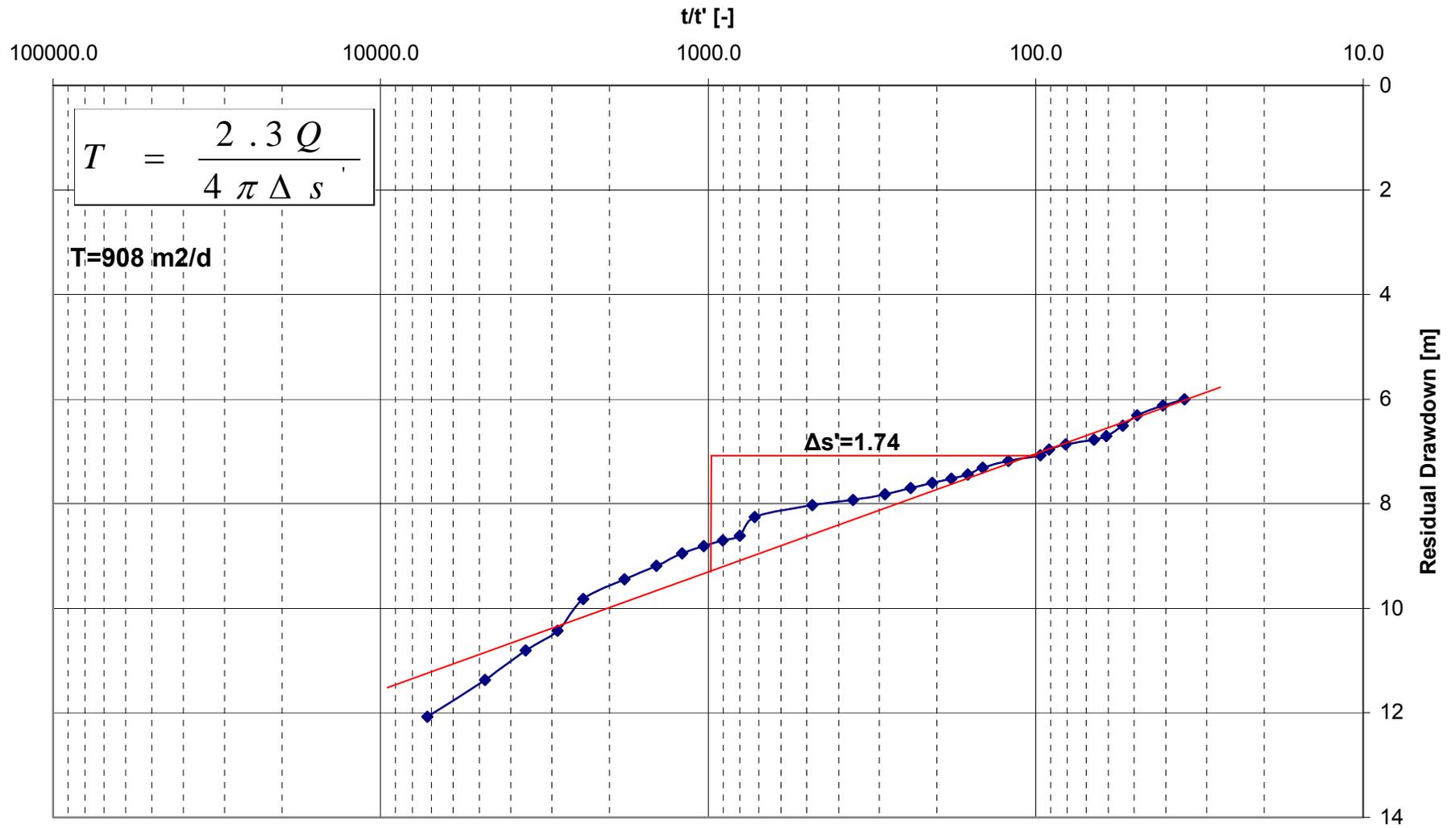
Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>y</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB09YJSW007	CID	Theis Recovery (manual)	0	12.2	14.15	-	-	908	16.3	300
		Logan (manual)				-	-	598	3.57	
		Neuman				-	-	243	5.56	
PZ08YJSW024	CID	Neuman (manual)	17	15.54	15.71	2.97x10 <sup>-3</sup>	9.99x10 <sup>-3</sup>	343	5.3	350
		Cooper-Jacob (Middle)				3.22x10 <sup>-1</sup>	-	488	7.29	
		Cooper-Jacob Late)				-	-	278	4.16	
		Theis				1.09x10 <sup>-1</sup>	-	612	9.14	
PZ08YJSW025	CID	Neuman	119	10.85	4.37	5.56x10 <sup>-4</sup>	5.56x10 <sup>-4</sup>	486	7.26	500
		Cooper-Jacob (Middle)				4.23x10 <sup>-1</sup>	-	524	4.77	
		Cooper-Jacob Late)				-	-	319	7.83	
		Theis				-	-	433	6.47	
PZ08YJSW026	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	693	14.97	1.906	5.04x10 <sup>-3</sup>	1.04x10 <sup>-1</sup>	1720	25.7	1500
		Cooper-Jacob (Middle)				2.34x10 <sup>-1</sup>	-	1970	29.4	
		Cooper-Jacob Late)				2.64x10 <sup>-1</sup>	-	595	8.89	

		Theis				$3.08 \times 10^{-1}$	-	971	14.4	
D08YJ106	CID	Neuman	527	18.68	3.546	$2.79 \times 10^{-4}$	$2.79 \times 10^{-2}$	771	11.5	800
		Cooper-Jacob (Middle)				$1.66 \times 10^{-1}$	-	1110	16.6	
		Cooper-Jacob Late)				-	-	486	7.25	
		Theis				-	-	771	11.5	

### WB09YJSW007 Constant Rate Test (Semi-Log)



### WB09YJSW007 Recovery Test

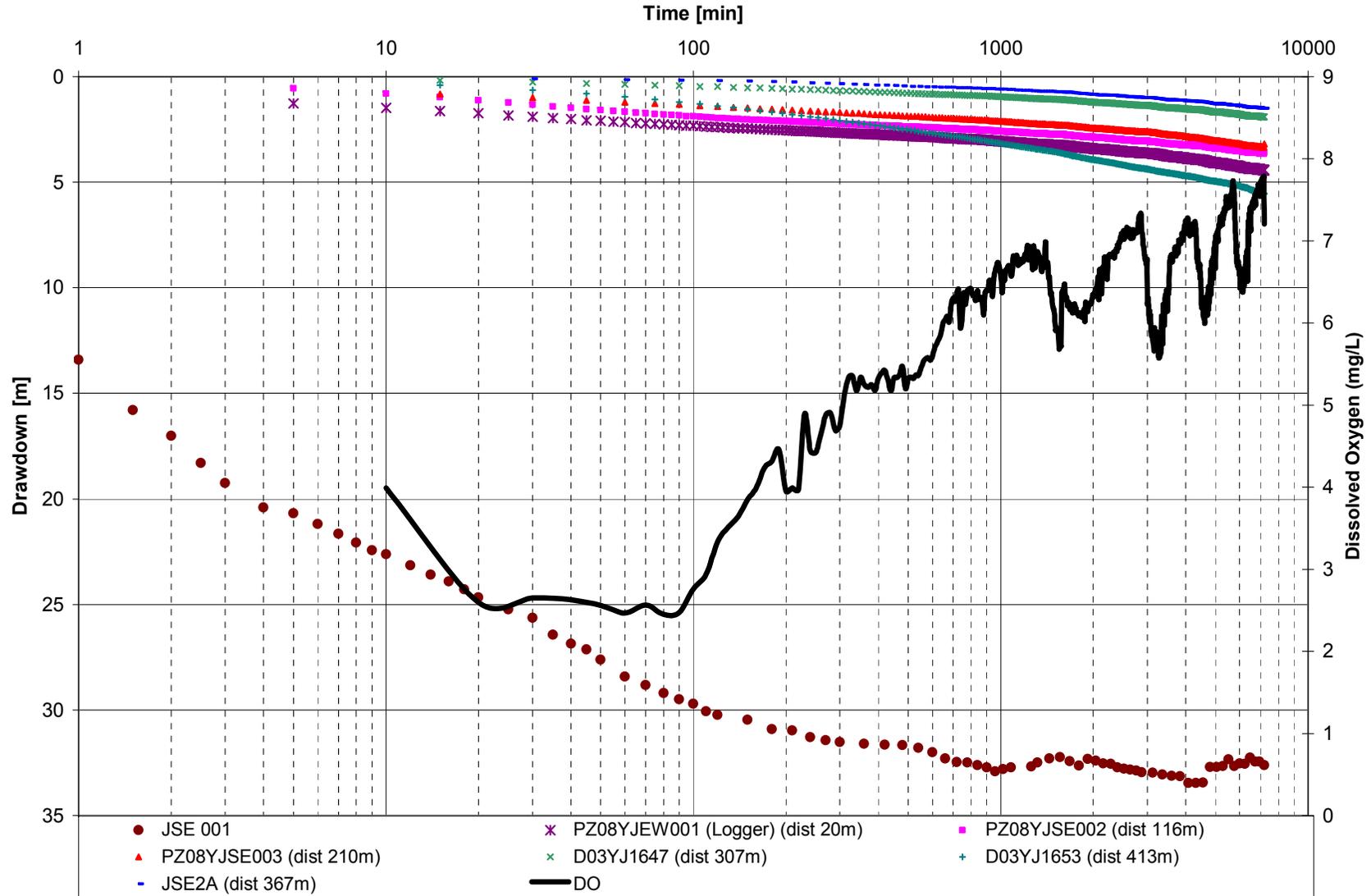


WB08YJSE001 Aquifer Parameters

Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>y</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Adopted T (m <sup>2</sup> /d)
WB08YJSE001	CID	Theis Recovery (manual)	0	25.4	32.62	-	-	1668	25.6	300
		Logan (manual)				-	-	330	5.1	
		Neuman				-	-	236	3.6	
PZ08YJSE001	CID	Neuman (manual)	20	25.16	4.135	4.2x10 <sup>-3</sup>	1.2x10 <sup>-2</sup>	882	13.5	700
		Cooper-Jacob (Middle)				2.2x10 <sup>-1</sup>	-	1590	23.7	
		Cooper-Jacob Late)				9.71x10 <sup>-1</sup>	-	760	11.3	
		Theis				2.66x10 <sup>-1</sup>	-	1490	22.2	
PZ08YJSE002	CID	Neuman	116	23.67	3.598	8.84x10 <sup>-5</sup>	1.84x10 <sup>-1</sup>	941	14	1100
		Cooper-Jacob (Middle)				1.31x10 <sup>-1</sup>	-	1510	22.6	
		Cooper-Jacob Late)				6.75x10 <sup>-1</sup>	-	1150	17.2	
		Theis				4.74x10 <sup>-1</sup>	-	1050	15.7	
PZ08YJSE003	CID	Neuman	210	23.62	3.179	1.3x10 <sup>-4</sup>	2.02x10 <sup>-4</sup>	1058	16.3	1100
		Cooper-Jacob (All)				-	-	1970	29.4	
		Theis				-	-	971	14.4	
D03YJ1647	CID	Neuman	307	24.93	1.895	8.94x10 <sup>-5</sup>	1.94x10 <sup>-1</sup>	1870	28	2000

		Cooper- Jacob (Middle)				$1.01 \times 10^{-1}$	-	2220	33.1	
		Cooper- Jacob Late)				$4.13 \times 10^{-1}$	-	1110	16.7	
		Theis				$5.96 \times 10^{-1}$	-	2360	35.2	

### WB08YJSE001 Constant Rate Test (Semi-Log)



# WB08YJSW001 Recovery Test

$t/t'$  [-]

10000.0

1000.0

100.0

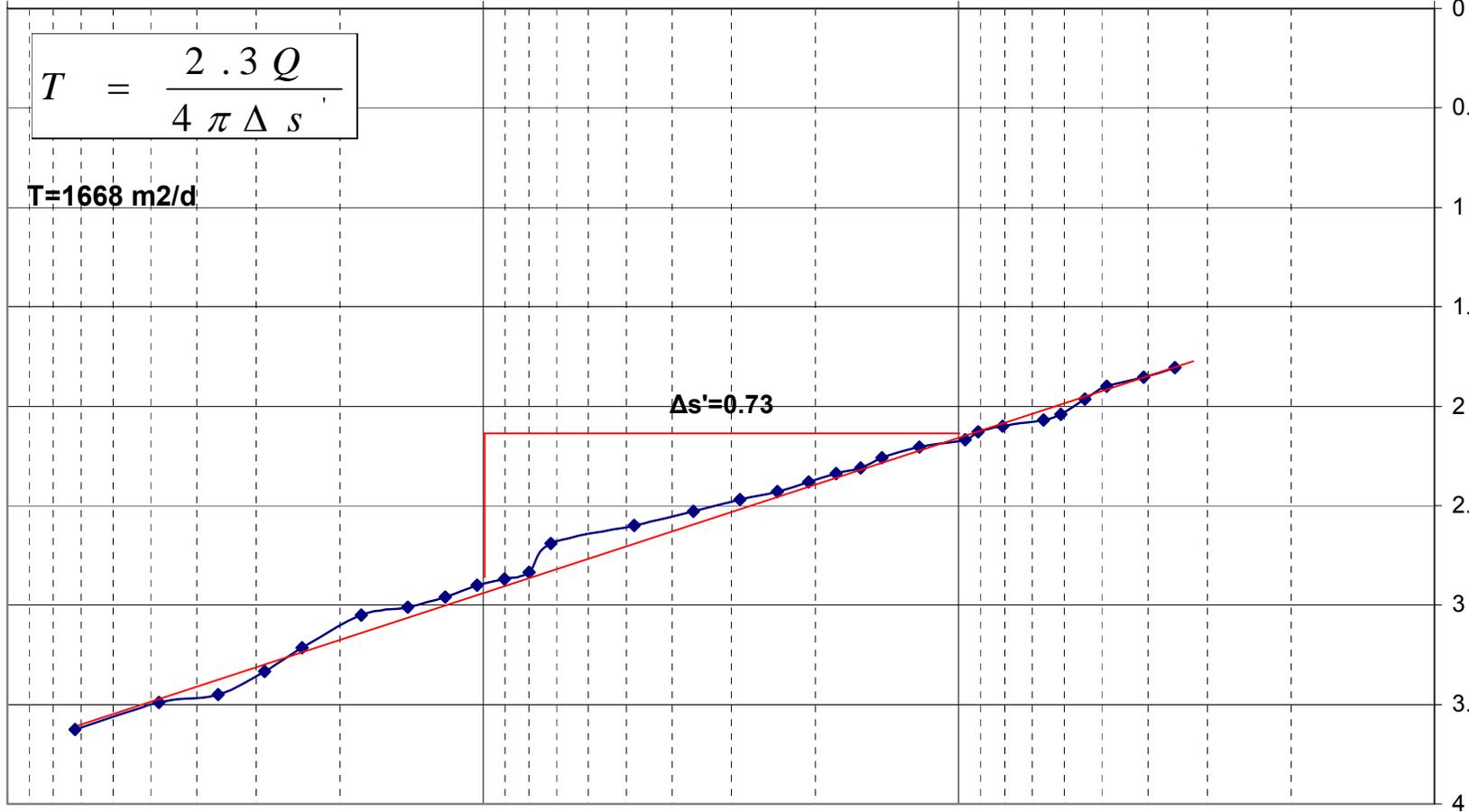
10.0

$$T = \frac{2.3 Q}{4 \pi \Delta s'}$$

$T=1668 \text{ m}^2/\text{d}$

$\Delta s'=0.73$

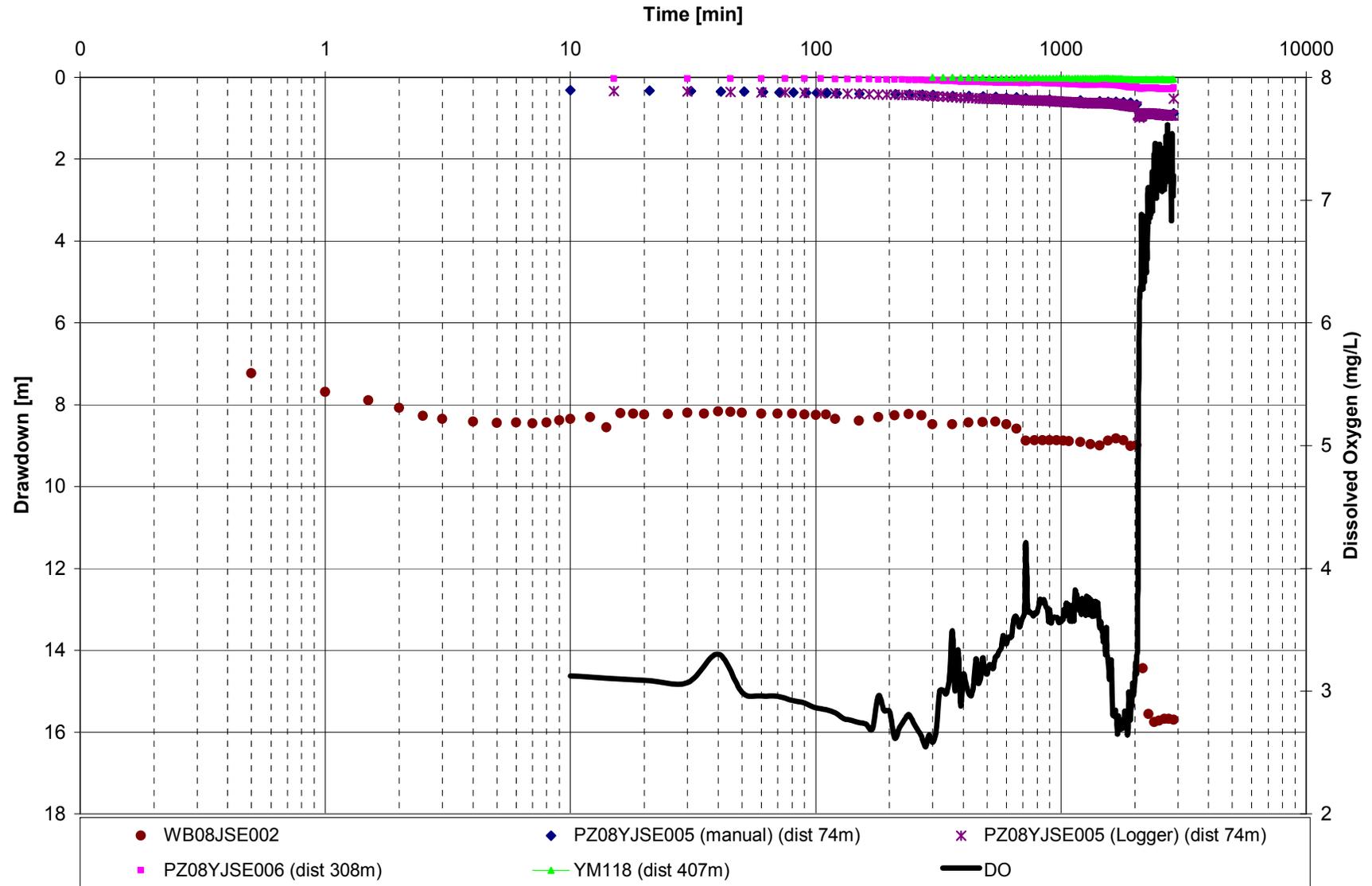
Residual Drawdown [m]



WB08YJSE002 Aquifer Parameters

Bore	Stratigraphical Unit	Analysis Method	Distance from well (m)	Standing water level (mbrp)	Drawdown (m)	Calculated Aquifer Parameters				
						Storativity (S)	Specific Yield (S <sub>v</sub> )	T (m <sup>2</sup> /d)	K (m/d)	Average T (m <sup>2</sup> /d)
WB08YJSE002	ALLUVIAL/INSITU WEATHERED BEDROCK	Theis Recovery (manual)	0	10.8	9	-	-	329.5	11	150
		Logan (manual)				-	-	137	4.5	
		Neuman				-	-	108	3.6	
PZ08YJSE005	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman	74	10.2	0.523	8.93x10 <sup>-5</sup>	2.16x10 <sup>-3</sup>	612	9.14	600
		Cooper-Jacob (All)				-	-	589	8.8	
		Theis				4.31x10 <sup>-1</sup>	-	687	10.2	
PZ08YJSE006	ALLUVIAL/INSITU WEATHERED BEDROCK	Neuman (manual)	308	10.05	0.252	2.89x10 <sup>-4</sup>	3.25x10 <sup>-4</sup>	771	11.5	800
		Cooper-Jacob (All)				2.2x10 <sup>-1</sup>	-	1004	15.6	
		Theis				-	-	1008	16.2	

### WB08YJSE002 Constant Rate Test (Semi-Log)



### WB08YJSE002 Recovery Test

